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Campbell, CA 95008
PH: 408.879.0600
FAX: 408.377.6066

ADD 01

Addendum No. 01

PROJECT: **Shade Structures at Five Sites**
Berryessa Union School District
Bid No. B-02-2020-21

Date: 10/26 /2020

1. This Addendum shall supersede all previously issued Contract Documents wherein it modifies same. All other conditions of the Contract remain unchanged. The following changes, additions, or deletions as set forth herein shall apply to the Contract Documents and shall be made a part thereof and shall be subject to all the requirements thereof as though originally shown and/or specified.
2. Bidders shall acknowledge receipt of this Addendum on Bid Form.
3. **SPECIFICATION REVISIONS**

 Item 1.1 Document 00 43 13 – Bid Bond - See attached bid bond form with the correct project name.
4. **APPROVED SHADE STRUCTURE SUBSTITUTIONS – PLAN SETS**

 Item 1.2 – Morrill Middle School
 Item 1.3 – Piedmont Middle School
 Item 1.4 – Noble Elementary School
 Item 1.5 – Ruskin Elementary School
 Item 1.6 – Vinci Park Elementary School
5. **PRE-BID RFI'S**

 Items 1.7 – 1.18 – See attached pre-bid RFI's and responses 1 through 11.
6. **ATTACHMENTS**

 Document 00 43 13 – Bid Bond (2 pages)
 Pre-Bid RFI'S #1 - #11 (11 pages)
 Morrill MS – Plan Set – (14 pages)
 Piedmont MS – Plan Set – (18 pages)
 Noble ES – Plan Set – (9 pages)
 Ruskin ES – Plan Set – (9 pages)
 Vinci Park ES – Plan Set - (11 pages)

END OF ADDENDUM 1

Mark Finney
Sugimura Finney Architects

DOCUMENT 00 43 13**BID BOND**

KNOW ALL MEN BY THESE PRESENTS that we the undersigned _____ as Principal and _____ as Surety, are hereby held and firmly bound unto the Berryessa Union School District ("Owner") in the sum of _____ Dollars (\$_____) for payment of which sum, well and truly to be made, we hereby jointly and severally bind ourselves, our heirs, executors, administrators, successors and assigns.

The condition of the above obligation is such that whereas the Principal has submitted to the Owner a certain bid, attached hereto and hereby made a part hereof, to enter into a Contract in writing for the construction of **Shade Structures at Five Sites** in strict accordance with Contract Documents.

NOW, THEREFORE,

- a. If said bid shall be rejected, or, in the alternative;
- b. If said bid shall be accepted and the Principal shall execute and deliver a contract in the form of agreement attached hereto and shall execute and deliver Performance and Payment Bonds in the forms attached hereto (all properly completed in accordance with said bid), and shall in all other respects perform the agreement created by the acceptance of said bid;

Then this obligation shall be void, otherwise the same shall remain in full force and effect, it being expressly understood and agreed that the liability of the Surety for any and all default of the Principal hereunder shall be the amount of this obligation as herein stated.

Surety, for value received, hereby stipulates and agrees that no change, extension of time, alteration or addition to the terms of the Contract on the call for bids, or to the Work to be performed hereunder, or the specifications accompanying the same, shall in any way affect its obligation under this bond, and it does hereby waive notice of any such change, extension of time, alteration or addition to the terms of said Contract or the call for bids, or to the Work, or to the specifications.

IN WITNESS WHEREOF, the above-bounden parties have executed this instrument under several seals this ____ day of _____, 202__, the name and corporate party being hereto affixed and these presents duly signed by its undersigned representative, pursuant to authority of its governing body. In the presence of:

(Notary Seal)

(Principal)

(Business Address)

(Corporate Surety)

Business Address)

By:_____

The rate or premium of this bond is _____ per thousand, the total amount of premium charged, \$_____.

(The above must be filled in by Corporate Surety).

END OF DOCUMENT



SUBMITTAL

For **APPROVED EQUAL -**
Morrill ES - Berryessa USD



Valley School Shelters & Covered Walkways
Submitted by:



1555 Tahoe Court
Redding . California . 96003

Toll Free: 877-473-7619
Facsimile: 530-246-0518

PROPOSED EQUAL TO CURRENT MORRIL DSA APPROVED DRAWINGS

(E) MAIN BUILDING

(E) LANDSCAPING

(E) CONC. PAVING

Structure 'E' 14'x41'-8"

Structure 'C' 14'x34'

Structure 'F' 14'x41'-8"

Structure 'A' 14'x34'

Structure 'B' 14'x34'

Structure 'D' 14'x28'

Structure 'G' 14'x36'

(E) AMPHITHEATER CONC. BOTTOM LEVEL -6'-0"

(E) LANDSCAPING

(E) CONC. PAVING








(E) BUILDING "THE UNION"

A0.2

[illegible]

1. (N) UPPER LEVEL SHADE STRUCTURE PC # 04-117117 OPTION VC14.
SEE MANUFACTURER'S DRAWINGS.
2. AMPHITHEATER NEW SHADE STRUCTURE PC # 04-117117 OPTION VC14.
SEE MANUFACTURER'S DRAWINGS.
3. (N) SHADE STRUCTURE COLUMN 13'-0" HEIGHT, SEE 2/S-7, 2/S-5
4. (N) SHADE STRUCTURE COLUMN 11'-0" HEIGHT, TYP. SEE 2/S-7, 2/S-5
6. NOT USED
6. NOT USED
7. (N) SHADE STRUCTURE COLUMN FOOTING, TYP. SEE 1 & 3/S-10
8. SEISMIC GAP PER S-2, TYP.
9. ROOF DECK CANTILEVER, TYP. PER DET. 3/S6, 3/S5, 5/S11
10. (E) STORM DRAIN INLET, VERIFY EXACT LOCATION (TABLE.)
11. TEMPORARY FENCING, SEE DET. 1/A0.4
12. WHEELCHAIR SPACES WITH COMPANION SEAT PER CBC 11B-221
ASSEMBLY AREA

-
- EXISTING PROPERTY LINE
- ROOF OVERHANG
- CHAINLINK FENCE
- WOOD FENCE
- DECORATIVE FENCE

- | | |
|---|------------------------------|
|  | NEW SHADE STRUCTURE |
|  | EXISTING BUILDING |
|  | EXISTING RESTROOMS |
|  | (E) DRY STAND PIPE |
|  | DRINKING FOUNTAIN |
|  | (E) FIRE HYDRANT |
|  | (E) SIGN |
| M | (E) MENS TOILET ROOM |
| W | (E) WOMENS TOILET ROOM |
| G | (E) GIRLS TOILET ROOM |
| B | (E) BOYS TOILET ROOM |
| U | (E) UNISEX TOILET ROOM |
| K | (E) KINDERGARTEN TOILET ROOM |

SUGIMURA
FINNEY
ARCHITECTS

SFA

ARCHITECTURE INTERIORS PLANNING

2155 SOUTH BASCOM AVE.
SUITE 200
CAMPBELL, CA 95003
PHONE: 408-379-0600
FAX: 408-377-6066



NEW ENLARGED SITE PLAN

SHADE STRUCTURES
MORRILL MIDDLE SCHOOL
1970 MORRILL AVE., SAN JOSE, CA 95132
BERRYESSA UNION SCHOOL DISTRICT

[illegible]

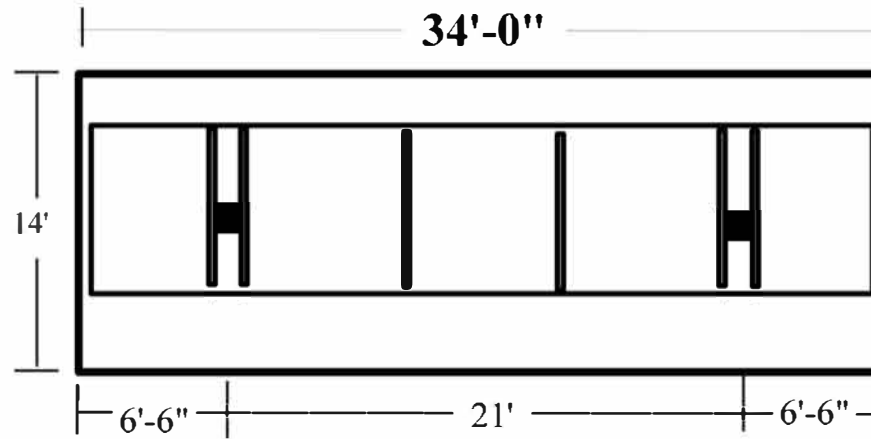
A0.2

Valley School Shelters

Single Post Walkway Cover

14' x 34'-0" with Maximum Cantilevered Ends

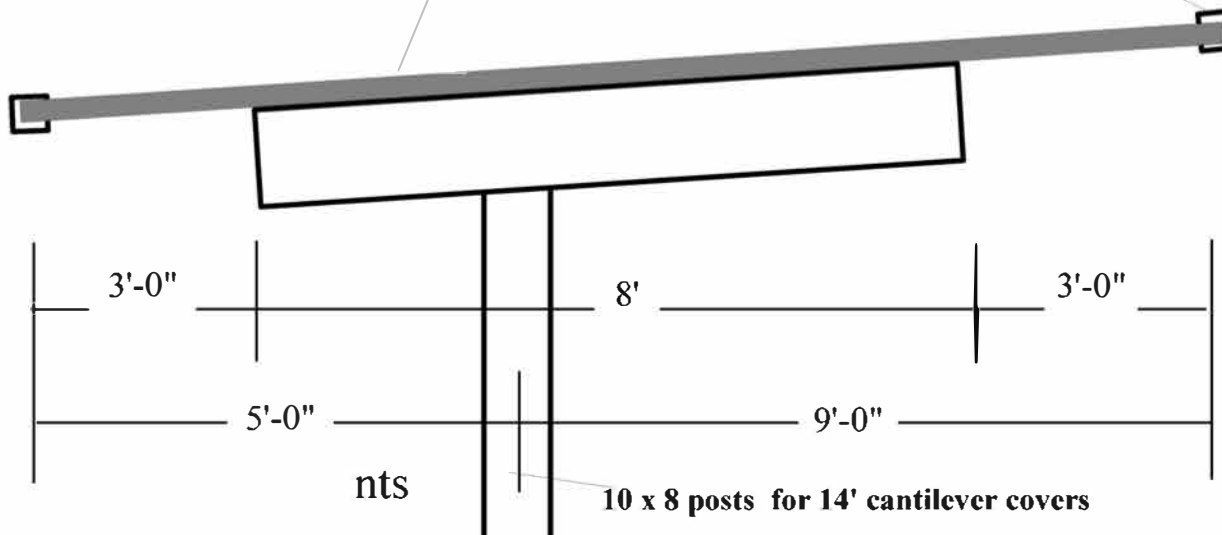
MORRILL ES STRUCTURES 'A', 'B', and 'C'



**1/4:12 up to 2:12
pitch STD**

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included

Posts can be set from 21' up to 25'
on center with 6'-6" cantilevers
maximum to 4'-6" minimum



Valley School Shelters
Tulare, CA 93275
www.SchoolLunchShelters.com

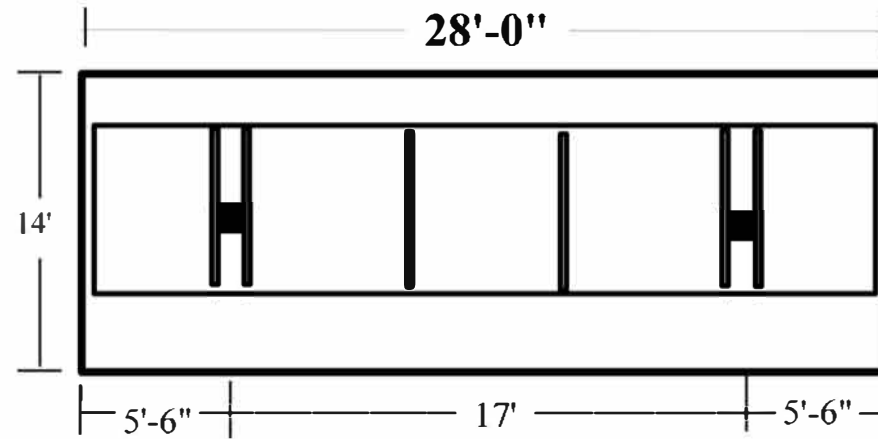
NTS

Valley School Shelters

Single Post Walkway Cover

14' x 28'-0" with Maximum Cantilevered Ends

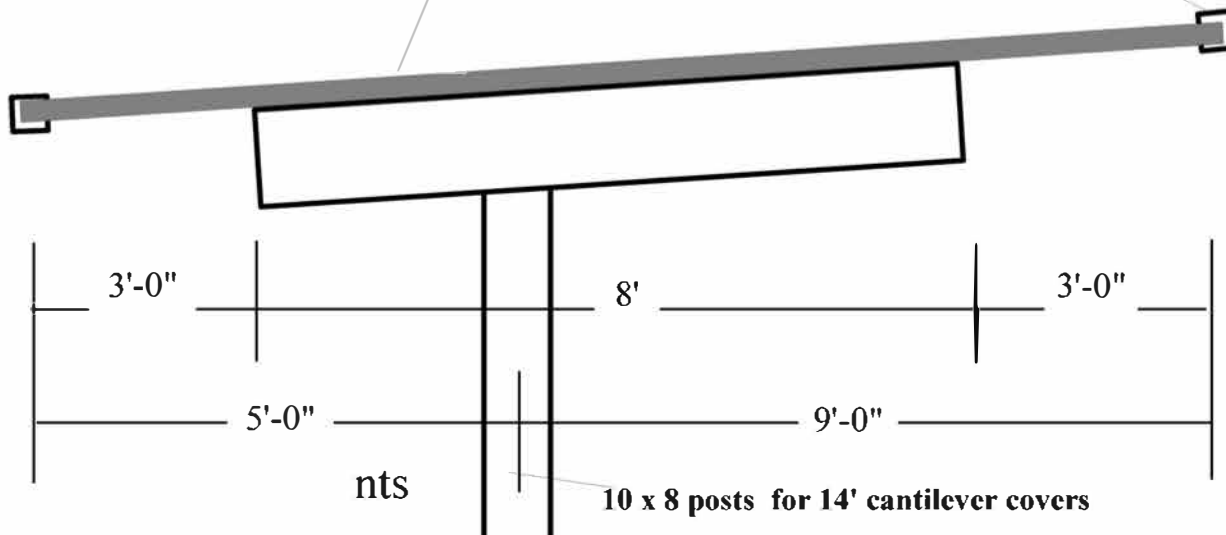
MORRILL ES STRUCTURE 'D'



**1/4:12 up to 2:12
pitch STD**

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included

Posts can be set from 17' up to 25'
on center with 5'-6" cantilevers
maximum to 1'-6" minimum



Valley School Shelters
Tulare, CA 93275
www.SchoolLunchShelters.com

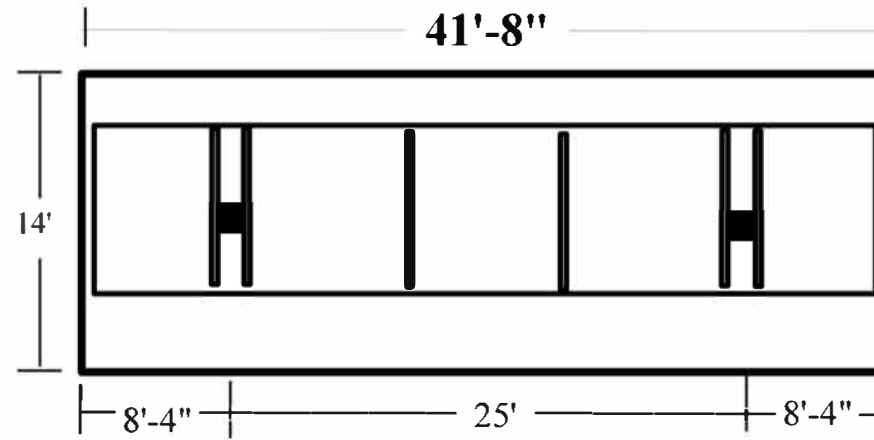
NTS

Valley School Shelters

Single Post Walkway Cover

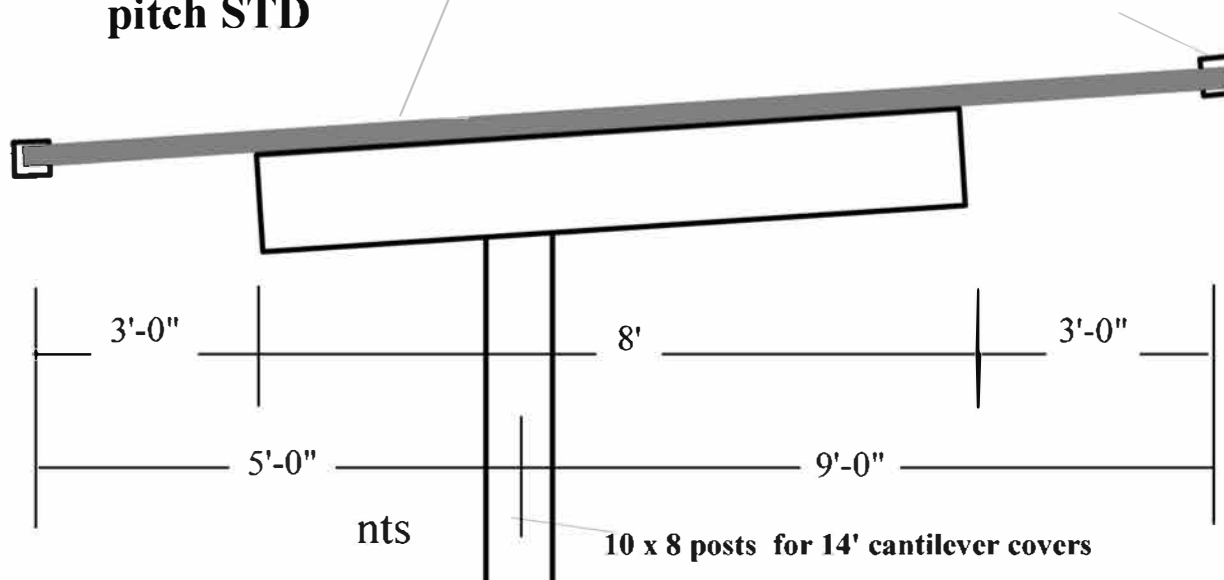
14' x 41'-8" with Maximum Cantilevered Ends

MORRIL ES STRUCTURES 'E' and 'F'



**1/4:12 up to 2:12
pitch STD**

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included



This is the maximum length of a
single post walkway when using
only two posts

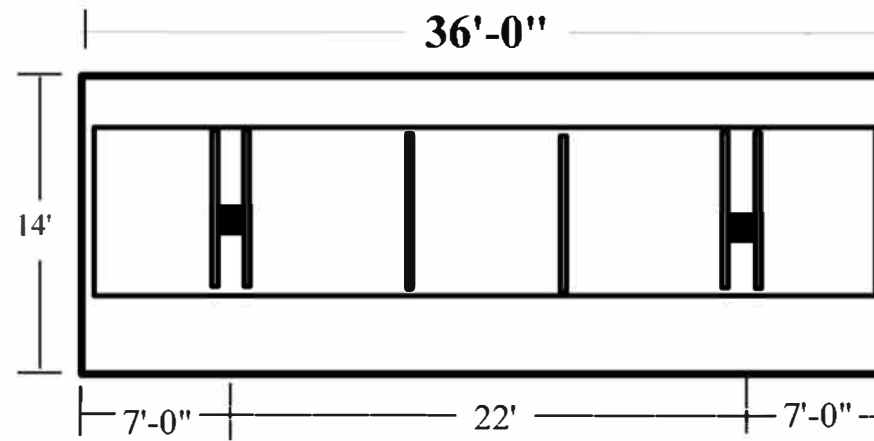
Valley School Shelters
Tulare, CA 93275
www.SchoolLunchShelters.com
NTS

Valley School Shelters

Single Post Walkway Cover

14' x 36'-0" with Maximum Cantilevered Ends

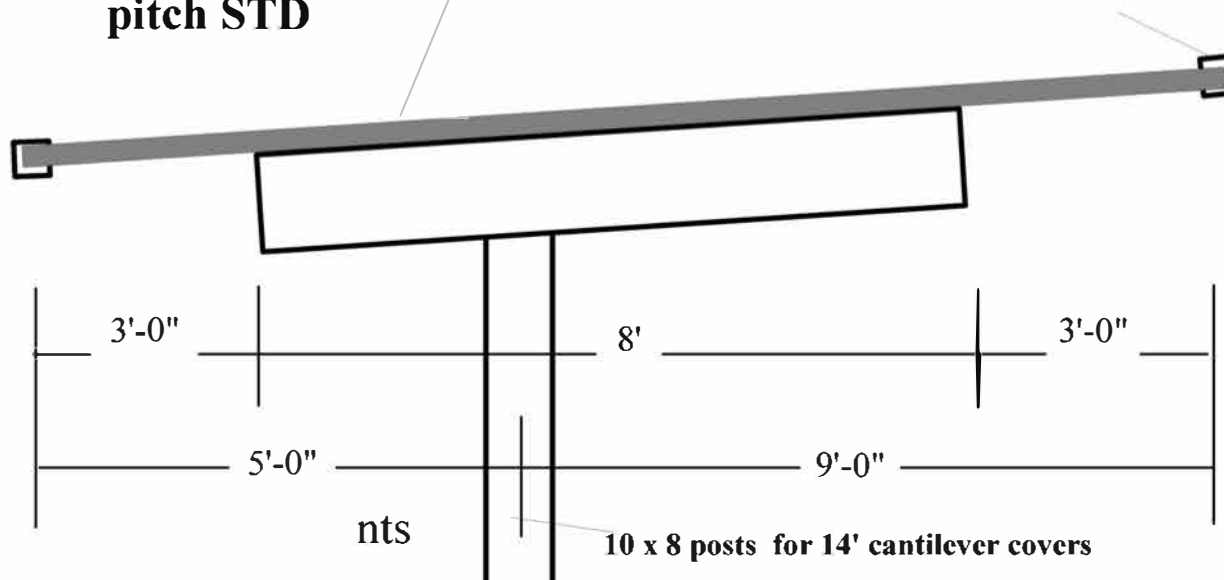
MORRILL ES STRUCTURE ' G'



**1/4:12 up to 2:12
pitch STD**

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included

Posts can be set from 22' up to 25'
on center with 7' cantilevers
maximum to 5'-6" minimum



Valley School Shelters
Tulare, CA 93275
www.SchoolLunchShelters.com

NTS

Single Post Walkway Cover DSA Pre-Checked Plans No Stamp

Stamped drawings to be provided if request for approved
equal is approved.

- Morrill Elementary Structure 'A'
- Morrill Elementary Structure 'B'
- Morrill Elementary Structure 'C'
- Morrill Elementary Structure 'D'
- Morrill Elementary Structure 'E'
- Morrill Elementary Structure 'F'
- Morrill Elementary Structure 'G'

GENERAL NOTES

1. ALL DIMENSIONS, CONDITIONS AND ELEVATIONS ARE TO BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORK OR FABRICATION. IF ANY DISCREPANCIES ARE FOUND OR IF ANY CONDITION EXISTS NOT AS SHOWN ON THE DRAWINGS THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

2. THE CONTRACTOR WILL HOLD HARMLESS, INDEMNIFY AND DEFEND THE OWNER, THE ENGINEER, AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS, FROM ANY AND ALL LIABILITY CLAIMS, LOSSES OR DAMAGE ARISING OR ALLEGED TO ARISE FROM THE PERFORMANCE OF THE WORK DESCRIBED HEREIN, BUT NOT INCLUDING THE SOLE NEGLIGENCE OF THE OWNER, THE ENGINEER AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS.

3. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE DESIGN INTENT. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES.

IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.

ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION.

4. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON, OR NEAR THE CONSTRUCTION SITE.

5. ANY SUPPORT SERVICES PERFORMED BY THE ENGINEER OR HIS REPRESENTATIVES DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER OF MATERIAL OR WORK AND WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION ARE NOT PERFORMED SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATION, BUT THEY DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

6. ANY CHANGES TO THE APPROVED SET OF PLANS WITHOUT NOTIFYING THE ENGINEER PRIOR TO SUCH CHANGES ABSOLVES SAID ENGINEER FROM ANY AND ALL RESPONSIBILITY WITH RESPECT TO LIABILITY, DAMAGE OR EXTRA WORK RESULTING FROM SAID CHANGES.

7. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.

8. THE TYPICAL DETAILS SHOWN ON THESE SHEETS SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY SHOWN OTHERWISE. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS SHOWN FOR OTHER SIMILAR WORK.

9. DESIGN LOADS: RISK CATEGORY II OR RISK CATEGORY III
ROOF LIVE LOAD: 20 PSF, ROOF DEAD LOAD: 3 PSF
SNOW LOAD: $E_s = P_g + 20 \text{ PSF} + 30 \text{ PSF}$ $C_s = 1.1$ $C_t = 1.2$ $C_e = 1.0$
 $I_s = 1.0$ RISK CATEGORY II $I_s = 1.1$ RISK CATEGORY III

SEISMIC: $I_e = 1.0$ AT RISK CATEGORY II $I_e = 1.25$ AT RISK CATEGORY III
SEISMIC DESIGN CATEGORY = D $\rho = 1.3$ FOR OFFSET CONFIGURATION
 $S_{DS} = 1.0$ & $S_{D5} = 1.75$

STRUCTURE IS A STEEL, ORDINARY CANTILEVERED COLUMN SYSTEM (G2 PER ASCE7-10)
 $R = 1.25/0.7$ (FOR WORKING STRESS)
 $C_D = 1.25$ $C_A = 1.25$
SOIL SITE CLASS = D
FOR 12' COLUMN HEIGHT $T = 0.361$ RISK CATEGORY II $V = 0.728W$ (ASD) FOR $S_{DS} = 1.0$
FOR 14' COLUMN HEIGHT $T = 0.45$ RISK CATEGORY II $V = 1.82W$ (ASD) FOR $S_{DS} = 2.5$
FOR 16' COLUMN HEIGHT $T = 0.48$ HIGH SEISMIC $C_{u1} V = 0.716W$ (ASD) FOR $S_{DS} = 2.5$
HIGH SEISMIC $C_{u1} V = 1.55W$ (ASD) FOR $S_{DS} = 2.5$

WIND LOAD: 135 MPH, EXPOSURE C $K_d = 0.85$ RISK CATEGORY II OR RISK CATEGORY III
 $K_z = 0.90$ $K_{zt} = 1.0$
 $q_h = 21.42$ ($q_h = 0.00256 \times K_z \times K_{zt} \times K_d \times V^2 \times 0.6$ (FOR WORKING STRESS))
MAXIMUM BAY SPACING FOR PROJECT LOCATED IN SPECIAL WIND REGIONS SHALL BE EQUAL TO OR LESS THAN 135 MPH AND CONFORM WITH THE ADOPTED ORDINANCE OF THE CITY, COUNTY OR CITY AND COUNTY IN WHICH THE PROJECT SITE IS LOCATED AND SHALL BE APPROVED BY DSA-SS.

10. GOVERNING CODE: 2016 CBC

11. ALLOWABLE SOIL BEARING IS BASED ON 1500 PSF & 100 PCF/2 PASSIVE PRESSURE PER CBC TABLE 1806A.2 & SECTION 1806A.3.4. SKIN FRICTION IS PER 1810A.3.1.4 & IS EQUAL TO 1500/6=250 PSF. FOR UPLIFT SKIN FRICTION SHALL BE 125 PSF (S.F. OF 2).

12. ALL WORK TO BE PERFORMED UNDER THE CONTINUOUS INSPECTION OF A D.S.A. APPROVED INSPECTOR.

13. ALL WORK SHALL CONFORM TO TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

14. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDA OR A CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR AND THE ENGINEER.

15. A PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE OFFICE OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

FOUNDATION NOTES

1. ALL FOOTINGS SHALL EXTEND TO FIRM BEARING IN UNDISTURBED SOIL OR ENGINEERED FILL.

2. NOMINAL TOP OF FLOOR SLAB ELEVATION = DATUM +0'-0" UNLESS OTHERWISE NOTED.

3. ANY EXISTING FILL AT THE BUILDING PAD SHALL MEET THE 82% COMPACTION REQUIREMENTS. ALL ORGANIC MATERIAL, RUBBLE, OR OTHER DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SITE.

4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR WALKWAYS.

5. ALL REINFORCING STEEL, ANCHOR BOLTS, AND OTHER EMBEDDED ITEMS SHALL BE SECURELY POSITIONED IN THE FORMS PRIOR TO POURING OF CONCRETE.

6. SHORING AND BRACING: IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, AND FORM WORK AS REQUIRED FOR THE CONSTRUCTION OF THIS BUILDING. PROVIDE TEMPORARY BRACING AS REQUIRED TO HOLD THE VARIOUS ELEMENTS IN PLACE UNTIL FINAL SUPPORT IS SECURELY ANCHORED.

7. EXCAVATION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL EXCAVATION PROCEDURES AND FOR PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.

8. BACKFILL: DO NOT BACKFILL AROUND THE EXTERIOR PERIMETER WALL UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. IF THE FLOOR SLABS ARE CONCRETE, DO NOT BACKFILL UNTIL 7 DAYS MINIMUM AFTER COMPLETION OF THE FLOOR SLABS. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF DAMP-PROOFING.

CONCRETE NOTES

1. ALL MOLDS, ORNAMENTS, GROOVES, ETC. SHOWN ON THE ARCHITECTURAL DRAWINGS SHALL BE PROVIDED FOR IN THE FORM WORK BEFORE THE CONCRETE IS Poured.

2. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER EMBEDS SHALL BE IN PLACE AND SECURED TO FORM WORK PRIOR TO POURING OF CONCRETE.

3. REFER TO BOTH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF PLUMBING FIXTURES.

4. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE WALLS OR STRUCTURAL SLABS UNLESS SPECIFICALLY DETAILED.

5. CONSTRUCTION JOINTS NOT INDICATED ON THE DRAWINGS SHALL BE SO MADE AND LOCATED AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. PROVISION SHALL BE MADE FOR TRANSFER OF SHEAR AND OTHER FORCES THROUGHOUT THE JOINTS. THE CONTRACTOR SHALL OBTAIN THE ARCHITECT'S APPROVAL OF CONSTRUCTION JOINT LOCATION IN ALL STRUCTURAL SLAB, BEAMS AND SHEAR WALLS.

6. SIDES OF FOOTINGS MAY BE POURED AGAINST STABLE EARTH.

7. THE QUALITY AND DESIGN OF CONCRETE SHALL COMPLY WITH TITLE 24 PART 2 EXCEPT ITEMS NOT SPECIFICALLY COVERED THEREIN SHALL CONFORM TO ACI 318.

8. ALL REINFORCING SHALL BE NEW STOCK DEFORMED BARS CONFORMING TO ASTM A615.

A. #4 BARS AND SMALLER.....GRADE 40 OR 60
B. #5 BARS AND LARGER.....GRADE 60
C. SEPARATE BARS 1-1/2 DIAMETERS CLEAR OR 1-1/2" CLEAR, WHICHEVER IS LARGER.

9. MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:
CAST AGAINST EARTH (EXCEPT SLABS ON GRADE).....3"
EXPOSED TO EARTH OR WEATHER.....1-1/2"
#5 BARS AND SMALLER.....1-1/2"
#6 BARS AND LARGER.....2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
SLABS, WALLS, JOISTS.....3/4"
#11 BARS AND SMALLER.....1-1/2"
#14 AND #18 BARS.....1-1/2"
BEAMS, GIRDERS, COLUMNS
PRINCIPAL REINFORCING, TIES
STIRRUPS, OR SPIRALS.....1-1/2"
SHELLS AND FOLDED PLATE MEMBERS
#5 BARS AND SMALLER.....1/2"
#6 BARS AND LARGER.....3/4"

10. CONCRETE SHALL HAVE FOLLOWING MINIMUM REQUIREMENTS.
F0- 3000 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.5.
F2- 4500 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.45 & MINIMUM AIR CONTENT OF 7%.

11. THE ENGINEER DOES NOT PROVIDE CONTRACT ADMINISTRATION FOR THE PROJECT, INCLUDING REVIEW OF CONCRETE MIXES.

12. PER ACI SECTION 19.3.21. FOOTING SHALL NOT BE EXPOSED TO FREEZING AND THAWING CYCLES. SHALL NOT BE EXPOSED TO WATER-SOLUBLE SULFATE IN SOIL BY PERCENT OF MASS 20.10%, SHALL NOT BE EXPOSED TO EXTERNAL SOURCE OF CHLORIDES.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL EXCEPT W SHAPES SHALL CONFORM TO ASTM A-36 AND SHALL BE FABRICATED AND ERECTED AS PER AISC SPECIFICATIONS FOR BUILDINGS. W SHAPES SHALL CONFORM TO ASTM A992.

2. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE "B" AND STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500 GRADE "B", $F_y=46$ KSI.

3. ALL LIGHT GAGE STEEL TO CONFORM TO ASTM A653 GRADE 55 FOR ALL STRUCTURAL SHAPES, A653 GRADE 33 FOR ALL BLOCKING, FLASHINGS, MISCELLANEOUS CONNECTION PLATES, AND ANGLES.

4. ALL UNFINISHED BOLTS SHALL BE ASTM A-307 UNLESS NOTED OTHERWISE.

5. USE AISC USUAL GAGES FOR BOLT HOLES IN ALL STEEL SECTIONS UNLESS OTHERWISE NOTED.

6. THE STEEL FABRICATOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR ERECTION.

7. ALL BOLT HOLES ARE TO BE 1/16" OVERSIZED. ALL BOLTS SHALL HAVE WASHERS INSTALLED UNDER BOTH HEAD & NUT.

8. ALL STEEL SHALL BE PROTECTED FROM WEATHER AS FOLLOWS: STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED (MINIMUM ASTM A123 OR A153, CLASS D) OR PAINTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT OR EQUIVALENT PAINT SYSTEM. COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM-ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION.

ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT-DIP GALVANIZED (ASTM A153, CLASS D MINIMUM), OR PROTECTED WITH CORROSION-PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT. (EXAMPLE: PROPRIETARY COATINGS THAT DO COMPLY WITH THE 1,000 HOUR REQUIREMENT INCLUDE, BUT ARE NOT NECESSARILY LIMITED TO: QUIK GUARD BY SIMPSON, Kwik-COTE BY HILTI, STALGARD BY ELCO, VISTACORR BY SFS INTEC, ETC.)

GOVERNING CODES:

1. 2016 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE (PART 1, TITLE 24, CCR).

2. 2016 CALIFORNIA BUILDING CODE, VOLUMES 1 & 2 (PART 2, TITLE 24, CCR).

3. 2016 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR).

4. 2016 CALIFORNIA FIRE CODE (PART 9, TITLE 24, CCR).

NOTES

1. COVERS ARE NOT DESIGNED TO BE ENCLOSED OR FOR STORAGE OF COMBUSTIBLE MATERIALS.

2. WALKWAY COVER HAS BEEN CHECKED FOR OBSTRUCTED WIND FLOW CONDITION & CAN BE WITHIN 6" MIN. FROM AN EXISTING BUILDING. SHALL BE REVIEWED ON A SITE SPECIFIC BASIS.

3. WALKWAY PIER FOOTING HAS BEEN CHECKED FOR D.S.A. BULLETIN 09-06 REV.

TESTING & INSPECTIONS REQUIREMENTS

1. INSPECTOR CLASS (MINIMUM REQUIREMENTS)
CLASS 2

2. SELECTION OF THE PROJECT INSPECTOR AND TESTING AGENCY
BY THE SCHOOL DISTRICT AND APPROVED BY D.S.A., A/E OF RECORD AND STRUCTURAL ENGINEER

3. COST OF THE PROJECT INSPECTOR (CA ADMIN. CODE 4-333(B)) AND TESTING AGENCY (CA ADMIN. CODE 4-333)
BY THE SCHOOL DISTRICT

4. COPIES OF THE REPORT TO
ARCHITECT; STRUCTURAL ENGINEER; SCHOOL DISTRICT; D.S.A. (ORIGINAL); IOR; MANUFACTURER

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316 (D & E) OF THE CALIFORNIA CODE OF REGULATIONS, THE DISTRICT SHALL HIRE AN ARCHITECT OR STRUCTURAL ENGINEER TO BE IN GENERAL RESPONSIBLE CHARGE OF SITE SPECIFIC PROJECT.

2. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.

3. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR A PORTION OF THE PROJECT AS DESIGNED BY THE ARCHITECT FOR INCORPORATION INTO THE PROJECT.

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GERARD HOMER & ASSOCIATES RESPONSIBILITY FOR SITE SPECIFIC PROJECT.

SHADE STRUCTURE TESTING & INSPECTION GUIDELINE				
THE EXAMPLE FORM DSA-103 SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECT-SPECIFIC FORM DSA-103. A FORM DSA-103 IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND THE EXAMPLE FORM DSA-103 IS TO BE CROSSED OUT ON THIS DRAWING.				
DSA-103 STATEMENT OF STRUCTURAL TESTS & SPECIAL INSPECTIONS - 2016 CBC				
IMPORTANT: THIS FORM IS ONLY A SUMMARY LIST OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS REQUIRED FOR A PROJECT. THE ACTUAL TESTS AND INSPECTIONS MUST BE PERFORMED AS DETAILED ON THE DSA APPROVED DOCUMENTS. THE PROJECT INSPECTOR IS RESPONSIBLE FOR PROVIDING INSPECTION OF ALL FACETS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, SPECIAL INSPECTIONS NOT LISTED ON THIS FORM SUCH AS STRUCTURAL WOOD FRAMING, HIGH-LOAD WOOD DIAPHRAGMS, COLD-FORMED STEEL FRAMING, ANCHORAGE OF NON-STRUCTURAL COMPONENTS, ETC., PER TITLE 24, PART 2, CHAPTER 17A.				
NOTE: REFERENCES ARE TO THE 2016 EDITION OF THE CALIFORNIA BUILDING CODE (CBC) UNLESS OTHERWISE NOTED.				
TEST OR SPECIAL INSPECTION	TYPE	PERFORMED BY	CODE REFERENCE AND NOTES	
SOILS				
1. GENERAL:	TABLE 1705A.6			
A. VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF CONTROLLED FILL AND/OR EXCAVATIONS FOR FOUNDATIONS.	PERIODIC	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE	
B. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH PIER.	CONTINUOUS	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE	
C. VERIFY LOCATIONS OF PIERS.	CONTINUOUS	PI		
D. CONFIRM PIER DIAMETERS, PLUMBNESS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, AND EMBEDMENT INTO BEDROCK (IF APPLICABLE). RECORD CONCRETE OR GROUT VOLUMES.	CONTINUOUS	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE	
E. CONCRETE PIERS.	PROVIDE TEST AND INSPECTIONS PER CONCRETE SECTION BELOW			
CONCRETE				
TABLE 1705A.3				
CAST IN PLACE CONCRETE				
MATERIAL VERIFICATION AND TESTING:				
A. VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	SI & PI	TO BE PERFORMED BY BATCH-PLANT SPECIAL INSPECTOR AND PROJECT INSPECTOR.	
B. TEST REINFORCING STEEL.	TEST	LAB	1913A.2 (1913.2.6), ASTM A370, DSA IR17-10	
C. PERFORM SLUMP, TEMPERATURE, AND (WHERE REQUIRED) AIR CONTENT TEST.	TEST	LAB	ASTM C172, ASTM C31	
D. TEST CONCRETE (COMPRESSION).	TEST	LAB	ACI 318 SECTION 5.6 AND 1905A.1.2 (1913.3.1+), ASTM C39.	
INSPECTION:				
E. BATCH PLANT INSPECTION	CONTINUOUS	SI	1705A.3.2. IF APPROVED BY DSA, BATCH PLANT INSPECTION MAY BE REDUCED TO PERIODIC IF PLANT COMPLIES WITH 1705A.3.3. ITEM 1, AND REQUIRES FIRST BATCH INSPECTION, WEIGHMASTER, AND BATCH TICKETS.	
F. BATCH PLANT INSPECTION - DESIGN COMPLIES WITH 1705A.3.3 ITEM 2	PERIODIC	SI	1705A.3.3. ITEM 2. REQUIRES FIRST BATCH INSPECTION, WEIGHMASTER, AND BATCH TICKETS.	
G. INSPECT PLACEMENT OF FORMWORK, REINFORCING STEEL, EMBEDDED ITEMS, AND CONCRETE. INSPECT CURING AND FORM REMOVAL.	CONTINUOUS	PI	MAY BE PERFORMED BY A SPECIAL INSPECTOR WHEN SPECIFICALLY APPROVED BY DSA.	
STEEL				
TABLE 1705A.2.1, AISC 303-10, AISC 360-10, AISC 341-10, AISC 358-10, AISI S100-07/82-10				
17. STRUCTURAL STEEL AND COLD-FORMED STEEL USED FOR STRUCTURAL PURPOSES				
MATERIAL VERIFICATION:				
A. VERIFY THAT ALL MATERIALS ARE APPROPRIATELY MARKED AND THAT: MILL CERTIFICATES INDICATE MATERIAL PROPERTIES THAT COMPLY WITH REQUIREMENTS. MATERIAL SIZES, TYPES AND GRADES COMPLY WITH REQUIREMENTS.	PERIODIC	*	BY SPECIAL INSPECTOR WHEN PERFORMED OFF-SITE; BY PROJECT INSPECTOR FOR STEEL SHIPPED, DIRECTLY TO PROJECT SITE WITHOUT WELDING OR FABRICATION.	
B. TEST UNIDENTIFIED MATERIALS	TEST	LAB	2203A.1 (2203.1+), ASTM A370	
C. EXAMINE SEAM WELDS OF STRUCTURAL TUBES AND PIPES	PERIODIC	SI	DSA IR 17-3	
D. VERIFY MEMBER LOCATIONS, BRACING AND ALL DETAILS CONSTRUCTED IN THE FIELD.	CONTINUOUS	PI		
E. VERIFY STIFFENER LOCATIONS, CONNECTIONS TAB LOCATIONS AND ALL CONSTRUCTION DETAILS FABRICATED IN THE SHOP.	PERIODIC	SI		
KEY TO COLUMNS				
1. TYPE -	2. PERFORMED BY -			
CONTINUOUS -	INDICATES THAT A CONTINUOUS SPECIAL INSPECTION IS REQUIRED			
PERIODIC -	INDICATES THAT A PERIODIC SPECIAL INSPECTION IS REQUIRED			
TEST -	INDICATES THAT A TEST IS REQUIRED			
SI -	INDICATES THAT THE SPECIAL INSPECTION IS TO BE PERFORMED BY A SPECIAL INSPECTOR			
+ IN THE CODE REFERENCE AND NOTES COLUMN, IT INDICATES DSA-SS/CC SECTIONS THAT MAY BE USED BY COMMUNITY COLLEGES, PER 2016 CBC SEC. 1.9.2.2.				

SHEET INDEX	
SI	FOUNDATION PLANS, GENERAL NOTES, DETAILS
S2	ROOF FRAMING PLANS
S3	SECTION, TYPICAL ELEVATION, DETAILS
S4	SECTION, DETAIL

BUILDING DATA	
A-3 OCCUPANCY	
II-B CONSTRUCTION	
ALLOW AREA = 4500 SQ.FT. TABLE 503	
HEIGHT = 0'-0" MIN. TO 16'-0" MAX. COLUMN HEIGHT	

GENERAL NOTES

SHEET INDEX & STRUCTURAL TESTS AND INSPECTIONS SHEET

4K

SITE SPECIFIC INFORMATION TABLE			
SEISMIC COEFFICIENT, S_{ds}		SELECT OCCUPANT LOAD FACTOR (OLF)	DETERMINATION OF BAY SPACING FOR COVER NEXT TO RISK CATEGORY III & $S_{DS} > 2.0$ $28' \text{ BAYS} \times 2.0 = \square$ MAXIMUM BAY ACTUAL $S_{DS} \times 1.25$ SPACING IN FEET * * BAY SPACING ON PLAN VIEWS TO BE REVISED TO MATCH & USE HIGHER SEISMIC COLLUMS & FOOTINGS.
WIND SPEED (3 SEC. GUST.) @ MPH		ASSEMBLY-CONCENTRATED (7 SQ. FT./OCCUPANT)	
WIND EXPOSURE CATEGORY		ASSEMBLY-NONCONCENTRATED (15 SQ. FT./ OCCUPANT)	
WIDTH OF COVER AREA IN FT.		DETERMINE OCCUPANT LOAD = AREA/OLF	
LENGTH OF COVERED AREA IN FT.		DETERMINE RISK CATEGORY	
AREA IN SQ. FT.		RISK CATEGORY II OCCUPANTS \leq 300	
NUMBER OF BAYS		RISK CATEGORY III OCCUPANTS $>$ 300	
CLEAR HEIGHT OF EAVES IN FT.		RISK CATEGORY III ADJACENT TO RISK CATEGORY III BUILDING	
ROOF SLOPE IN./FT.		NOTE: DUE TO REDUNDANCY FACTOR THERE CAN BE NO REDUCTION IN S_{DS} PER CBC 1816A.1.12 FOR COVERS.	
CONCRETE EXPOSURE F0		CONCRETE EXPOSURE F2*	
GEOHAZARD REPORTS			
GEOHAZARD REPORT	REQUIRED	NOT REQUIRED	LIQUEFABLE SOIL OR SITE CLASS F
			LIQUEFABLE SOILS
			YES
			NO
GEOHAZARD REPORTS ARE NOT REQUIRED FOR CANTILEVERED COLUMN OPEN STRUCTURES PROVIDED THEY ARE CONSTRUCTED OF METAL, DO NOT EXCEED 4,000 ST. IN PLAN AREA AND ARE NOT LOCATED WITHIN STATE OR LOCAL GEOHAZARD ZONES. THE STRUCTURES MAY BE SPLIT INTO MULTIPLE SEISMICALLY SEPARATED STRUCTURES TO STAY BELOW THE 4,000 ST. TRIGGER. IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFABLE SOIL, OR SITE CLASS F, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS NOT IN A MAJOR LIQUEFACTION HAZARD ZONE. IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.			

* F2 IS REQUIRED FOR ANY SNOW LOAD 30 PSF OR GREATER.

SINGLE POLE WALKWAY OPTIONS TABLE			
LOADING	COLUMN HEIGHT	ROOF PANEL OPTION	PROJECTION
20 PSF	12' HEIGHT	MELODY MEGA-RIB	0'-0"
20 PSF	14' HEIGHT	AEP SPAN HR-36	10'-0"
20 PSF	16' HEIGHT	3 P.S.F.*	12'-0"
ROOF PITCH	COLUMN LOCATION	SEISMIC	DEAD LOAD
2:12 MAX.	OFF-SET	LOWER SEISMIC	YES
4:12 MAX.	CENTERED	HIGHER SEISMIC	NO
			"A" "B" "C" "D"

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC-VC
IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES
APPL # 02-115686

AC TM F/LS H/SS ST
DATE 5/29/18

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

OPTIONAL SPREAD FOOTING
@ ADDITIONAL COST

SCALE 1/2" = 1'-0"

8B

OPTIONS TABLE & SITE SPECIFIC INFORMATION TABLE

8K

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FAX (559) 734-5232
Email: ghomerse@gmail.com

Gerard
Homer and Associates
STRUCTURAL ENGINEERS

SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY: T.E.H.
CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE
FOUNDATION PLAN
GENERAL NOTES
DETAILS

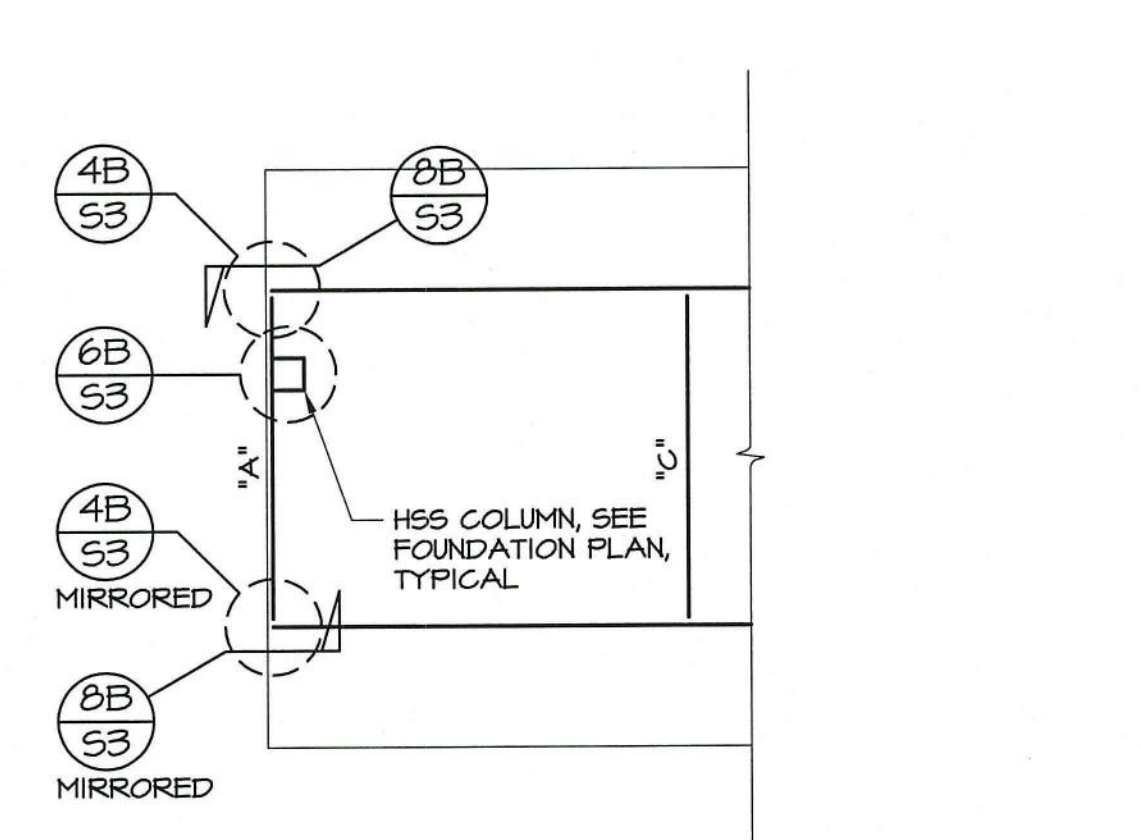
SHEET NUMBER

S1

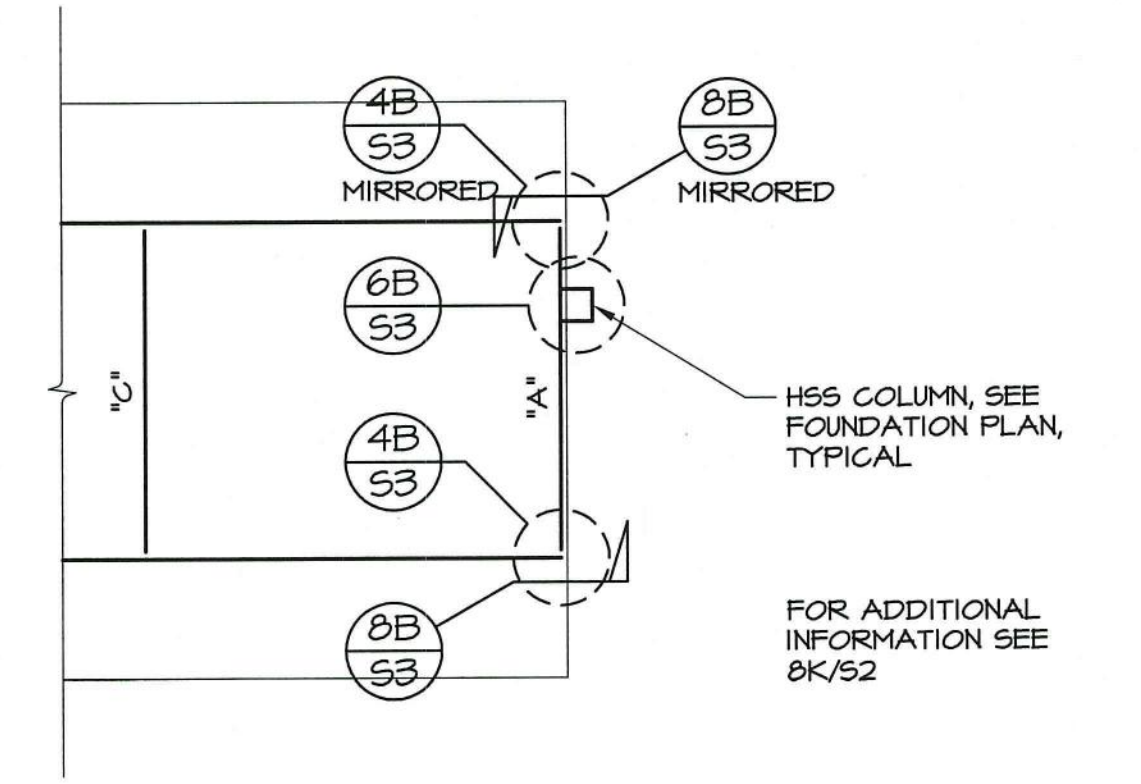
OF 4 SHEETS

COLUMN & FOOTING TABLE (NOTE 6)														
COLUMN HEIGHT	ROOF PITCH	LOAD	LOWER SEISMIC ($S_{DS} = 1.0$ RISK CATEGORY II) ($S_{DS} \leq 0.8$ RISK CATEGORY III)						HIGHER SEISMIC ($S_{DS} = 2.5$ RISK CATEGORY II) ($S_{DS} \leq 2.0$ RISK CATEGORY III)					
			$\leq 12'-0"$ PROJECTION			$\leq 14'-0"$ PROJECTION			$\leq 12'-0"$ PROJECTION			$\leq 14'-0"$ PROJECTION		
			COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1
12' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x1/4	2'-0"Øx10'-3" DEEP	9'-3"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx11'-4" DEEP	10'-4"SQ.x2'-6" THICK (4)	H550x8x3/8	2'-0"Øx11'-10" DEEP	10'-6"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx13'-2" DEEP	11'-4"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H5510x8x3/8	2'-0"Øx12'-2" DEEP	9'-8"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx13'-0" DEEP	10'-10"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx12'-2" DEEP		H5510x8x3/8		
14' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x1/4 H550x8x3/8	2'-0"Øx10'-8" DEEP	9'-8"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx12'-2" DEEP	10'-10"SQ.x2'-6" THICK (4)	H550x8x3/8	2'-0"Øx12'-3" DEEP	11'-0"SQ.x2'-6" THICK (5)	H5510x8x3/8	2'-0"Øx13'-6" DEEP	11'-10"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H5510x8x3/8	2'-0"Øx12'-6" DEEP	9'-10"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx13'-2" DEEP	11'-0"SQ.x2'-6" THICK (5)	H5510x8x3/8	2'-0"Øx12'-6" DEEP		H5510x8x3/8		
16' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x3/8	2'-0"Øx11'-0" DEEP	10'-0"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"Øx12'-6" DEEP	11'-3"SQ.x2'-6" THICK (5)	H5510x8x3/8	2'-0"Øx12'-8" DEEP	11'-4"SQ.x2'-6" THICK (5)	H5510x8x1/2	2'-0"Øx14'-0" DEEP	12'-4"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H5510x8x3/8	2'-0"Øx12'-8" DEEP	10'-2"SQ.x2'-6" THICK (4)	H5510x8x1/2	2'-0"Øx13'-6" DEEP			2'-0"Øx12'-8" DEEP				

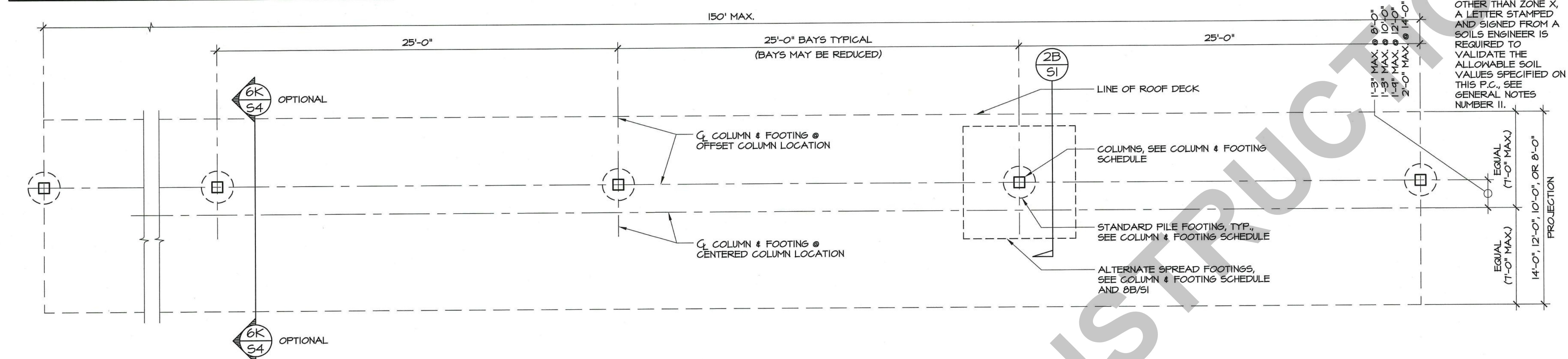
- NOTES:
1. ALTERNATE SPREAD FOOTINGS ARE OPTIONAL FOOTINGS @ AN ADDITIONAL COST.
 2. NO SNOW LOAD ALLOWED W/ 3 P.S.F. FIRE SPRINKLER DEAD LOAD.
 3. COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 3'-6" MAX. FROM BOTTOM, SEE SECTION 6K/S3.
 4. PROVIDE 8-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.
 5. PROVIDE 10-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.
 6. IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C., SEE GENERAL NOTES NUMBER 11.



PLAN @ UPPER ROOF WHERE STEP OCCURS



PLAN @ LOWER ROOF WHERE STEP OCCURS



FOUNDATION PLAN

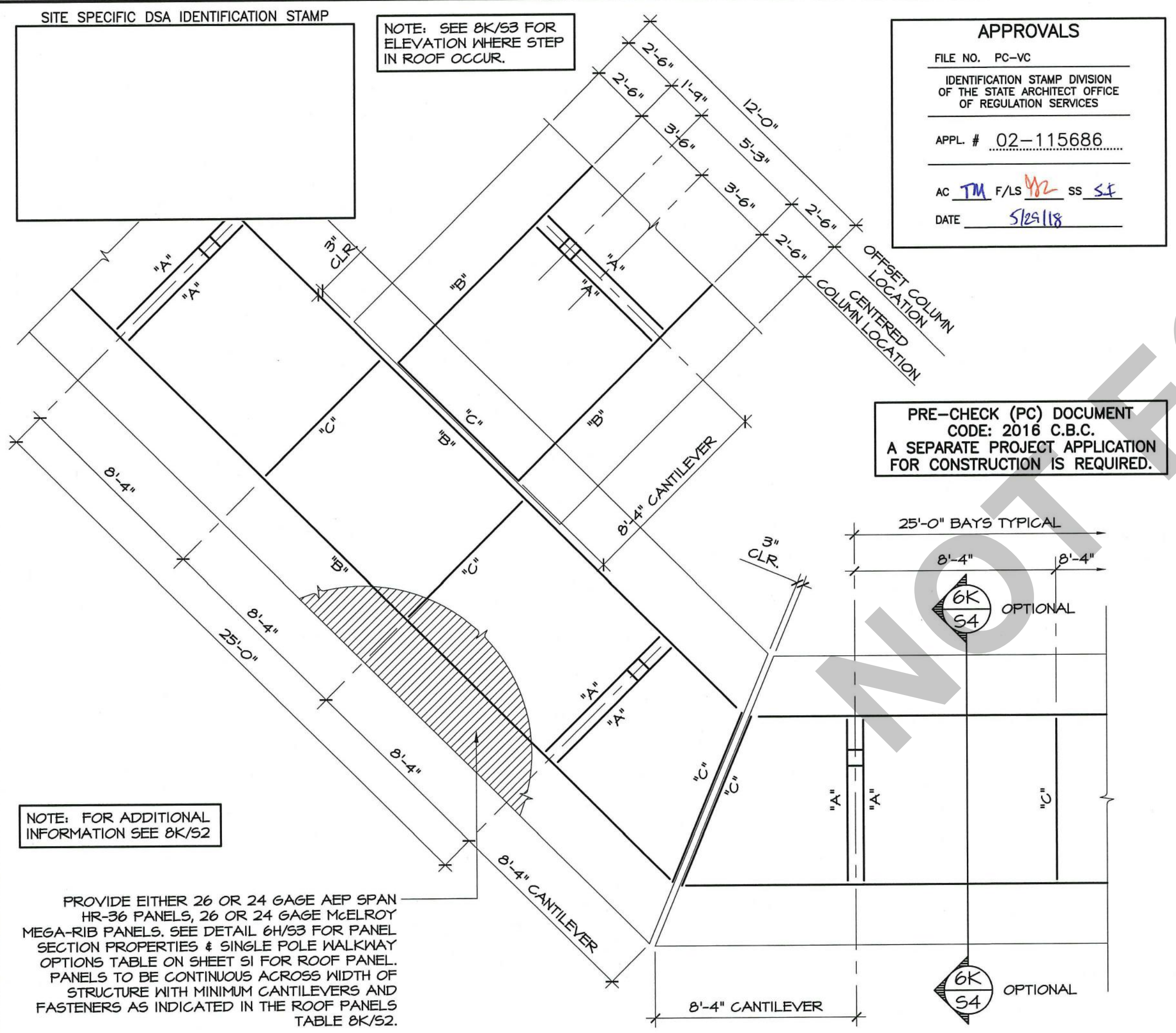
SCALE 1/4" = 1'-0"

4H

ROOF FRAMING PLAN

SCALE 1/4" = 1'-0"

4K



ROOF FRAMING PLAN @ 45° OPTION

SCALE 1/4" = 1'-0"

8B

ROOF FRAMING PLANS

ROOF PANEL TABLE (1, 3, 4, & 6)					
ROOF PITCH	LOAD	MINIMUM REQUIRED CANTILEVER LENGTHS @ 8'-0" CENTER SPAN (3)	MINIMUM REQUIRED CANTILEVER LENGTHS @ 7'-0" CENTER SPAN (3)	MINIMUM REQUIRED CANTILEVER LENGTHS @ 8'-0" CENTER SPAN (3 & 5)	SCREWS PER PANEL PER SUPPORT
2:12 MAX.	20psf LL/SL 30psf SL	NO CANTILEVER REQUIRED	1'-0" MIN. 1'-6" MIN.	0'-6" MIN. 1'-6" MIN.	(4)- #12-14x1" LONG W/ 1/2"Ø THINSEAL WASHERS
4:12 MAX.	20psf LL/SL 30psf SL	0'-8" MIN.	2'-0" MIN.	1'-4" MIN. 2'-0" MIN.	(5)- #12-14x1" LONG W/ 1/2"Ø THINSEAL WASHERS

1. SEE 8K/S1 FOR ROOF PANEL OPTIONS AND 6H/S3 FOR PROFILES & PROPERTIES.

2. 1/4 IN 12 MINIMUM.

3. WHEN NOT USING STANDARD LAYOUTS SHOWN ON ROOF FRAMING PLAN BELOW, THE ROOF PANELS MUST MEET THE MINIMUM REQUIRED CANTILEVER LENGTHS.

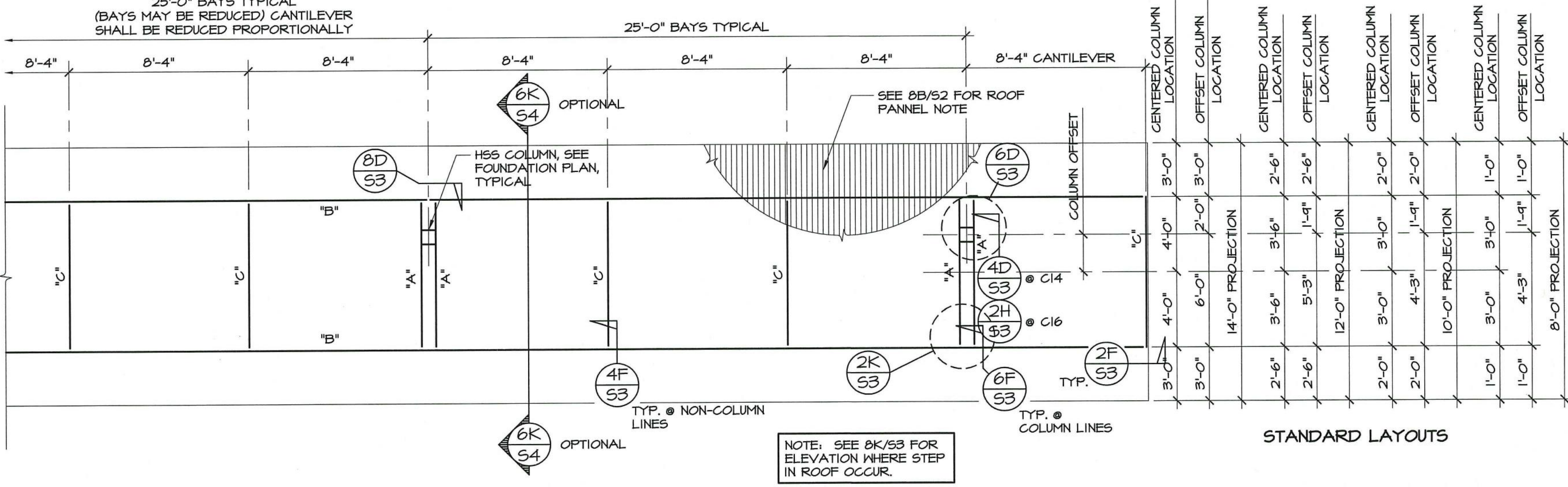
4. LENGTHS OF CANTILEVERS MUST BE EQUAL EACH SIDE, TYPICAL.

5. MUST USE 24 GAGE ROOF PANEL OPTIONS @ 8'-0" CENTER SPAN LAYOUT OPTION.

6. MAXIMUM ALLOWABLE CANTILEVER AT 24 GAGE ROOF PANELS IS 3'-0". IN NO CASE SHALL THE ROOF PANEL LENGTH EXCEED THE 14'-0" MAXIMUM ROOF PROJECTION.

25'-0" BAYS TYPICAL (BAYS MAY BE REDUCED) CANTILEVER SHALL BE REDUCED PROPORTIONALLY

BEAM TABLE						
PITCH	PROJECTION	LOADING	COLUMN OFFSET	CROSS BEAM "A"	SIDE BEAM "B"	BLOCKING & END CLOSURES "C"
1/4:12 MIN. 2:12 MAX.	$\leq 12'-0"$	20psf LL/SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C14x2 1/2x12 GAGE	20 GAGE CEE W 1 1/2" FLANGES, DEPTH TO MATCH SIDE BEAM DEPTH, SEE 2F & 4F/S3
		30psf SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	
	$>12'-0"$, $\leq 14'-0"$	20psf LL/SL	1'-0" TO 2'-0"	C16x3 1/2x12 GAGE	C16x3 1/2x12 GAGE	
		30psf SL	0'-0" TO 0'-10"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	
4:12 MAX.	$\leq 12'-0"$	20psf LL/SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	20 GAGE CEE W 1 1/2" FLANGES, DEPTH TO MATCH SIDE BEAM DEPTH, SEE 2F & 4F/S3
		30psf SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	
	$>12'-0"$, $\leq 14'-0"$	20psf LL/SL	1'-4" TO 2'-0"	C16x3 1/2x12 GAGE	C16x3 1/2x12 GAGE	
		30psf SL	0'-0" TO 1'-2"	C16x3 1/2x12 GAGE	C16x3 1/2x12 GAGE	



ROOF FRAMING PLANS

SCALE 1/4" = 1'-0"

8K

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Email: ghomes@gmail.com

Grand Homer and Associates
STRUCTURAL ENGINEERS

SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS

NO.	DATE	BY	DESCRIPTION
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			

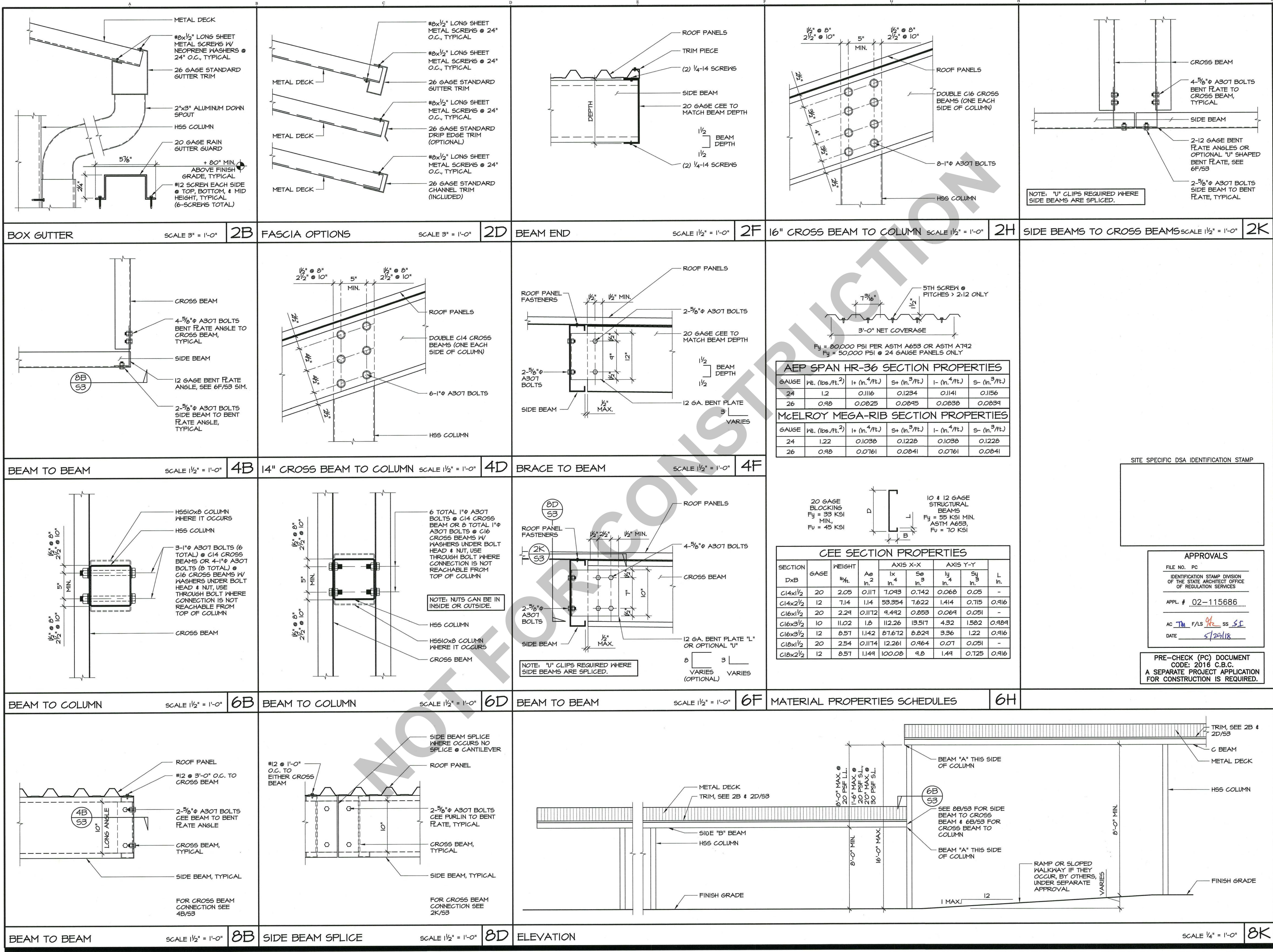
DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE
ROOF FRAMING PLANS

SHEET NUMBER
S2
OF 4 SHEETS



PROJECT: SINGLE POST WALKWAY COVER VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE

SECTION
TYPICAL ELEVATION
DETAILS

SHEET NUMBER

S3

OF 4 SHEETS

Grand
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Phone (559) 329-8830
Fax (559) 329-8807

CA Lic. 981366

LABOR AND / OR MATERIALS WARRANTY/GUARANTEE

Warranty for that certain project known as <School Name>, located at: <School Address>, installed for:

<School District>

We hereby warrant that the labor and / or materials which we have provided for the above project have been completed in accordance with the requirements of specifications sections for lunch shelters and walkway covers, and the Contract Documents.

We agree to repair or replace any or all of our work, together with any other adjacent work which may be displaced by so doing, that may prove to be defective in its workmanship or material, within a period of one year from the date of final acceptance by Owner or from the Date of Certificate of Occupancy, whichever is the earlier.

We also agree to repair any and all damages resulting from such defects, all without additional expense to the Owner. Ordinary wear and tear and unusual abuse or neglect accepted.

In the event of our failure to comply with the above mentioned conditions within 30 days after being notified in writing by the Owner, we collectively or separately do hereby authorized the Owner to proceed to have such defective work repaired or replaced and made good at our expense, and we will honor and pay the costs and charges therefrom upon demand.

Signed: _____ Date: <Date>
(Name/Title) Michael Messerschmidt (Owner)

Insurance Company:
Street Address:
City/State/Zip:

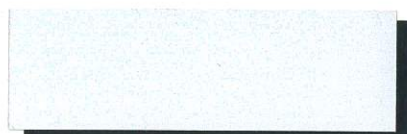
For maintenance, repair, or replacement service, contact:
Valley School Shelters
PO BOX 177
Tulare, CA 93275
(559) 329-8830

Contact person: Michael Messerschmidt

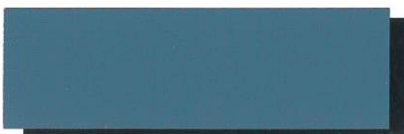


Valley School Shelters
P.O. Box 1499
Tulare, CA 93275-1499
559-329-8830
www.valleyschoolshelters.com

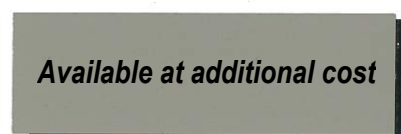
M
COLORS BY
McELROY METAL



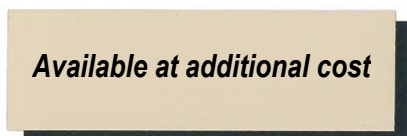
SP / REGAL WHITE



ROMAN BLUE



CLAY



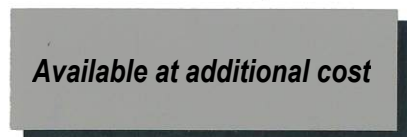
IVORY



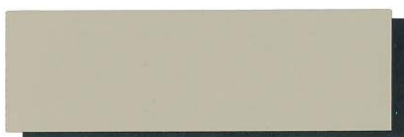
SURREY BEIGE



PATRICIAN BRONZE



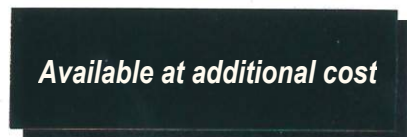
ASH GRAY



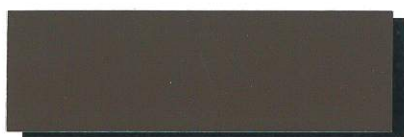
LIGHT STONE



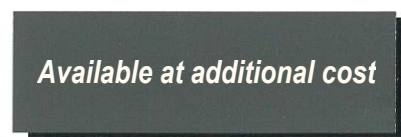
AUTUMN RED



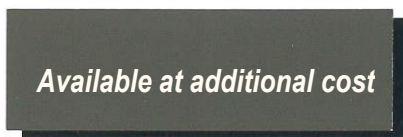
MATTE BLACK



TUDOR BROWN



CHARCOAL



TERRATONE
KYNAR 500® ONLY



EVERGREEN



BRANDYWINE



HARTFORD GREEN

- ADDITIONAL COST WILL APPLY FOR BRANDYWINE AND HARTFORD GREEN.
- AVAILABLE IN KYNAR 500® ONLY.

COLOR SELECTOR



ALL KYNAR 500® COLORS ARE ENERGY STAR COMPLIANT.

MM210CA



SUBMITTAL

For APPROVED EQUAL -
Noble ES - Berryessa USD



Valley School Shelters & Covered Walkways
Submitted by:



1555 Tahoe Court
Redding . California . 96003

Toll Free: 877-473-7619
Facsimile: 530-246-0518

TOTAL 'E' OCCUPANCY BUILDINGS WITH OCCUPANTS = 18,912
18,912 SF @ 20 SF/OCC = 945 OCCUPANTS
TOTAL 'B' OCCUPANCY BUILDINGS WITH OCCUPANTS = 9,028
9,028 SF @ 100 SF/OCC = 90 OCCUPANTS
TOTAL 'F-' OCCUPANCY BUILDINGS WITH OCCUPANTS = 5,154
5,154 SF @ 300 SF/OCC = 17 OCCUPANTS

TOTAL OCCUPANTS = 945 + 90 + 17 = 1,052
MINIMUM DISPERSAL AREA REQUIRED: OCCUPANTS x 5 SF/OCC
1,052 x 5 SF = 5,260 SF
AREA PROVIDED = 5,600 SF THEREFORE OK.

NOBLE ELEMENTARY SCHOOL

DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE STATEMENT:

THE PATH OF TRAVEL IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS IS COMPLIANT WITH THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENT FOR ALTERATIONS, ADDITIONS AND STRUCTURAL REPAIRS. AS PART OF THE DESIGN OF THIS PROJECT, THE P.O.T. WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WERE DETERMINED TO BE NONCOMPLIANT 1) HAVE BEEN IDENTIFIED AND 2) THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS, AND SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS. ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WILL NOT BE CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARDSHIP ARE SO INDICATED IN THESE CONSTRUCTION DOCUMENTS.

DURING CONSTRUCTION, IF P.O.T. ITEMS WITHIN THE SCOPE OF THIS PROJECT REPRESENTED AS CODE COMPLIANT ARE FOUND TO BE NONCONFORMING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THEY SHALL BE BROUGHT INTO COMPLIANCE WITH THE CBC AS A PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

PROJECT SUMMARY

INSTALLATION OF (1) NEW METAL SHADE STRUCTURE PC #04-117117 AND ASSOCIATED SITE WORK.

GENERAL NOTES

- THIS SHEET IS FOR ACCESS COMPLIANCE CODE RELATED ITEMS. FOR SCOPE OF WORK SEE SHEETS A0.1 AND A0.2.
- REFER TO P.C. DRAWINGS FOR EXTENT OF P.C. WORK.
- ACCESSIBLE PATH OF TRAVEL (P.O.T.), AS INDICATED, IS A COMMON BARRIER FREE ACCESS ROUTE WITHOUT ANY ABRUPT VERTICAL CHANGES EXCEEDING A 1/2" BEVELED AT 1:2 MAXIMUM SLOPE, EXCEPT THAT LEVEL CHANGES DO NOT EXCEED 1/4" VERTICAL AND AT LEAST 48" WIDE. THE PATH SURFACE IS SLIP RESISTANT, STABLE, FIRM, AND SMOOTH. PASSING SPACES (11B-403.3) AT LEAST 60" WIDE ARE LOCATED NOT MORE THAN 200' APART. PARTS OF P.O.T. WITH CONTINUOUS GRADIENTS HAVE 60" LEVEL AREAS (11B-403.7) NOT MORE THAN 400' APART. THE CROSS SLOPE DOES NOT EXCEED 2% AND SLOPE IN THE DIRECTION OF TRAVEL AND IS LESS THAN 8% UNLESS OTHERWISE INDICATED. P.O.T. SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM (11B-307.4) AND PROTRUDING OBJECTS GREATER THAN 4" PROJECTION FROM WALL AND ABOVE 27" AND LESS THAN 80" (11B-307.2).
- GATES IN THE PATH OF TRAVEL SHALL MEET DOOR REQUIREMENTS PER CBC SECTION 11B-404. ALL GATES TO HAVE ACCESSIBLE HARDWARE AND 10" MIN. SMOOTH BOTTOM OR KICK PLATES. PANIC HARDWARE AND EXIT SIGN MAY BE REQUIRED. COORDINATE WITH FIRE AND LIFE SAFETY.
- CONTRACTOR TO VERIFY ALL BARRIERS IN P.O.T. HAVE BEEN REMOVED.
- ALL EXTERIOR ENTRANCES AND EXITS IDENTIFIED WITH A TRIANGULAR SYMBOL ON THIS PLAN ARE ACCESSIBLE AND COMPLY WITH CBC 11B-401 AND INCLUDE A 32" CLEAR OPENING, THE REQUIRED STRIKE EDGE CLEARANCE AT PULL SIDE OF DOOR, LEVEL LANDINGS WITH A 2% MAX. SLOPE, AND AN ACCESSIBLE THRESHOLD, HARDWARE, CLOSER AND KICK PLATE.
- A DSA CERTIFIED PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE DSA SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
- GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.

SITE PLAN - FIRE LIFE SAFETY & ACCESS COMPLIANCE NOTES

- EXISTING FIRE HYDRANT.
- EXISTING TOW AWAY SIGN PER DSA #01-115309
- (E) DA PARKING STALLS PER DSA #01-115309
- (E) DA PARKING SIGN PER DSA #01-115309
- (E) ACCESSIBLE DRINKING FOUNTAIN PER DSA #01-115309
- (E) ACCESSIBLE BOYS RESTROOMS PER DSA #01-115309, SEE REFERENCE DRAWINGS
- (E) ACCESSIBLE GIRLS RESTROOMS PER DSA #01-115309, SEE REFERENCE DRAWINGS
- (E) ACCESSIBLE MEN'S RESTROOMS PER DSA #01-115309, SEE REFERENCE DRAWINGS
- (E) ACCESSIBLE WOMEN'S RESTROOMS PER DSA #01-115309, SEE REFERENCE DRAWINGS
- (N) METAL SHADE STRUCTURE PC #04-117117, SEE MANUFACTURER'S DRAWINGS.



810

FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

To facilitate the Division of the State Architect's (DSA) fire and life safety plan review of project site conditions, DSA requires the design professional to provide the following information at time of project submittal for projects consisting of construction of a new campus, construction of new building(s), additions to existing buildings, and for site alternate design means for fire department emergency vehicle access, and fire suppression water supply.

Information associated with compliance items 1-3 below is to be provided for all project types indicated above. Information associated with items 4-7 is to be completed when an alternate means is utilized. Acknowledgement by the school district and signature from the local fire authority (LFA) is only required when an alternate design means is being requested.

Page 1 of the completed form must be imaged onto the fire access site plan. When an alternate design means is proposed, completed pages 1 and 2 are to be imaged on the fire access site plan.

For additional information refer to the instructions at the end of this form and DSA Policy 09-01.

PROJECT INFORMATION			
School District/Owner: BERRYESSA UNION SCHOOL DISTRICT			
Project Name/School: NOBLE ELEMENTARY SCHOOL SHADE STRUCTURES			
Project Address: 3466 GROSSMONT AVE., SAN JOSE, CA 95132			
FIRE & LIFE SAFETY INFORMATION			
1. Has a fire hydrant flow test been performed within the past 12 months? (If yes, provide a copy of the test data.)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>	
2. Was the fire hydrant water flow test performed as part of this LFA review? (If yes, indicate fire hazard severity zone as established by Cal Fire? (If yes, indicate fire hazard zone classification below))	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>	
Refer to the following for fire hazard zone locations: www.fire.ca.gov/fire_prevention/fire_prevention_wildland_zones_maps			Moderate <input type="checkbox"/> High <input type="checkbox"/> Very High <input type="checkbox"/>
Wildland Interface Area (WIFA) (If any designations are checked, project design must meet the requirements of CBC Chapter 7A.)			WIFA <input type="checkbox"/>
CONDITION MEANS AND METHODS RESOLUTION		ALTERNATE ACCEPTED	
	Yes	No	N/A / N/R
4. Emergency vehicle access roadways do not meet CFC requirements.			<input checked="" type="checkbox"/>
4a. Acceptable Alternate: Emergency vehicle and personnel access as proposed by the project architect is acceptable for providing fire suppression and protection of life and property.			
5. Fire Hydrants: Number and spacing does not meet CFC requirements.			<input checked="" type="checkbox"/>
5a. Acceptable Alternate: Number of fire hydrants and spacing as proposed by the project architect is acceptable for fire suppression and protection of life and property.			
6. Fire Hydrants: Water flow and pressure are less than CFC minimum.			<input checked="" type="checkbox"/>
6a. Acceptable Alternate: The available flow and pressure is acceptable for providing fire suppression and protection of life and property.			
7. Location of fire department connection(s) serving fire sprinkler systems or standpipe systems does not meet CFC requirements.			<input checked="" type="checkbox"/>
7a. Acceptable Alternate: The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable for providing fire suppression and protection of life and property.			

DSA 810 (rev 10-22-18) DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA Page 1 of 4

GRAPHIC KEY

- EXISTING PROPERTY LINE
- ASSUMED PROPERTY LINE
- ACCESSIBLE PATH OF TRAVEL
- ROOF OVERHANG
- CHAIN LINK FENCE
- WOOD FENCE
- DECORATIVE FENCE
- FIRE DEPARTMENT ACCESS
- (E) DRY STAND PIPE
- (E) FIRE HYDRANT
- DRINKING FOUNTAIN
- (E) SIGN
- NEW SHADE STRUCTURE
- EXISTING BUILDING

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01-118971 INC.
REVIEWED FOR
DATE: 06/10/2020



SITE PLAN FIRE LIFE SAFETY & ACCESS COMPLIANCE

SHADE STRUCTURE
NOBLE ELEMENTARY SCHOOL
3466 GROSSMONT DR., SAN JOSE, CA 95132
BERRYESSA UNION SCHOOL DISTRICT

REVISIONS	NO.	ITEM	DATE
DRAWN BY:		MK	
CHECKED BY:		NJ	
SFA JOB NO:		DATE:	
19065		06/17/2019	

T2

BUILDING CODE ANALYSIS

BUILDING	CONSTRUCTION TYPE OCCUPANCY TYPE	AREA (SQ.FT.)	ALLOWABLE (SQ.FT.)	# OF STORIES
CLASSROOM BUILDING A	V-B / E	5,202	9,500	1
CLASSROOM BUILDING B	V-B / E	6,707	9,500	1
CLASSROOM BUILDING C	V-B / E	5,372	9,500	1
CLASSROOM BUILDING D	V-B / E	7,246	9,500	1
CLASSROOM BUILDING E	V-B / E	4,430	9,500	1
MULTIPURPOSE BLDG. F	V-1 / E / A2	6,553	11,288	1
(N) SHADE STRUCTURE	IIB/A3	1,920	9,500	1

1 SITE PLAN - FIRE LIFE SAFETY & ACCESS COMPLIANCE

OCCUPANT LOAD ANALYSIS:

PER CBC 2019 SECTION 1004 TABLE 1004.5
MULTIPURPOSE BLDG. F: (E) EDUCATIONAL CLASSROOM AREA: 2,183 SQ. FT. @ 119 = 18.3; (A2) ASSEMBLY WITHOUT FIXED SEATS: 4,370 SQ. FT. @ 874 = 5.0; TOTAL 18.3 + 5.0 = 23.3. SEE REFERENCE DRAWINGS DSA # 01-115309 FOR SPRINKLER SYSTEM INSTALLED.

(N) SHADE STRUCTURE: (A3) ASSEMBLY UNCONCENTRATED: 1,920 SQ. FT. @ 115 = 16.7; SPRINKLERS NOT REQUIRED.

PARKING COUNT

PER 2019 CBC, TABLE 11B-208.2

(E) PARKING LOT

TOTAL PARKING SPACES (INCLUDING ALL ACCESSIBLE PARKING SPACES) = 73
MINIMUM ACCESSIBLE PARKING SPACES REQUIRED = 3
TOTAL STANDARD ACCESSIBLE SPACES + TOTAL VAN ACCESSIBLE SPACES* PROVIDED = 1 + 3 = 4 THEREFORE, OKAY.

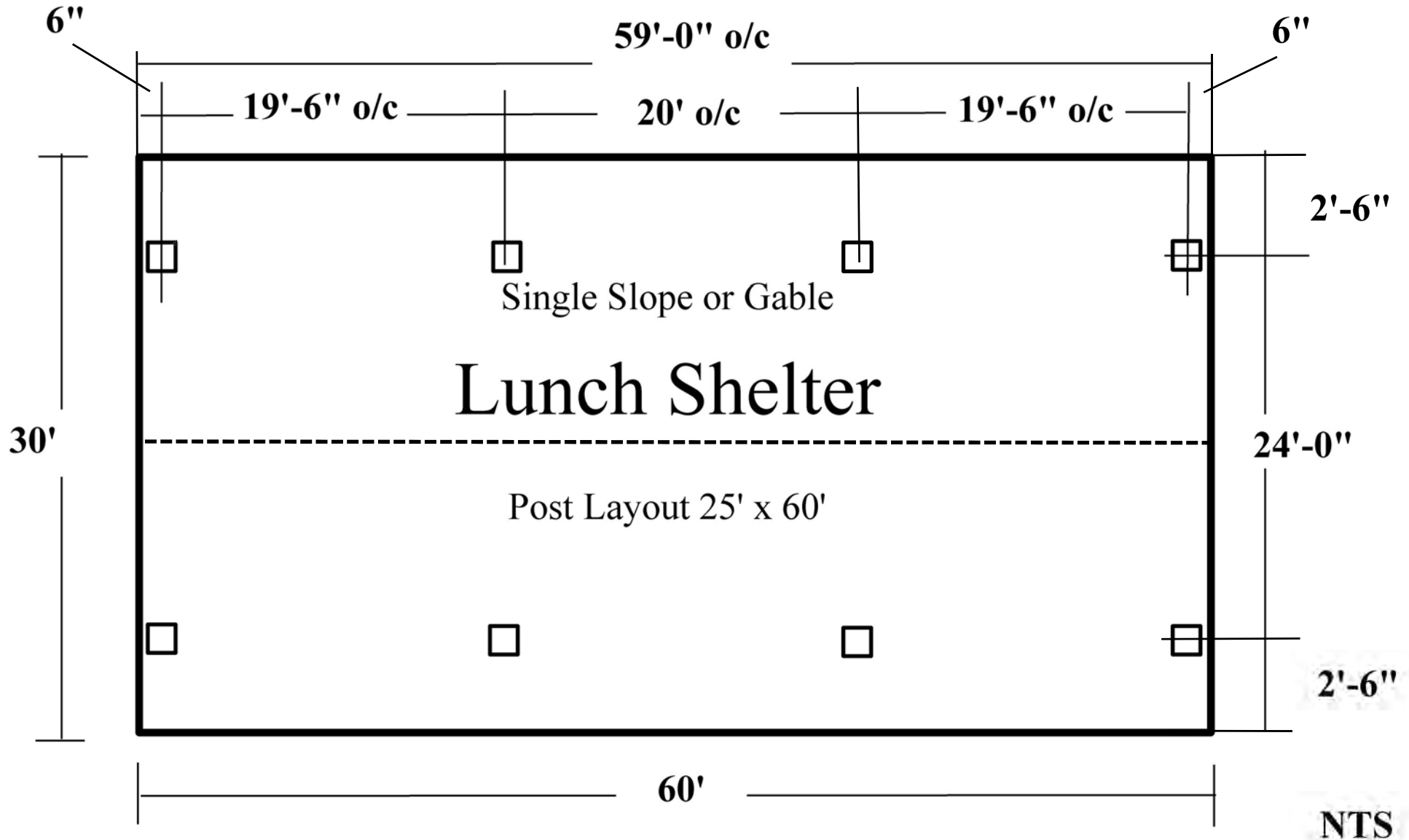
* FOR EVERY SIX STANDARD ACCESSIBLE SPACES REQUIRED, AT LEAST ONE SHALL BE A VAN PARKING SPACE.

School Lunch Shelters

PO Box 177 - Tulare, CA 93275 - Telephone (559) 329-8830

30' x 60' with 8 posts

Noble ES Structure 'A'

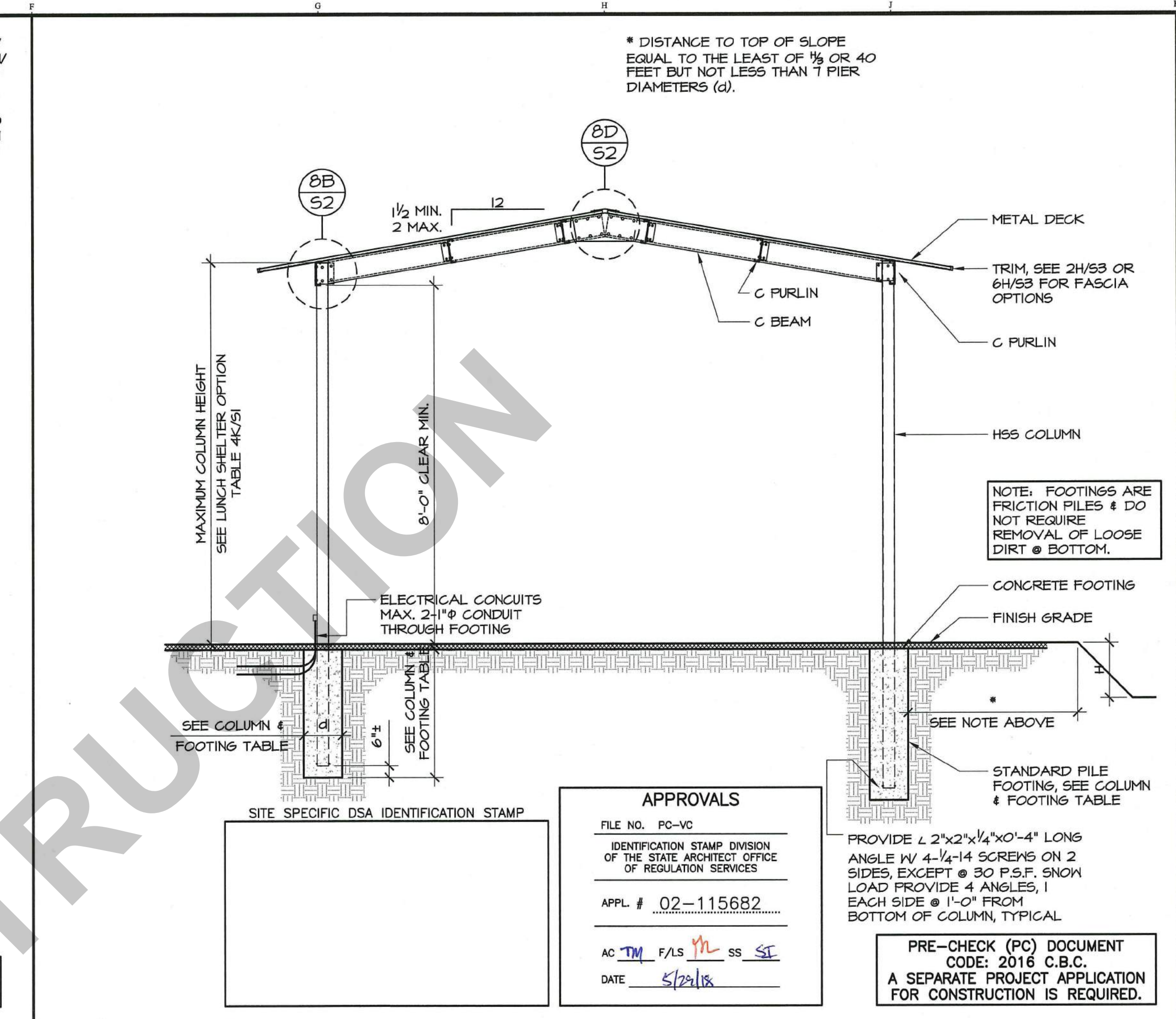


Gable Lunch Shelter Cover DSA Pre-Checked Plans NO STAMP

Stamped plans to be provided if approved equal is approved.

Gable Lunch Shelter Covers Used On:

- **Noble Elementary Structure 'A'**



COLUMN & FOOTING TABLE

LOADING	LOWER SEISMIC ($S_{DS} \leq 1.0$)			HIGHER SEISMIC ($S_{DS} \leq 1.75 \text{ MAX.}$)		
	COLUMN	PILE FOOTING	(1) ALTERNATE SPREAD FOOTING	COLUMN	PILE FOOTING	(1) ALTERNATE SPREAD FOOTING
20 PSF LL	H556x6x ³ / ₁₆	1'-6"φx7'-10"	7'-0" SQ.x2'-4" (6 & 8)	H556x6x ³ / ₁₆	1'-6"φx8'-6"	7'-0" SQ.x2'-4" (6 & 8)
20 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx8'-10"		H556x6x ³ / ₁₆ (4)	1'-6"φx8'-10"	
30 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx10'-9"		H556x6x ³ / ₁₆ (4)	1'-6"φx10'-9"	
20 PSF LL	H556x6x ³ / ₁₆	1'-6"φx8'-0"	7'-0" SQ.x2'-4" (6 & 8)	H556x6x ³ / ₁₆ (4)	1'-6"φx8'-10"	7'-0" SQ.x2'-4" (6 & 8)
20 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx8'-10"		H556x6x ³ / ₁₆ (4)	1'-6"φx8'-10"	
30 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx10'-9"		H556x6x ³ / ₁₆ (4)	1'-6"φx10'-9"	
20 PSF LL	H556x6x ³ / ₁₆	1'-6"φx8'-3"	7'-0" SQ.x2'-4" (6 & 8)	H556x6x ³ / ₁₆ (4)	1'-6"φx9'-0"	7'-0" SQ.x2'-4" (6 & 8)
20 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx8'-10"		H556x6x ³ / ₁₆ (4)	1'-6"φx9'-0"	
30 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx10'-9"		H556x6x ³ / ₁₆ (4)	1'-6"φx10'-9"	

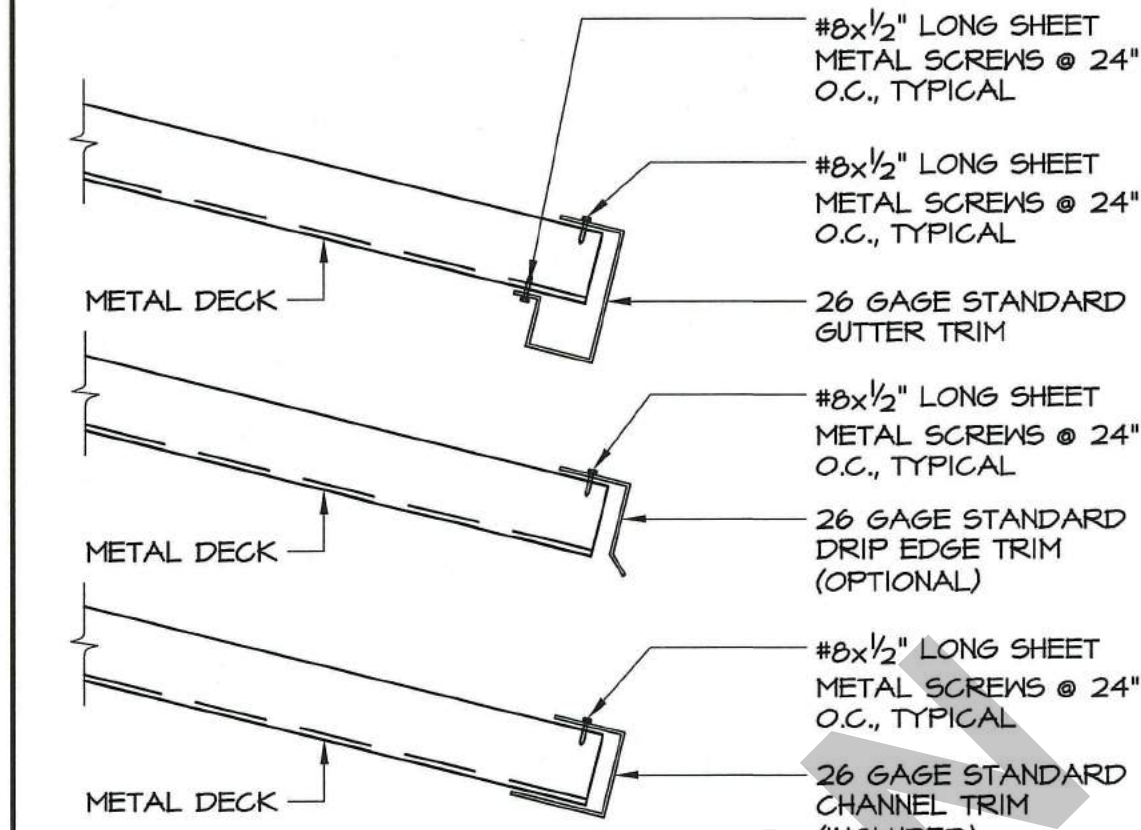
LOADING	LOWER SEISMIC ($S_{DS} \leq 1.0$)			HIGHER SEISMIC ($S_{DS} \leq 2.5 \text{ MAX.}$)		
	COLUMN	PILE FOOTING	(1) ALTERNATE SPREAD FOOTING	COLUMN	PILE FOOTING	(1) ALTERNATE SPREAD FOOTING
20 PSF LL	H556x6x ³ / ₁₆	1'-6"φx8'-10"	7'-6" SQ.x2'-4" (6 & 8)	H556x6x ³ / ₁₆	1'-6"φx10'-8"	8'-0" SQ.x2'-4" (7 & 8)
20 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx9'-6"		H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"	
30 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"		H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"	
20 PSF LL	H556x6x ³ / ₁₆	1'-6"φx8'-10"	7'-6" SQ.x2'-4" (6 & 8)	H556x6x ³ / ₁₆ (4)	1'-6"φx11'-0"	9'-0" SQ.x2'-4" (7 & 8)
20 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx9'-6"		H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"	
30 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"		H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"	
20 PSF LL	H556x6x ³ / ₁₆	1'-6"φx8'-10"	7'-6" SQ.x2'-4" (6 & 8)	H556x6x ³ / ₁₆ (4)	1'-6"φx11'-3"	9'-6" SQ.x2'-4" (7 & 8)
20 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx9'-6"		H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"	
30 PSF SL	H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"		H556x6x ³ / ₁₆ (4)	1'-6"φx11'-6"	

NOTES:

- ALTERNATE SPREAD FOOTINGS ARE OPTIONAL FOOTINGS @ AN ADDITIONAL COST.
- ~~SEE SECTION 4K/52 FOR DETAILS.~~
- COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 6" MAX. FROM BOTTOM. SEE SECTION 4K/52.
- AT CORNER COLUMN MAY USE H556x6x³/₁₆.
- AT CORNER COLUMN MAY USE H556x6x³/₁₆.
- USE 8-#5 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 4 1/2" CLEAR FROM BOTTOM OF FOOTING. USE 10-#5 REBARS EACH WAY, TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 4 1/2" CLEAR FROM BOTTOM OF FOOTING.
- USE 4-L 2x2x³/₁₆x3'-6" LONG ANGLES @ BOTTOM OF COLUMN, EACH W/ 5-1/4"-14 TKS SCREWS TO COLUMN. SEE 8H/53.
- IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C., SEE GENERAL NOTES NUMBER 11.

<p>2374 WEST WHITTENDALE VISALIA, CA 93277</p>		<p>(559) 734-6675 FAX (559) 734-5232 Email: ghomerstrudengr@abcglobal.net</p>																									
 <p>Gerard Ghomer and Associates STRUCTURAL ENGINEERS</p>																											
<p>PROJECT: GABLE SCHOOL LUNCH SHELTER VALLEY SCHOOL SHELTERS</p>																											
<table border="1" style="width: 100%; border-collapse: collapse;"><thead><tr><th colspan="4" style="text-align: center;">REVISIONS</th></tr><tr><th style="width: 15%;">NO.</th><th style="width: 15%;">DATE</th><th style="width: 15%;">BY</th><th style="width: 55%;">DESCRIPTION</th></tr></thead><tbody><tr><td> </td><td> </td><td> </td><td> </td></tr><tr><td> </td><td> </td><td> </td><td> </td></tr><tr><td> </td><td> </td><td> </td><td> </td></tr><tr><td> </td><td> </td><td> </td><td> </td></tr></tbody></table>				REVISIONS				NO.	DATE	BY	DESCRIPTION																
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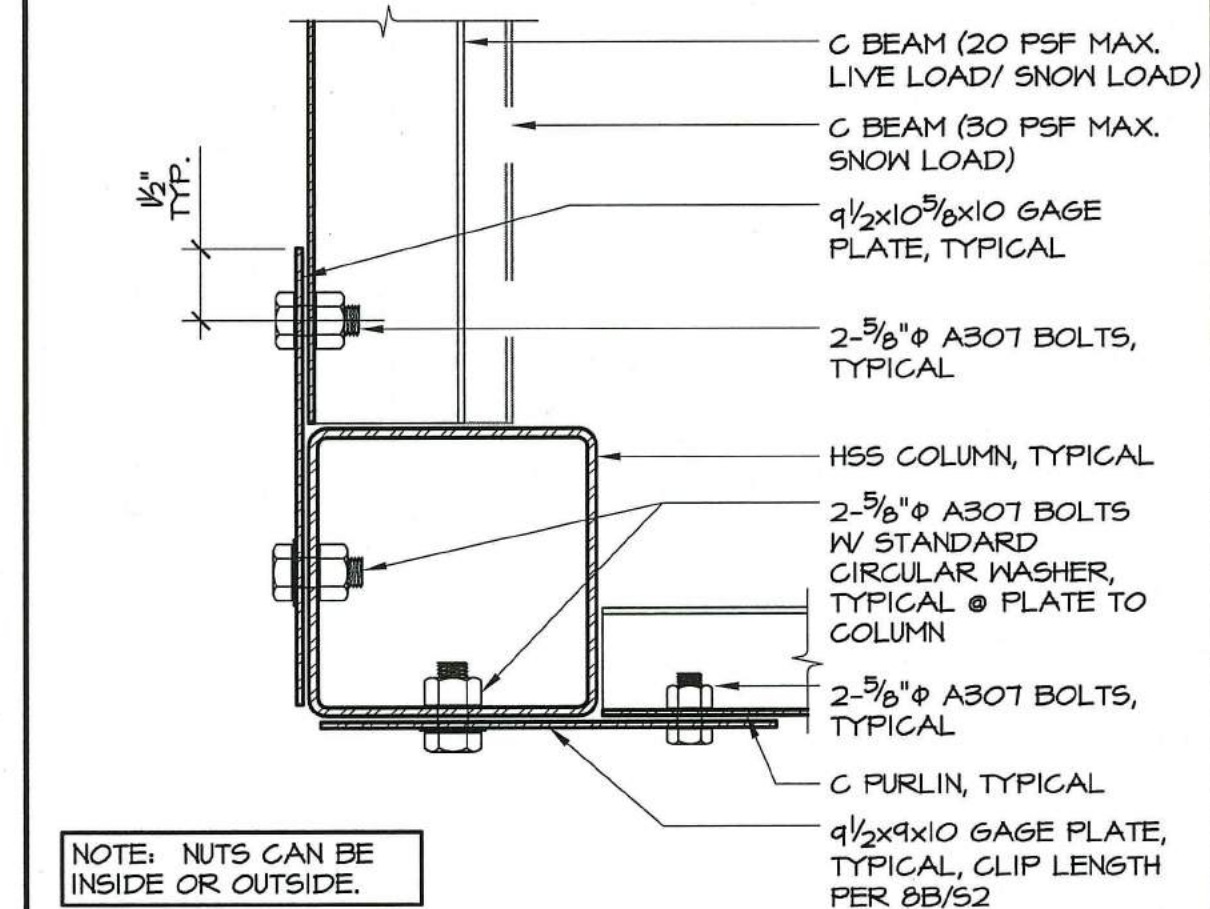
NOT FOR CONSTRUCTION



FASCIA OPTIONS

SCALE 3" = 1'-0"

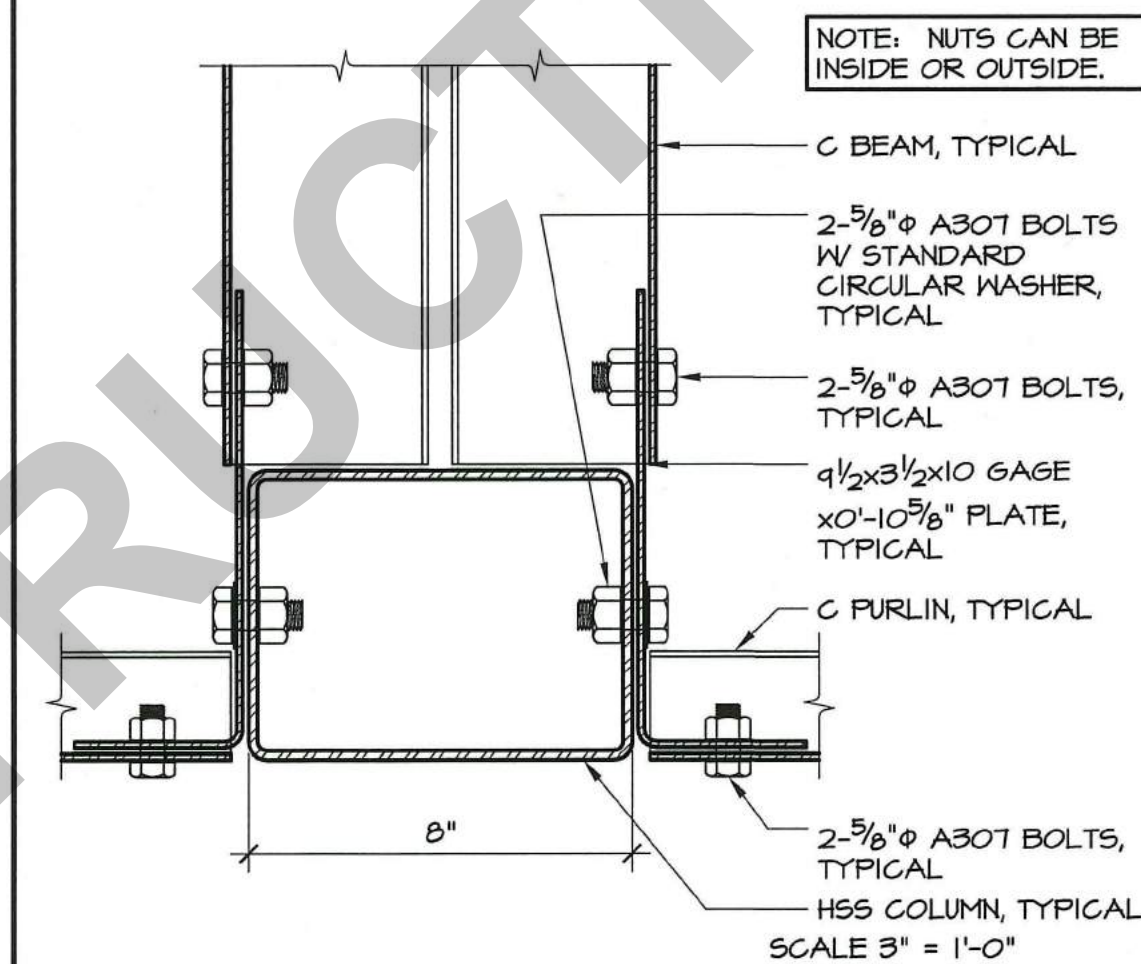
2H



BEAM TO COLUMN @ CORNERS

SCALE 3" = 1'-0"

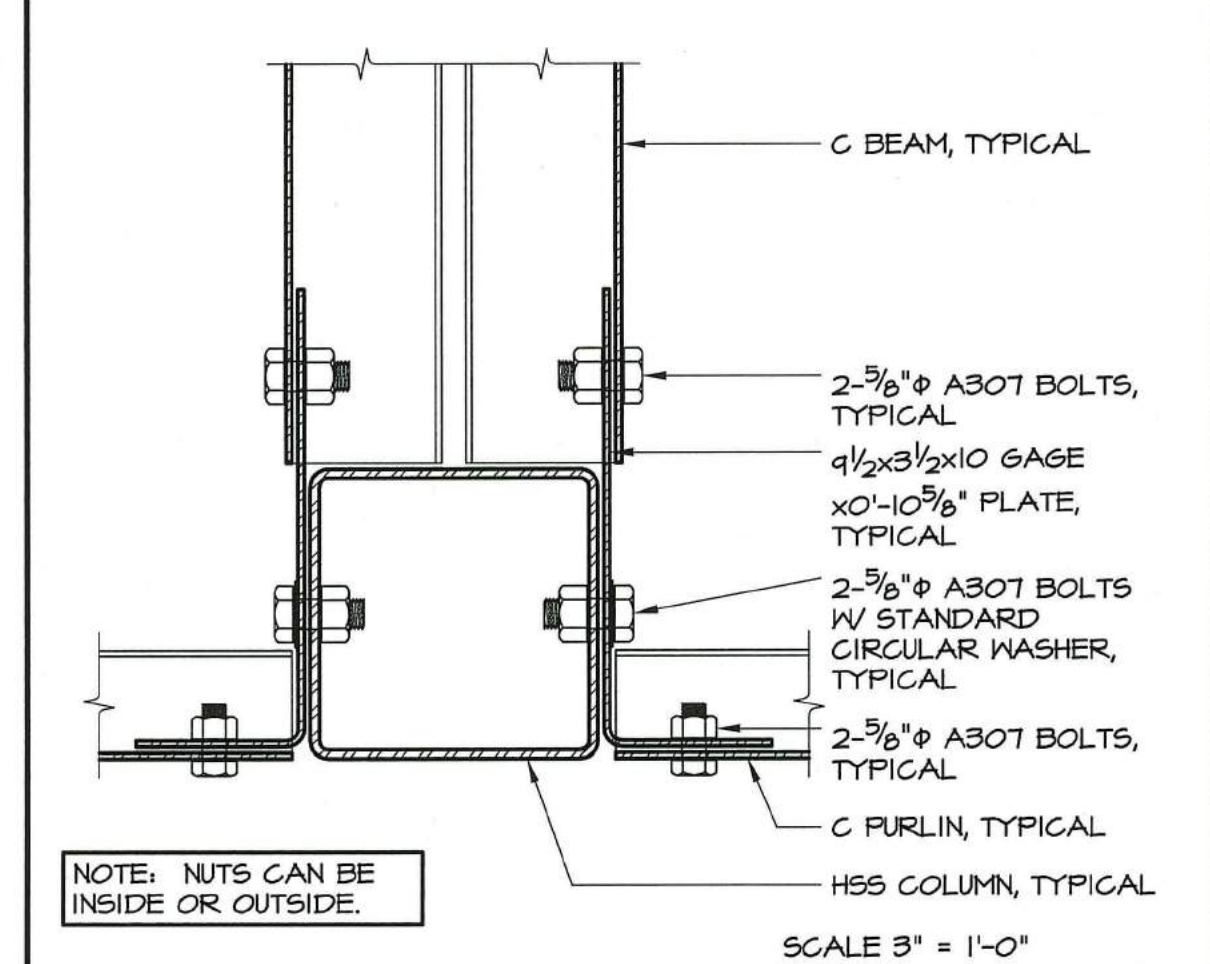
2K



BEAM TO COLUMN @ INTERMEDIATE LOCATIONS

8" COLUMNS

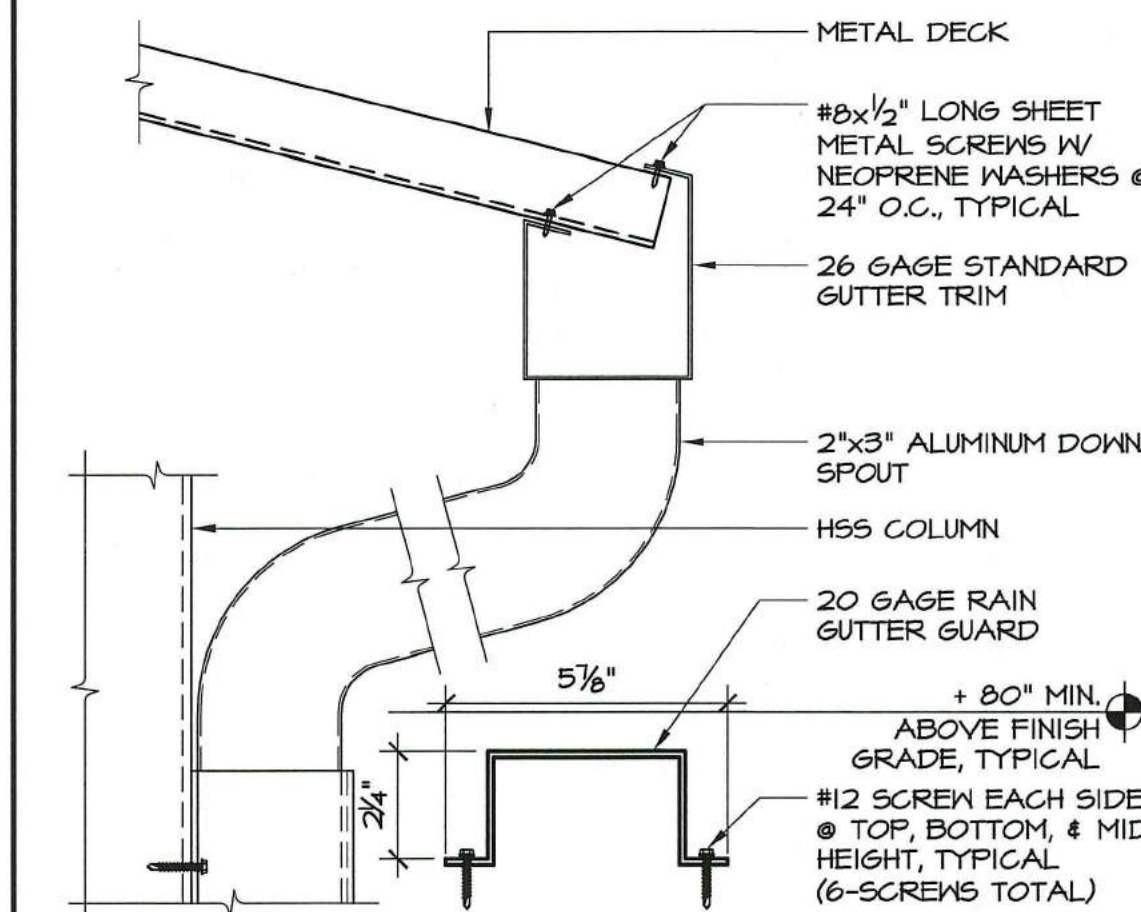
4H



BEAM TO COLUMN @ INTERMEDIATE LOCATIONS

6" COLUMNS

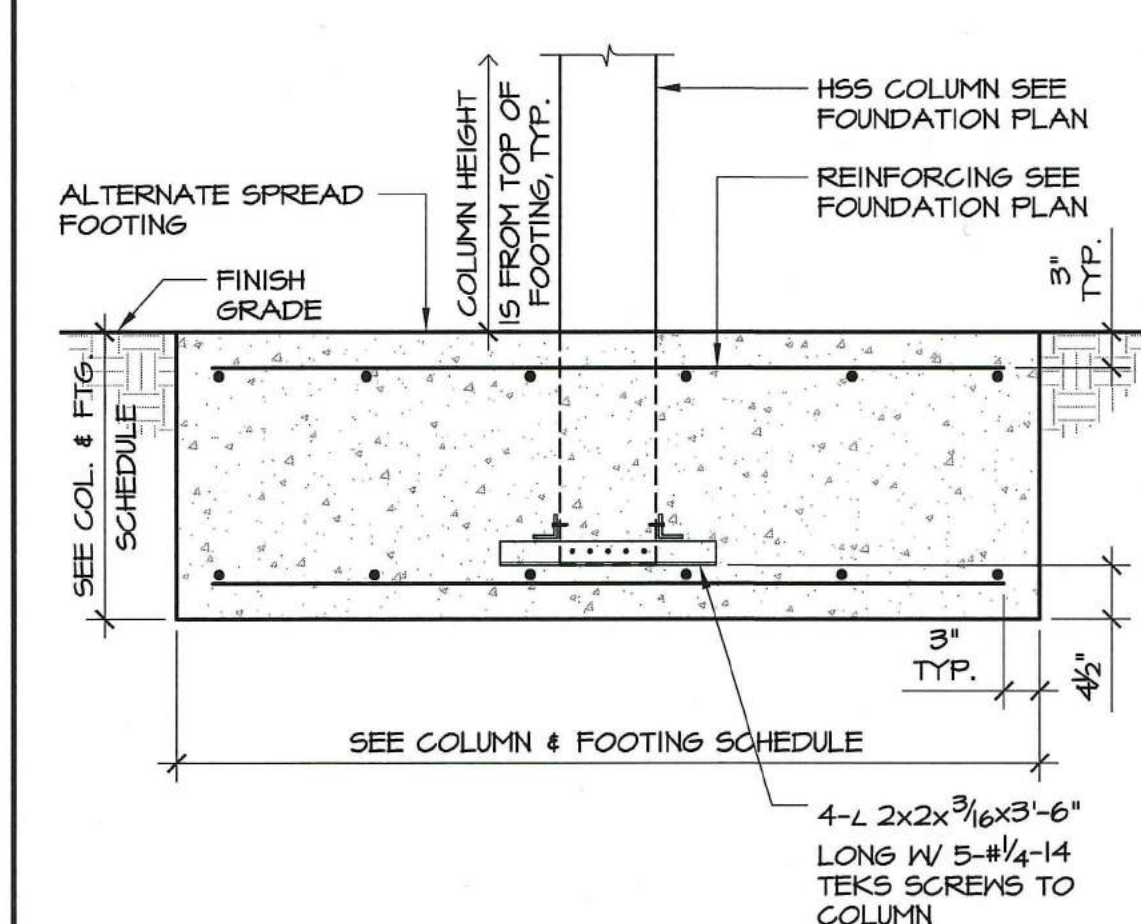
4K



BOX GUTTER

SCALE 3" = 1'-0"

6H



OPTIONAL SPREAD FOOTING @ ADDITIONAL COST

SCALE 3/4" = 1'-0"

8H

BEAM & PURLIN TABLE			
(RISK CATEGORY)	LOADING	BEAM "A"	PURLIN "B"
(RISK CATEGORY I) SEE NOTE 3	20 PSF LL	C12x3x12 GAGE	C12x2 1/4x14 GAGE
	20 PSF SL	C14x3x12 GAGE	C12x2 1/4x14 GAGE
	30 PSF SL	C18x3 1/4x12 GAGE	C12x2 1/4x12 GAGE
(RISK CATEGORY II) SEE NOTE 3	20 PSF LL/SL	C14x3x12 GAGE	C12x2 1/4x12 GAGE
	30 PSF SL	C18x3 1/4x12 GAGE	C12x2 1/4x12 GAGE

- NOTES:
1. RISK CATEGORY IS PER CBC 1604A.5.
 2. SEE DB/51 FOR SECTION PROPERTIES OF BEAMS.
 3. SEE SITE SPECIFIC TABLE TO DETERMINE RISK CATEGORY.

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS			
FILE NO.	PC-VC		
IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES			
APPL. #	02-115682		
AC	TM	F/LS	SS
DATE	5/25/18		

PRE-CHECK (PC) DOCUMENT CODE: 2016 C.B.C. A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

BEAM & PURLIN TABLE

8K

2374 WEST WHITENDALE
VISALIA, CA 93277

Grand
Gomer and Associates
STRUCTURAL ENGINEERS
Email: ghomerstructural@gmail.com

GABLE SCHOOL
LUNCH SHELTER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION

DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16324

DRAWING TITLE

TABLE DETAILS

SHEET NUMBER

S3

OF 3 SHEETS

Phone (559) 329-8830
Fax (559) 329-8807

CA Lic. 981366

LABOR AND / OR MATERIALS WARRANTY/GUARANTEE

Warranty for that certain project known as <School Name>, located at: <School Address>, installed for:

<School District>

We hereby warrant that the labor and / or materials which we have provided for the above project have been completed in accordance with the requirements of specifications sections for lunch shelters and walkway covers, and the Contract Documents.

We agree to repair or replace any or all of our work, together with any other adjacent work which may be displaced by so doing, that may prove to be defective in its workmanship or material, within a period of one year from the date of final acceptance by Owner or from the Date of Certificate of Occupancy, whichever is the earlier.

We also agree to repair any and all damages resulting from such defects, all without additional expense to the Owner. Ordinary wear and tear and unusual abuse or neglect accepted.

In the event of our failure to comply with the above mentioned conditions within 30 days after being notified in writing by the Owner, we collectively or separately do hereby authorized the Owner to proceed to have such defective work repaired or replaced and made good at our expense, and we will honor and pay the costs and charges therefrom upon demand.

Signed: _____ Date: <Date>
(Name/Title) Michael Messerschmidt (Owner)

Insurance Company:
Street Address:
City/State/Zip:

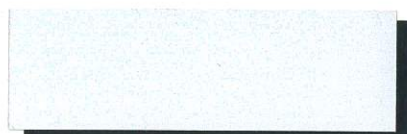
For maintenance, repair, or replacement service, contact:
Valley School Shelters
PO BOX 177
Tulare, CA 93275
(559) 329-8830

Contact person: Michael Messerschmidt

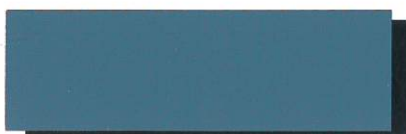


Valley School Shelters
P.O. Box 1499
Tulare, CA 93275-1499
559-329-8830
www.valleyschoolshelters.com

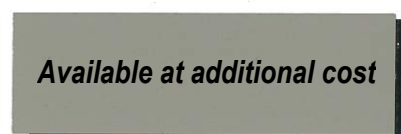
M
COLORS BY
McELROY METAL



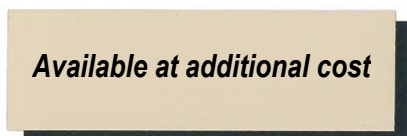
SP / REGAL WHITE



ROMAN BLUE



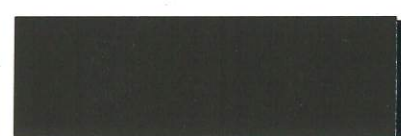
CLAY



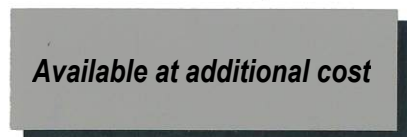
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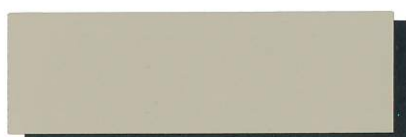
SURREY BEIGE



PATRICIAN BRONZE



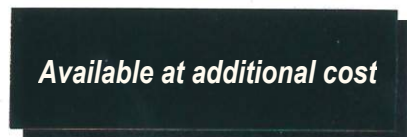
ASH GRAY



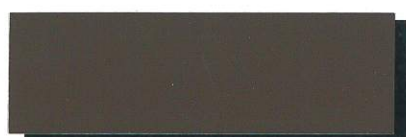
LIGHT STONE



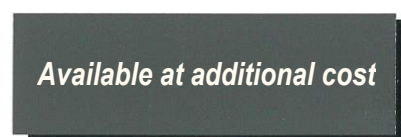
AUTUMN RED



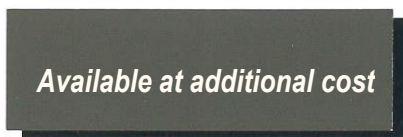
MATTE BLACK



TUDOR BROWN



CHARCOAL



TERRATONE
KYNAR 500® ONLY



EVERGREEN



BRANDYWINE



HARTFORD GREEN

• ADDITIONAL COST WILL APPLY FOR BRANDYWINE AND HARTFORD GREEN.
• AVAILABLE IN KYNAR 500® ONLY.

COLOR SELECTOR



ALL KYNAR 500® COLORS ARE ENERGY STAR COMPLIANT.

MM210CA



SUBMITTAL

For **APPROVED EQUAL -**
Piemont ES - Berryessa USD



Valley School Shelters & Covered Walkways
Submitted by:



1555 Tahoe Court
Redding . California . 96003

Toll Free: 877-473-7619
Facsimile: 530-246-0518

SAFE DISPERSAL AREA

TOTAL 'E' OCCUPANCY BUILDINGS WITH OCCUPANTS = 18,912
18,912 SF @ 20 SF/OCC = 945 OCCUPANTSTOTAL 'B' OCCUPANCY BUILDINGS WITH OCCUPANTS = 9,028
9,028 SF @ 100 SF/OCC = 90 OCCUPANTSTOTAL 'F-1' OCCUPANCY BUILDINGS WITH OCCUPANTS = 5,154
5,154 SF @ 300 SF/OCC = 17 OCCUPANTS

TOTAL OCCUPANTS = 945 + 90 + 17 = 1,052

MINIMUM DISPERSAL AREA REQUIRED: OCCUPANTS x 5 SF/OCC
1,052 x 5 SF = 5,260 SF
AREA PROVIDED = 5,300 SF THEREFORE OK.

PIEDMONT ELEMENTARY SCHOOL

DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE STATEMENT:

THE PATH OF TRAVEL IDENTIFIED IN THESE CONSTRUCTION DOCUMENTS IS COMPLIANT WITH THE CURRENT APPLICABLE CALIFORNIA BUILDING CODE ACCESSIBILITY PROVISIONS FOR PATH OF TRAVEL REQUIREMENT FOR ALTERATIONS. ADDITIONS AND STRUCTURAL REPAIRS, AS PART OF THE DESIGN OF THIS PROJECT, THE P.O.T. WAS EXAMINED AND ANY ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WERE DETERMINED TO BE NONCOMPLIANT 1) HAVE BEEN IDENTIFIED AND 2) THE CORRECTIVE WORK NECESSARY TO BRING THEM INTO COMPLIANCE HAS BEEN INCLUDED WITHIN THE SCOPE OF THIS PROJECT'S WORK THROUGH DETAILS, DRAWINGS, AND SPECIFICATIONS INCORPORATED INTO THESE CONSTRUCTION DOCUMENTS. ANY NONCOMPLIANT ELEMENTS, COMPONENTS OR PORTIONS OF THE P.O.T. THAT WILL NOT BE CORRECTED BY THIS PROJECT BASED ON VALUATION THRESHOLD LIMITATIONS OR A FINDING OF UNREASONABLE HARSHNESS ARE INDICATED IN THESE CONSTRUCTION DOCUMENTS.

DURING CONSTRUCTION, IF P.O.T. ITEMS WITHIN THE SCOPE OF THIS PROJECT REPRESENTED AS CODE COMPLIANT ARE FOUND TO BE NONCOMPLYING BEYOND REASONABLE CONSTRUCTION TOLERANCES, THEY SHALL BE BROUGHT INTO COMPLIANCE WITH THE CBC AS A PART OF THIS PROJECT BY MEANS OF A CONSTRUCTION CHANGE DOCUMENT.

PROJECT SUMMARY

INSTALLATION OF (2) NEW METAL LUNCH SHADE STRUCTURES, (1) ENTRY CANOPY, (1) MARQUEE SIGN, AND ASSOCIATED SITE WORK.

GENERAL NOTES

- A. THIS SHEET IS FOR ACCESS & FIRE LIFE SAFETY COMPLIANCE CODE RELATED ITEMS. FOR SCOPE OF WORK SEE SHEETS A0.1 AND A0.2, A0.3 & A0.4.
- B. REFER TO P.C. DRAWINGS FOR EXTENT OF P.C. WORK.
- C. ACCESSIBLE PATH OF TRAVEL (P.O.T.), AS INDICATED, IS A COMMON BARRIER FREE ACCESS ROUTE WITHOUT ANY ABRUPT VERTICAL CHANGES EXCEEDING A 1/2" BEVELED AT 1:2 MAXIMUM SLOPE, EXCEPT THAT LEVEL CHANGES DO NOT EXCEED 1/4" VERTICAL AND AT LEAST 48" WIDE. THE PATH SURFACE IS SLIP RESISTANT, STABLE, FIRM, AND SMOOTH. PASSING SPACES (11B-403.5.3) AT LEAST 60"x60" ARE LOCATED NOT MORE THAN 200' APART. PARTS OF P.O.T. WITH CONTINUOUS GRADIENTS HAVE 60" LEVEL AREAS (11B-403.7) NOT MORE THAN 400' APART. THE CROSS SLOPE DOES NOT EXCEED 2% AND SLOPE IN THE DIRECTION OF TRAVEL AND IS LESS THAN 5% UNLESS OTHERWISE INDICATED. P.O.T. SHALL BE MAINTAINED FREE OF OVERHANGING OBSTRUCTIONS TO 80" MINIMUM (11B-307.4) AND PROTRUDING OBJECTS GREATER THAN 4" PROJECTION FROM WALL AND ABOVE 27" AND LESS THAN 80" (11B-307.2).
- D. GATES IN THE PATH OF TRAVEL SHALL MEET DOOR REQUIREMENTS PER CBC SECTION 11B-404. ALL GATES TO HAVE ACCESSIBLE HARDWARE AND 10" MIN. SMOOTH BOTTOM OR KICK PLATES. PANIC HARDWARE AND EXIT SIGN MAY BE REQUIRED. COORDINATE WITH FIRE AND LIFE SAFETY.
- E. CONTRACTOR TO VERIFY ALL BARRIERS IN P.O.T. HAVE BEEN REMOVED.
- F. ALL EXTERIOR ENTRANCES AND EXITS IDENTIFIED WITH A TRIANGULAR SYMBOL ON THIS PLAN ARE ACCESSIBLE AND COMPLY WITH CBC 11B-401 AND INCLUDE A 32" CLEAR OPENING, THE REQUIRED STRIKE EDGE CLEARANCE AT PULL SIDE OF DOOR, LEVEL LANDINGS WITH A 2% MAX. SLOPE, AND AN ACCESSIBLE THRESHOLD, HARDWARE, CLOSER AND KICK PLATE.
- G. A 'DSA CERTIFIED' PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE DSA SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342. PART 1, TITLE 24, CBC.
- H. DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
- I. GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.

SITE PLAN - FIRE LIFE SAFETY & ACCESS COMPLIANCE NOTES

- EXISTING FIRE HYDRANT
- (N) PEDESTRIAN ACCESS GATE WITH PANIC HARDWARE. SEE DETAIL 9/A0.5
- EXISTING TOW AHEAD SIGN PER DSA #01-117027
- (E) DA PARKING STALLS PER DSA #01-117160
- (E) DA PARKING SIGN PER DSA #01-117160
- (E) ACCESSIBLE DRINKING FOUNTAIN PER DSA #01-117027
- (E) ACCESSIBLE BOYS RESTROOMS PER DSA #01-117027. SEE REFERENCE DRAWINGS
- (E) ACCESSIBLE GIRLS RESTROOMS PER DSA #01-117027. SEE REFERENCE DRAWINGS
- (N) METAL SHADE STRUCTURE PC #04-117117. SEE MANUFACTURER'S DRAWINGS.
- (N) METAL SHADE STRUCTURE PC #04-117117. SEE MANUFACTURER'S DRAWINGS.
- (N) ENTRY CANOPY PC #04-117117. IN COMPLIANCE WITH CBC SECTION 3105. SEE MANUFACTURER'S DRAWINGS. S2 DESIGN PARAMETERS, BUILDING DATA & S3 GENERAL NOTES, STEEL NOTES, CONCRETE NOTES SECTIONS.
- (N) MARQUEE SIGN. SEE MANUFACTURER DRAWING PC#04-116862
- (E) BACKFLOW PREVENTOR
- (E) STREET LIGHT
- (E) ACCESSIBLE RAMP PER 01-117027
- (E) STAFF RESTROOM PER DSA #01-117027
- (N) FIRE APPARATUS ACCESS GATE 22'-0" WIDE. SEE DETAIL 11/A0.5

DSA

810

FIRE & LIFE SAFETY SITE CONDITIONS SUBMITTAL

Division of the State Architect (DSA) documents referenced within this publication are available on the DSA Forms or DSA Publications webpages.

To facilitate the Division of the State Architect's (DSA) fire and life safety plan review of project site conditions, DSA requires the design professional to provide the following information at time of project submittal for projects consisting of construction of a new campus, construction of new building(s), additions to existing buildings, and for site alternate design means for fire department emergency vehicle access, and fire suppression water supply.

Information associated with compliance items 1 through 3 below is to be provided for all project types indicated above. Information associated with items 4 through 7 is to be completed when an alternate means is utilized. Acknowledgment by the school district and signature from the Local Fire Authority (LFA) is only required when an alternate design means is being requested.

The Project Information and Fire & Life Safety Information sections are to be completed for all projects and imaged onto the fire access site plan. When an alternate design/means is proposed, all sections on pages 1 and 2 are to be completed and imaged on the fire access site plan.

For additional information refer to the instructions at the end of this form and DSA Policy PL 09-01: Fire Flow for Buildings.

PROJECT INFORMATION		
School District/Owner:	BERRYESSA UNION SCHOOL DISTRICT	
Project Name/School:	PIEDMONT MIDDLE SCHOOL SHADE STRUCTURES, ENTRY CANOPY, AND MARQUEE	
Project Address:	955 PIEDMONT ROAD, SAN JOSE, CA 95132	

FIRE & LIFE SAFETY INFORMATION		
1. Has a fire hydrant flow test been performed within the past 12 months? (If yes, provide a copy of the test data.)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
2. Was the fire hydrant water flow test performed as part of this LFA review?	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
3. Is the project located within a designated fire hazard severity zone (FHSZ) as established by Cal-Fire? (If yes, indicate FHSZ classification below.)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
Refer to the following website for FHSZ locations: http://egis.fire.ca.gov/FHSZ/	Moderate <input type="checkbox"/>	High <input type="checkbox"/> Very High <input type="checkbox"/>
Wildland Interface Area (WIFA) (If any designations are checked, project design must meet the requirements of the WIFA.)	WIFA <input type="checkbox"/>	

CONDITION MEANS AND METHODS RESOLUTION	ALTERNATE ACCEPTED		
	Yes	No	N/A / N/R
4. Emergency vehicle access roadways do not meet CFC requirements.			<input checked="" type="checkbox"/>
4a. Acceptable Alternate: Emergency vehicle and personnel access as proposed by the project architect is acceptable for providing fire suppression and protection of life and property.			
5. Fire Hydrants: Number and spacing does not meet CFC requirements.			<input checked="" type="checkbox"/>
5a. Acceptable Alternate: Number of fire hydrants and spacing as proposed by the project architect is acceptable for fire suppression and protection of life and property.			
6. Fire Hydrants: Water flow and pressure are less than CFC minimum.			<input checked="" type="checkbox"/>
6a. Acceptable Alternate: The available flow and pressure is acceptable for providing fire suppression and protection of life and property.			
7. Location of fire department connection(s) serving fire sprinkler systems or standpipe systems does not meet CFC requirements.			<input checked="" type="checkbox"/>
7a. Acceptable Alternate: The location of fire department connection serving the fire sprinkler system and/or standpipe system is acceptable for providing fire suppression and protection of life and property.			

School District Acceptance of Acceptable Design Alternates

By signing this form, the school district acknowledges and accepts the proposed design as an alternative to California Building Code (CBC) and California Fire Code (CFC) minimum requirements, as indicated by one or more of the conditions indicated at items 4a, 5a, 6a or 7a, for providing fire and life safety protection of life and property.

Accepted by: _____ Title: _____
Signature: _____ Date: _____

LOCAL FIRE AUTHORITY (LFA) INFORMATION	
LFA Agency Name:	SJFD - Bureau of Fire Prevention
LFA Review Official:	Gordana Sabatelli
Title:	Associate Engineer
Work Email:	gordana.sabatelli@sanjoseca.gov
Work Phone:	(408) 535-5686

LFA Reviewer's Signature: _____ Date: _____

IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP: 01-116894 INC. REVIEWED FOR: SS <input checked="" type="checkbox"/> FLS <input checked="" type="checkbox"/> ACS <input checked="" type="checkbox"/> DATE: 08/13/2020 (DSA STAMP AREA)
SUGIMURA FINNEY ARCHITECTS SFA ARCHITECTURE INTERIORS PLANNING 2125 SOUTH BASCOM AVE. SUITE 200 CAMPBELL, CA 95008 PHONE: 408/577-0400 FAX: 408/577-0404



GRAPHIC KEY

- EXISTING PROPERTY LINE
- ASSUMED PROPERTY LINE
- ACCESSIBLE PATH OF TRAVEL
- ROOF OVERHANG
- CHAIN LINK FENCE
- WOOD FENCE
- DECORATIVE FENCE

FIRE DEPARTMENT ACCESS.
FIRE DEPARTMENT ACCESS IS 20' WIDE AND RATED FOR 90,000 LBS.

- (E) DRY STAND PIPE
- (E) FIRE HYDRANT
- DRINKING FOUNTAIN
- (E) SIGN

- NEW BUILDING
- EXISTING BUILDING

PARKING COUNT

PER 2016 CBC, TABLE 11B-208.2

(E) PARKING LOT
TOTAL PARKING SPACES (INCLUDING ALL ACCESSIBLE PARKING SPACES) = 73
MINIMUM ACCESSIBLE PARKING SPACES REQUIRED = 3
TOTAL STANDARD ACCESSIBLE SPACES * TOTAL VAN ACCESSIBLE SPACES*
PROVIDED = 1 + 3 = 4 THEREFORE, OKAY.

* FOR EVERY SIX STANDARD ACCESSIBLE SPACES REQUIRED, AT LEAST ONE SHALL BE A VAN PARKING SPACE.

BUILDING CODE ANALYSIS				
BUILDING	CONSTRUCTI ON TYPE OCCUPANCY TYPE	AREA (SQ.FT.)	ALLOWABLE (SQ.FT.)	# OF STORIES
(N) SHADE STRUCTURE	II-B / A3	3,825	6000	1

OCCUPANT LOAD ANALYSIS:

(N) SHADE STRUCTURE: (A3) ASSEMBLY UNCONCENTRATED:
3,825 SQ.FT./15 = 255<300. SPRINKLERS NOT REQUIRED.

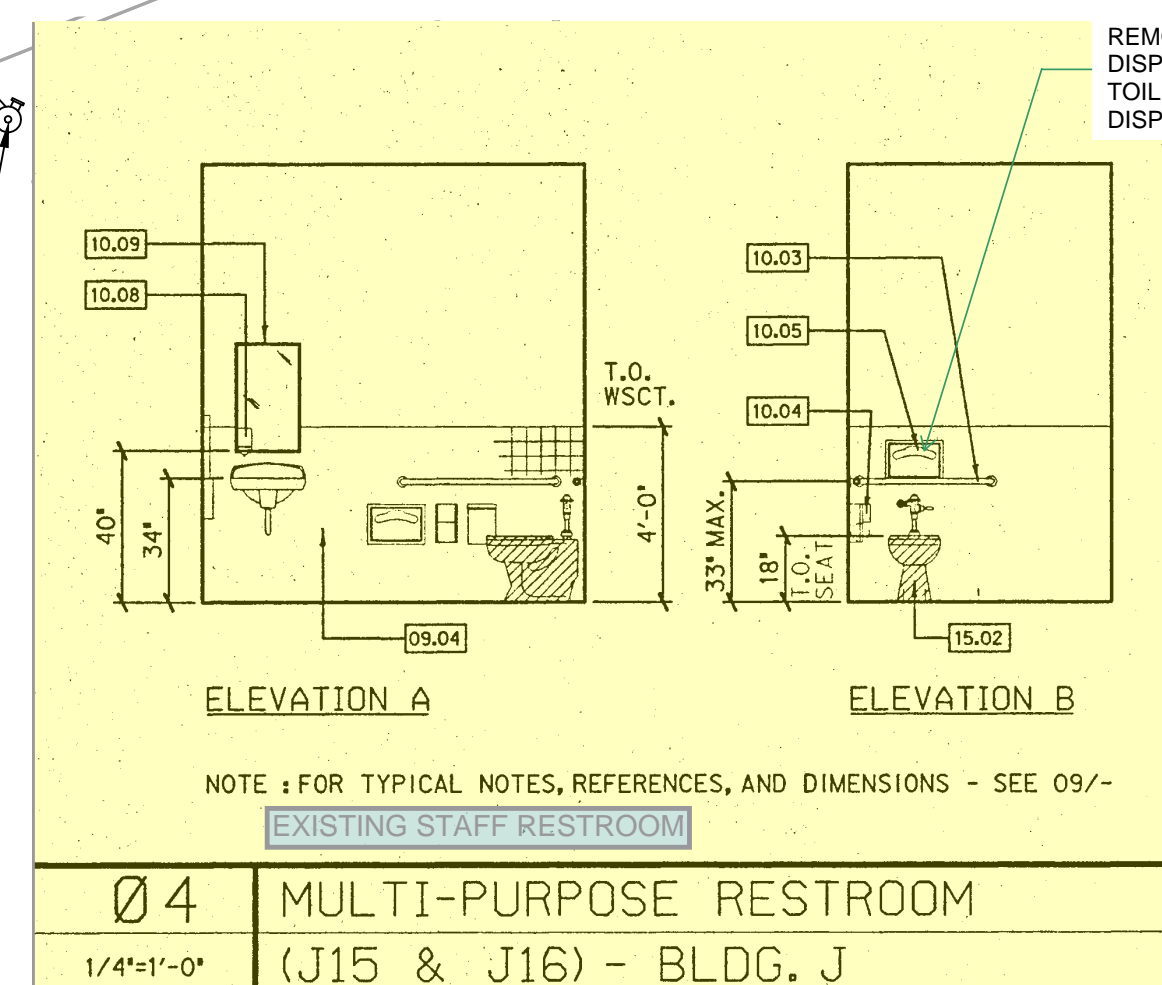
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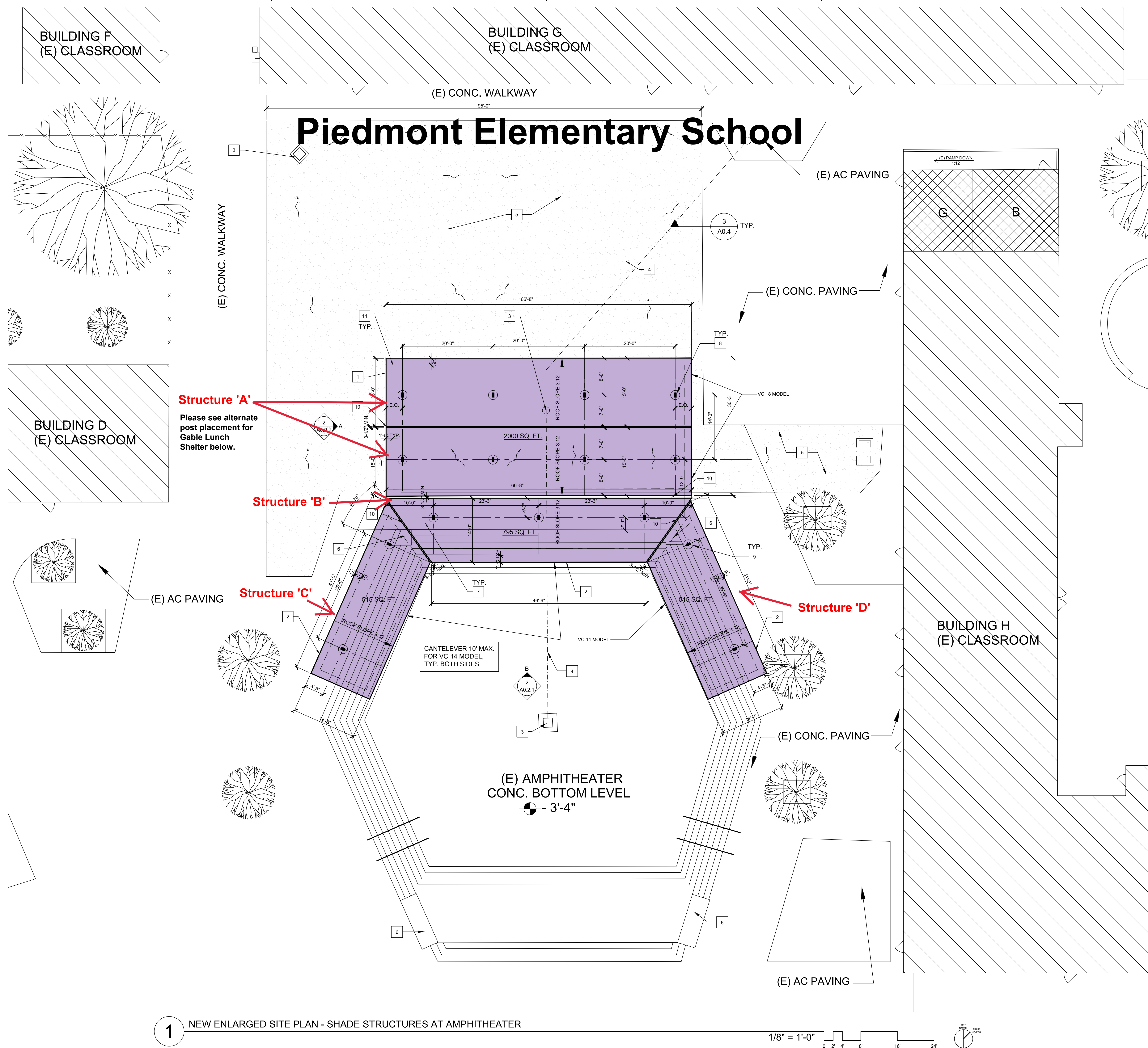
SITE PLAN - FIRE LIFE SAFETY & ACCESS COMPLIANCE

1" = 30'-0"

2

EXISTING STAFF RESTROOM BLDG. J SEE NOTE 16



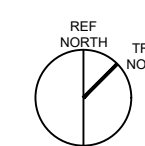


1

NEW ENLARGED SITE PLAN - SHADE STRUCTURES AT AMPHITHEATER

1/8" = 1'-0"

0 2' 4' 8' 16' 24'



GENERAL NOTES

- CONTRACTOR TO VERIFY ALL BARRIERS IN P.O.T. HAVE BEEN REMOVED.
- CONTRACTOR TO REMOVE ALL EXISTING ITEMS TO ALLOW THE NEW WORK, INCLUDING BUT NOT LIMITED TO TREES, SHRUBS, ASPHALT PAVING, FENCING.
- GENERAL CONTRACTOR SHALL SURVEY THE AREA OF NEW CONSTRUCTION FOR UNDERGROUND UTILITIES PRIOR TO THE BEGINNING OF CONSTRUCTION, AND REROUTE/CAP ALL EXISTING UTILITIES RUNNING BELOW THE AREA OF THE NEW SHADE STRUCTURES IF IT CONFLICTS WITH NEW SHADE STRUCTURE FOOTINGS.
- REFER TO SHADE STRUCTURES MANUFACTURER'S DRAWINGS FOR CONC. FOOTINGS DESIGN REQUIREMENTS.
- SHADE STRUCTURES & MARQUEE SIGN O.F.C.I.
- CONTRACTOR TO BACKFILL TRENCHES AND PATCH AC PAVING AS REQUIRED PER DET. 2/A0.4 & 4/A0.4
- PROVIDE TEMPORARY FENCING DURING CONSTRUCTION, SEE DETAIL 1/A0.4
- TIE IN (N) SHADE STRUCTURE DOWNSPOUTS TO THE NEAREST CLEAN OUT.

NEW SITE PLAN NOTES

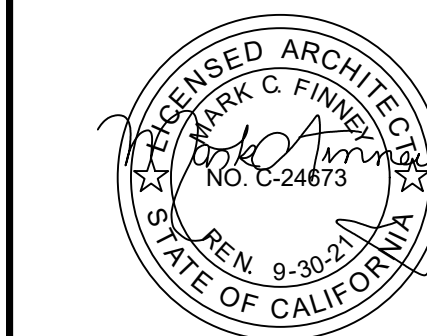
- (N) UPPER SHADE STRUCTURE PC # 04-117117 SEE MANUFACTURER'S DRAWINGS.
- (N) SHADE STRUCTURE AT (E) AMPHITHEATER, PC # 04-117117 SEE MANUFACTURER'S DRAWINGS.
- APPROXIMATE LOCATION OF (E) STORM DRAIN INLET.
- APPROXIMATE DIRECTION OF (E) STORM DRAIN LINE
- (N) AC PAVING, 2% MIN. SLOPE IN ALL DIRECTIONS, SEE DET. 2/A05
- (N) CONC INFILL. SEE DET. 5/A05
- (N) SHADE STRUCTURE COLUMN 10' HEIGHT, TYP.
- (N) SHADE STRUCTURE COLUMN 12'-0" HEIGHT, TYP. SEE 2/S-7, 2/S-5
- (N) SHADE STRUCTURE COLUMN FOOTING, TYP. SEE 1 & 3/S-10
- SEISMIC GAP PER S-2, TYP.
- ROOF DECK CANTILEVER, TYP. PER DET. 3/S6, 3/S5, 5/S11

GRAPHIC KEY

- EXISTING PROPERTY LINE
- ROOF OVERHANG
- CHAINLINK FENCE
- DECORATIVE FENCE
- NEW SHADE STRUCTURE
- EXISTING BUILDING
- EXISTING RESTROOMS
- NEW AC PAVING
- (E) DRY STAND PIPE
- DRINKING FOUNTAIN
- (E) FIRE HYDRANT
- (E) SIGN
- (E) MENS TOILET ROOM
- (E) WOMENS TOILET ROOM
- (E) GIRLS TOILET ROOM
- (E) BOYS TOILET ROOM
- (E) UNISEX TOILET ROOM
- (E) KINDERGARTEN TOILET ROOM

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01-118984 INC:
REVIEWED FOR:
SS ☒ FLS ☒ ACS ☒
DATE: 08/13/2020
(DSA STAMP AREA)

SUGIMURA
FINNEY
ARCHITECTS
SFA
ARCHITECTS INTERIORS PLANNING
2155 SOUTH BASCOM AVE.
SUITE 200
CAMPBELL, CA 95008
PHONE: 408-879-6800
FAX: 408-377-6006



NEW ENLARGED SITE PLAN - SHADE STRUCTURES
AT AMPHITHEATER

SHADE STRUCTURES
PIEDMONT MIDDLE SCHOOL
955 PIEDMONT RD. SAN JOSE, CA 95132
BERRYESSA UNION SCHOOL DISTRICT

REVISIONS
NO. ITEM DATE

DRAWN BY: MK
CHECKED BY: NJ
SFA JOB NO: 19083 DATE: 06/17/2019

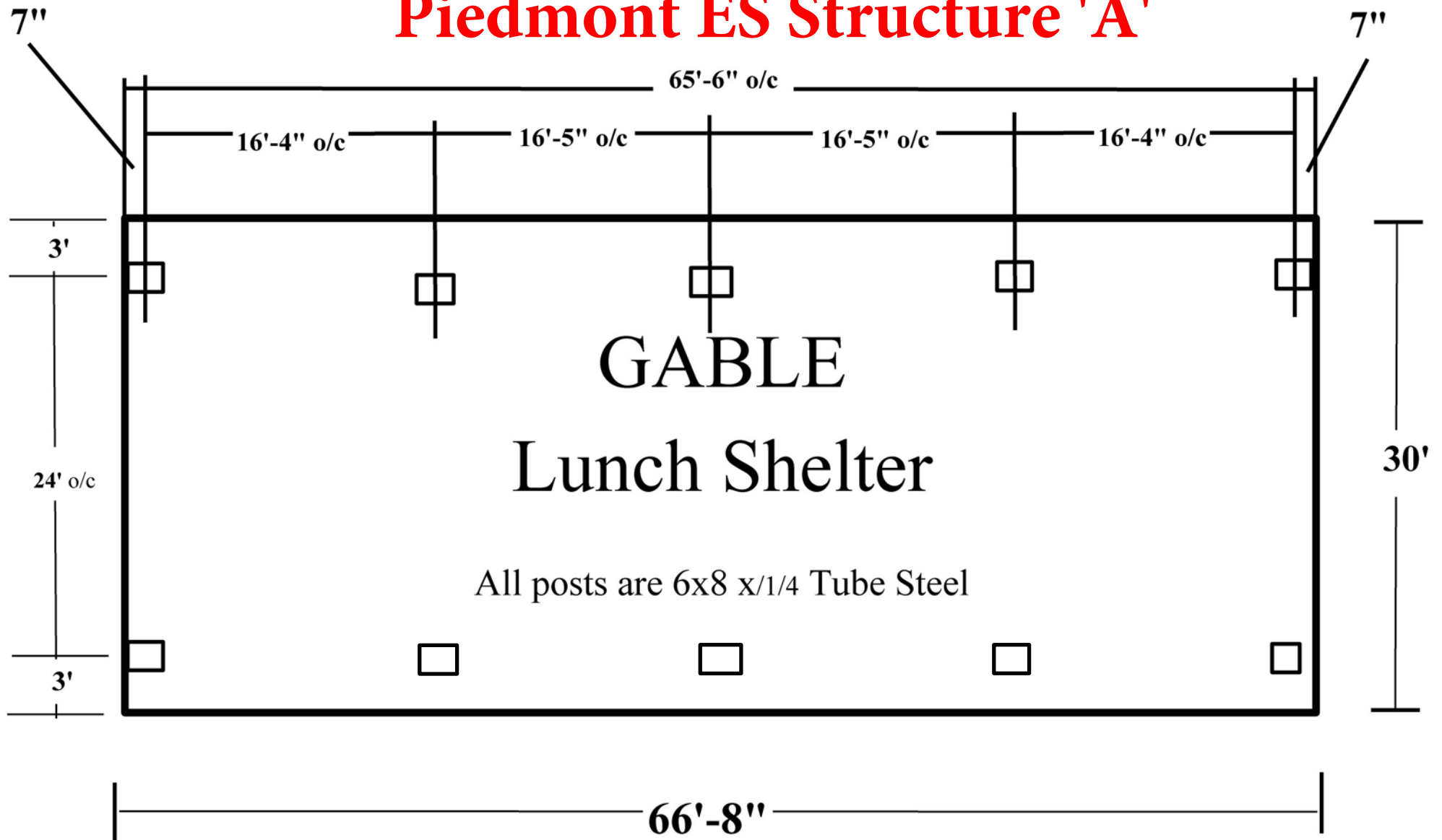
A0.2

Valley School Shelters

PO BOX 177 Tulare, CA 93275

Standard 30 x 66'-8" Standard Gable Lunch Shelter

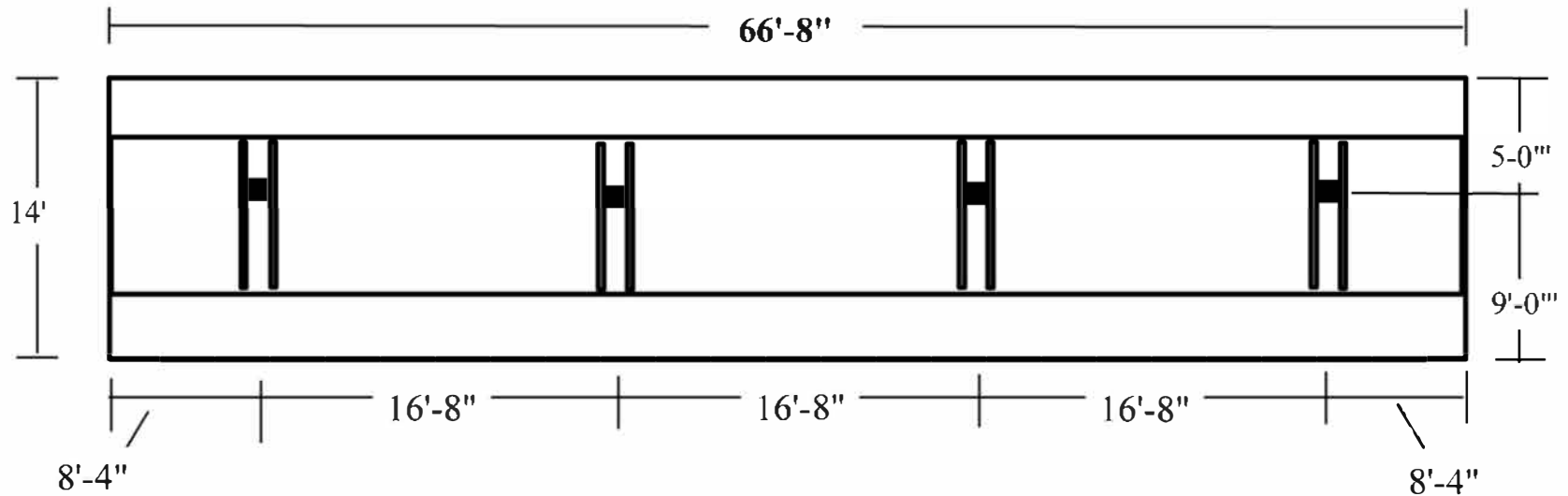
Piedmont ES Structure 'A'



Single Post Walkway Cover

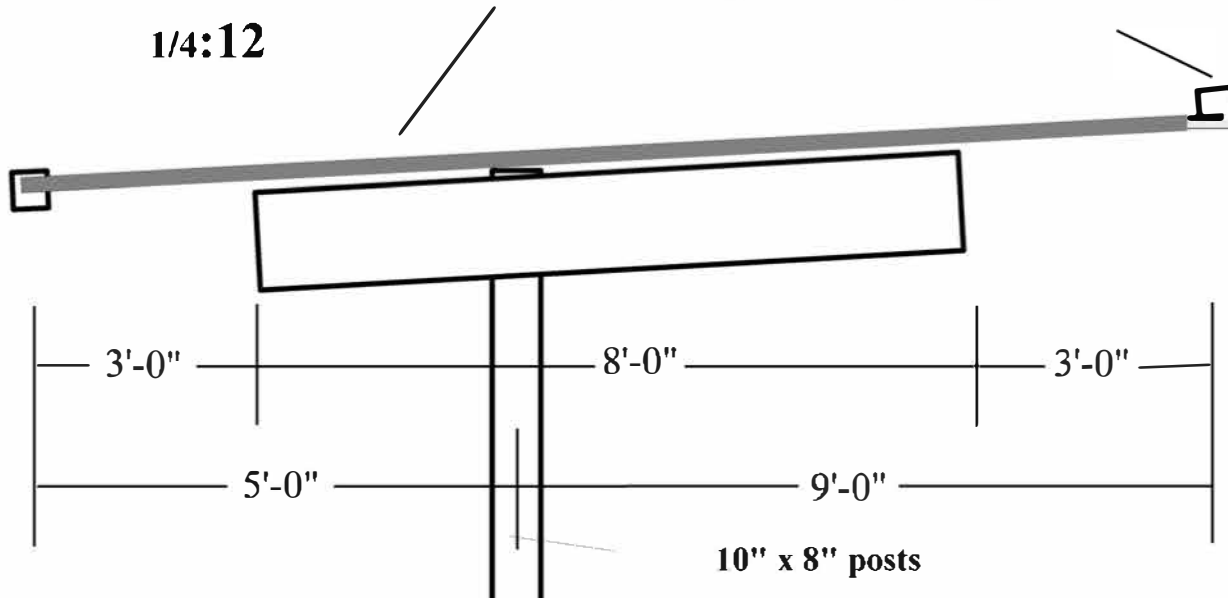
14' x 66'-8" with (4) posts

Piedmont ES Structure 'B'



STD pitch is
1/4:12

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included



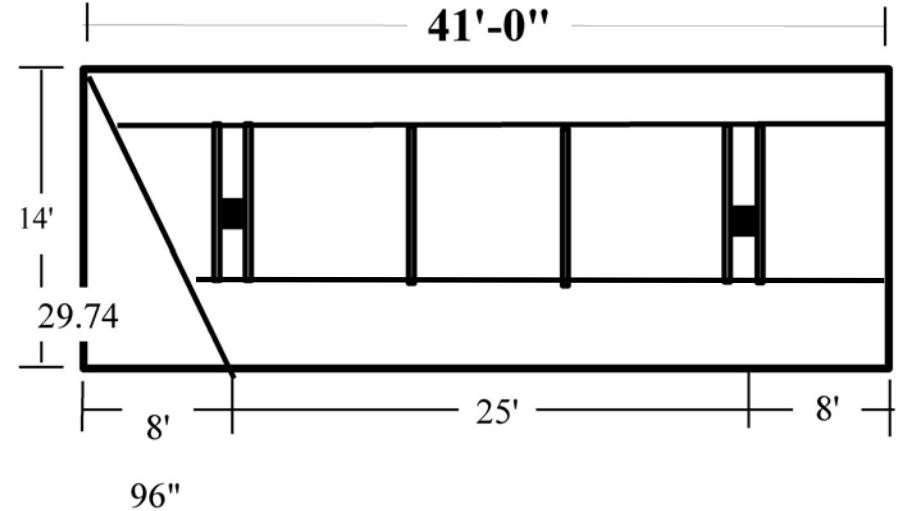
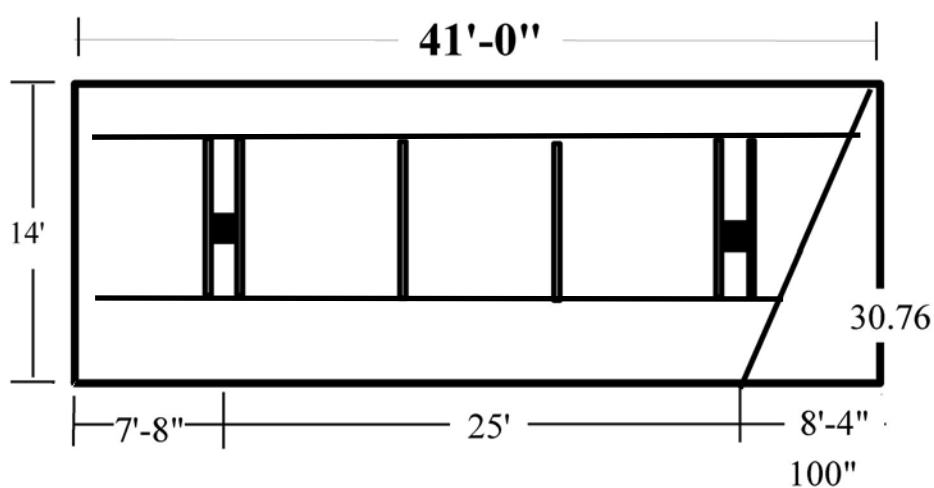
Valley School Shelters
P O Box 177
Tulare, CA 93275

Valley School Shelters

Single Post Walkway Cover

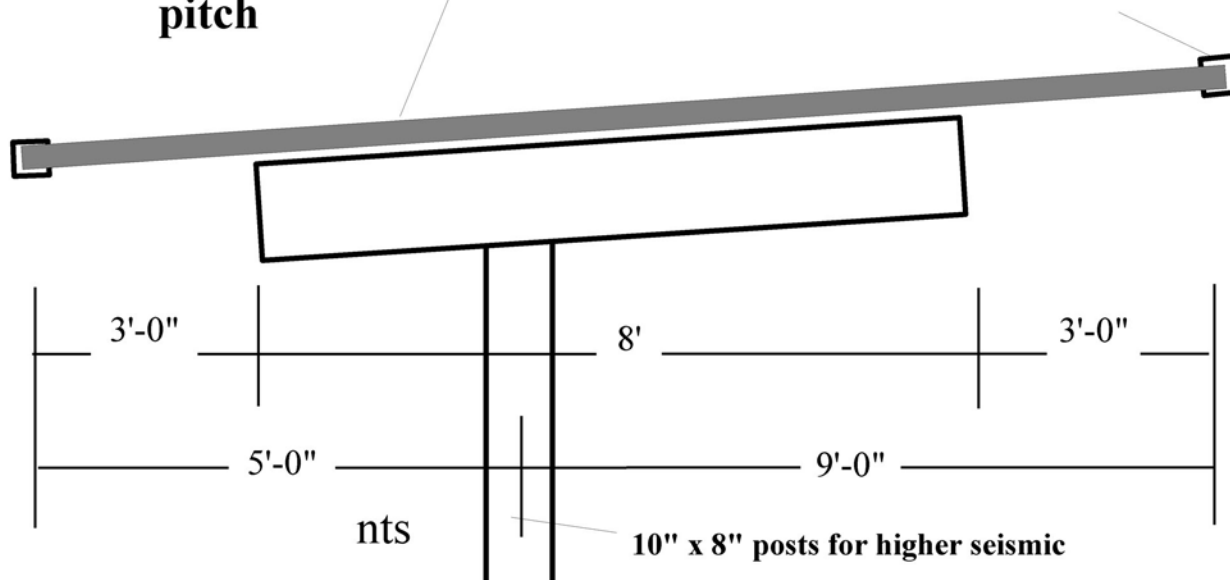
14' x 41' with Cantilevered Ends

Piedmont ES Structures 'C' and 'D'



**Up to 2:12
pitch**

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included



Cantilevers can be
from 6" min - to 3'-0"
each side.

Valley School Shelters
PO Box 177 - Tulare, CA 93275
www.SchoolLunchShelters.com

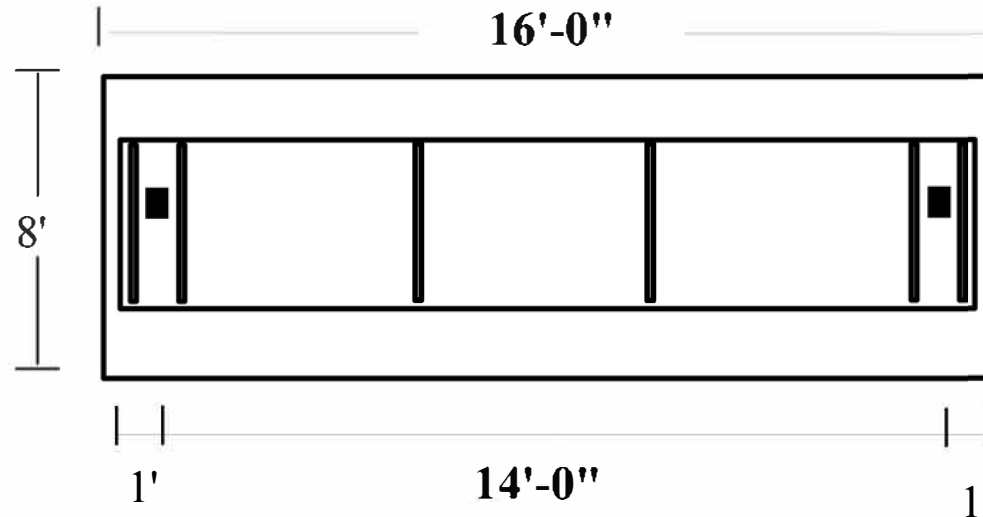
NTS

Valley School Shelters

Single Post Walkway Cover

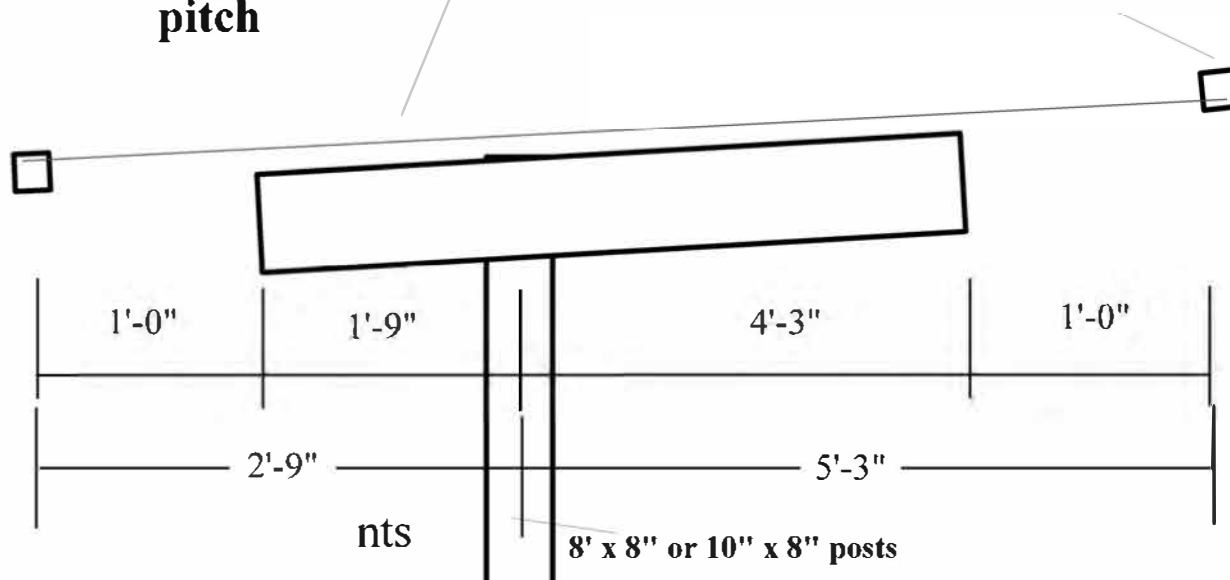
8' x 16' with 12" Cantilevered Ends

Piedmont ES Structures 'E'



**Up to 2:12
pitch**

Roofing is all 26 ga. Factory Painted
Over a dozen colors - Trim Included



Valley School Shelters
PO Box 177 - Tulare, CA 93275
www.SchoolLunchShelters.com

NTS

Single Post Walkway Cover DSA Pre-Checked Plans NO STAMP

Stamped plans to be provided if approved equal is approved.

Single Post Walkway Covers Used On:

- **Piedmont Elementary Structure 'B'**
- **Piedmont Elementary Structure 'C'**
- **Piedmont Elementary Structure 'D'**
- **Piedmont Elementary Structure 'E'**

GENERAL NOTES

1. ALL DIMENSIONS, CONDITIONS AND ELEVATIONS ARE TO BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORK OR FABRICATION. IF ANY DISCREPANCIES ARE FOUND OR IF ANY CONDITION EXISTS NOT AS SHOWN ON THE DRAWINGS THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

2. THE CONTRACTOR WILL HOLD HARMLESS, INDEMNIFY AND DEFEND THE OWNER, THE ENGINEER, AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS, FROM ANY AND ALL LIABILITY CLAIMS, LOSSES OR DAMAGE ARISING OR ALLEGED TO ARISE FROM THE PERFORMANCE OF THE WORK DESCRIBED HEREIN, BUT NOT INCLUDING THE SOLE NEGLIGENCE OF THE OWNER, THE ENGINEER AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS.

3. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE DESIGN INTENT. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES.

IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.

ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION.

4. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON, OR NEAR THE CONSTRUCTION SITE.

5. ANY SUPPORT SERVICES PERFORMED BY THE ENGINEER OR HIS REPRESENTATIVES DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER OF MATERIAL OR WORK AND WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION ARE ASSISTING SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATION, BUT THEY DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

6. ANY CHANGES TO THE APPROVED SET OF PLANS WITHOUT NOTIFYING THE ENGINEER PRIOR TO SUCH CHANGES ABSOLVES SAID ENGINEER FROM ANY AND ALL RESPONSIBILITY WITH RESPECT TO LIABILITY, DAMAGE OR EXTRA WORK RESULTING FROM SAID CHANGES.

7. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.

8. THE TYPICAL DETAILS SHOWN ON THESE SHEETS SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY SHOWN OTHERWISE. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS SHOWN FOR OTHER SIMILAR WORK.

9. DESIGN LOADS: RISK CATEGORY II OR RISK CATEGORY III
ROOF LIVE LOAD: 20 PSF, ROOF DEAD LOAD: 3 PSF
SNOW LOAD: $E_s = P_g + 20 \text{ PSF} + 30 \text{ PSF}$ $C_s = 1.1$ $C_t = 1.2$ $C_e = 1.0$
 $I_s = 1.0$ RISK CATEGORY II $I_s = 1.1$ RISK CATEGORY III

SEISMIC: $I_e = 1.0$ AT RISK CATEGORY II $I_e = 1.25$ AT RISK CATEGORY III
SEISMIC DESIGN CATEGORY = D $\rho = 1.3$ FOR OFFSET CONFIGURATION
 $S_{DS} = 1.0$ & $S_{D5} = 1.75$

STRUCTURE IS A STEEL, ORDINARY CANTILEVERED COLUMN SYSTEM (G2 PER ASCE7-10)
 $R = 1.25/0.7$ (FOR WORKING STRESS)
 $C_D = 1.25$ $C_A = 1.25$
SOIL SITE CLASS = D
FOR 12' COLUMN HEIGHT $T = 0.361$ RISK CATEGORY II $V = 0.728W$ (ASD) FOR $S_{DS} = 1.0$
FOR 14' COLUMN HEIGHT $T = 0.45$ RISK CATEGORY II $V = 1.82W$ (ASD) FOR $S_{DS} = 2.5$
FOR 16' COLUMN HEIGHT $T = 0.48$ HIGH SEISMIC $C_{u1} V = 0.716W$ (ASD) FOR $S_{DS} = 2.5$
HIGH SEISMIC $C_{u1} V = 1.55W$ (ASD) FOR $S_{DS} = 2.5$

WIND LOAD: 135 MPH, EXPOSURE C $K_d = 0.85$ RISK CATEGORY II OR RISK CATEGORY III
 $K_z = 0.90$ $K_{zt} = 1.0$
 $q_h = 21.42$ ($q_h = 0.00256 \times K_z \times K_{zt} \times K_d \times V^2 \times 0.6$ (FOR WORKING STRESS))
MAXIMUM BAY SPACING FOR PROJECT LOCATED IN SPECIAL WIND REGIONS SHALL BE EQUAL TO OR LESS THAN 135 MPH AND CONFORM WITH THE ADOPTED ORDINANCE OF THE CITY, COUNTY OR CITY AND COUNTY IN WHICH THE PROJECT SITE IS LOCATED AND SHALL BE APPROVED BY DSA-SS.

10. GOVERNING CODE: 2016 CBC

11. ALLOWABLE SOIL BEARING IS BASED ON 1500 PSF & 100 PCF/2 PASSIVE PRESSURE PER CBC TABLE 1806A.2 & SECTION 1806A.3.4. SKIN FRICTION IS PER 1810A.3.1.4 & IS EQUAL TO 1500/6=250 PSF. FOR UPLIFT SKIN FRICTION SHALL BE 125 PSF (S.F. OF 2).

12. ALL WORK TO BE PERFORMED UNDER THE CONTINUOUS INSPECTION OF A D.S.A. APPROVED INSPECTOR.

13. ALL WORK SHALL CONFORM TO TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

14. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDA OR A CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR AND THE ENGINEER.

15. A PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE OFFICE OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

FOUNDATION NOTES

1. ALL FOOTINGS SHALL EXTEND TO FIRM BEARING IN UNDISTURBED SOIL OR ENGINEERED FILL.

2. NOMINAL TOP OF FLOOR SLAB ELEVATION = DATUM +0'-0" UNLESS OTHERWISE NOTED.

3. ANY EXISTING FILL AT THE BUILDING PAD SHALL MEET THE 82% COMPACTION REQUIREMENTS. ALL ORGANIC MATERIAL, RUBBLE, OR OTHER DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SITE.

4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR WALKWAYS.

5. ALL REINFORCING STEEL, ANCHOR BOLTS, AND OTHER EMBEDDED ITEMS SHALL BE SECURELY POSITIONED IN THE FORMS PRIOR TO POURING OF CONCRETE.

6. SHORING AND BRACING: IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, AND FORM WORK AS REQUIRED FOR THE CONSTRUCTION OF THIS BUILDING. PROVIDE TEMPORARY BRACING AS REQUIRED TO HOLD THE VARIOUS ELEMENTS IN PLACE UNTIL FINAL SUPPORT IS SECURELY ANCHORED.

7. EXCAVATION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL EXCAVATION PROCEDURES AND FOR PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.

8. BACKFILL: DO NOT BACKFILL AROUND THE EXTERIOR PERIMETER WALL UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. IF THE FLOOR SLABS ARE CONCRETE, DO NOT BACKFILL UNTIL 7 DAYS MINIMUM AFTER COMPLETION OF THE FLOOR SLABS. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF DAMP-PROOFING.

CONCRETE NOTES

1. ALL MOLDS, ORNAMENTS, GROOVES, ETC. SHOWN ON THE ARCHITECTURAL DRAWINGS SHALL BE PROVIDED FOR IN THE FORM WORK BEFORE THE CONCRETE IS Poured.

2. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER EMBEDS SHALL BE IN PLACE AND SECURED TO FORM WORK PRIOR TO POURING OF CONCRETE.

3. REFER TO BOTH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF PLUMBING FIXTURES.

4. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE WALLS OR STRUCTURAL SLABS UNLESS SPECIFICALLY DETAILED.

5. CONSTRUCTION JOINTS NOT INDICATED ON THE DRAWINGS SHALL BE SO MADE AND LOCATED AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. PROVISION SHALL BE MADE FOR TRANSFER OF SHEAR AND OTHER FORCES THROUGHOUT THE JOINTS. THE CONTRACTOR SHALL OBTAIN THE ARCHITECT'S APPROVAL OF CONSTRUCTION JOINT LOCATION IN ALL STRUCTURAL SLAB, BEAMS AND SHEAR WALLS.

6. SIDES OF FOOTINGS MAY BE POURED AGAINST STABLE EARTH.

7. THE QUALITY AND DESIGN OF CONCRETE SHALL COMPLY WITH TITLE 24 PART 2 EXCEPT ITEMS NOT SPECIFICALLY COVERED THEREIN SHALL CONFORM TO ACI 318.

8. ALL REINFORCING SHALL BE NEW STOCK DEFORMED BARS CONFORMING TO ASTM A615.

A. #4 BARS AND SMALLER..... GRADE 40 OR 60
B. #5 BARS AND LARGER..... GRADE 60
C. SEPARATE BARS 1-1/2 DIAMETERS CLEAR OR 1-1/2" CLEAR, WHICHEVER IS LARGER.

9. MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:
CAST AGAINST EARTH (EXCEPT SLABS ON GRADE)..... 3"
EXPOSED TO EARTH OR WEATHER..... 1-1/2"
#5 BARS AND SMALLER..... 1-1/2"
#6 BARS AND LARGER..... 2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
SLABS, WALLS, JOISTS..... 3/4"
#11 BARS AND SMALLER..... 1-1/2"
#14 AND #18 BARS..... 1-1/2"
BEAMS, GIRDERS, COLUMNS
PRINCIPAL REINFORCING, TIES
STIRRUPS, OR SPIRALS..... 1-1/2"
SHEELS AND FOLDED PLATE MEMBERS
#5 BARS AND SMALLER..... 1/2"
#6 BARS AND LARGER..... 3/4"

10. CONCRETE SHALL HAVE FOLLOWING MINIMUM REQUIREMENTS.
F0- 3000 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.5.
F2- 4500 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.45 & MINIMUM AIR CONTENT OF 7%.

11. THE ENGINEER DOES NOT PROVIDE CONTRACT ADMINISTRATION FOR THE PROJECT, INCLUDING REVIEW OF CONCRETE MIXES.

12. PER ACI SECTION 19.3.21. FOOTING SHALL NOT BE EXPOSED TO FREEZING AND THAWING CYCLES. SHALL NOT BE EXPOSED TO WATER-SOLUBLE SULFATE IN SOIL BY PERCENT OF MASS 20.10%, SHALL NOT BE EXPOSED TO EXTERNAL SOURCE OF CHLORIDES.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL EXCEPT W SHAPES SHALL CONFORM TO ASTM A-36 AND SHALL BE FABRICATED AND ERECTED AS PER AISC SPECIFICATIONS FOR BUILDINGS. W SHAPES SHALL CONFORM TO ASTM A992.

2. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE "B" AND STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500 GRADE "B", $F_y=46\text{KSI}$.

3. ALL LIGHT GAGE STEEL TO CONFORM TO ASTM A653 GRADE 55 FOR ALL STRUCTURAL SHAPES, A653 GRADE 33 FOR ALL BLOCKING, FLASHINGS, MISCELLANEOUS CONNECTION PLATES, AND ANGLES.

4. ALL UNFINISHED BOLTS SHALL BE ASTM A-307 UNLESS NOTED OTHERWISE.

5. USE AISC USUAL GAGES FOR BOLT HOLES IN ALL STEEL SECTIONS UNLESS OTHERWISE NOTED.

6. THE STEEL FABRICATOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR ERECTION.

7. ALL BOLT HOLES ARE TO BE 1/16" OVERSIZED. ALL BOLTS SHALL HAVE WASHERS INSTALLED UNDER BOTH HEAD & NUT.

8. ALL STEEL SHALL BE PROTECTED FROM WEATHER AS FOLLOWS: STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED (MINIMUM ASTM A123 OR A153, CLASS D) OR PAINTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT OR EQUIVALENT PAINT SYSTEM. COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM-ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION.

ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT-DIP GALVANIZED (ASTM A153, CLASS D MINIMUM), OR PROTECTED WITH CORROSION-PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT. (EXAMPLE: PROPRIETARY COATINGS THAT DO COMPLY WITH THE 1,000 HOUR REQUIREMENT INCLUDE, BUT ARE NOT NECESSARILY LIMITED TO: QUIK GUARD BY SIMPSON, Kwik-COTE by HILTI, STALGARD by ELCO, VISTACORR by SFS INTEC, ETC.)

GOVERNING CODES:

1. 2016 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE (PART 1, TITLE 24, CCR).

2. 2016 CALIFORNIA BUILDING CODE, VOLUMES 1 & 2 (PART 2, TITLE 24, CCR).

3. 2016 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR).

4. 2016 CALIFORNIA FIRE CODE (PART 9, TITLE 24, CCR).

NOTES

1. COVERS ARE NOT DESIGNED TO BE ENCLOSED OR FOR STORAGE OF COMBUSTIBLE MATERIALS.

2. WALKWAY COVER HAS BEEN CHECKED FOR OBSTRUCTED WIND FLOW CONDITION & CAN BE WITHIN 6" MIN. FROM AN EXISTING BUILDING. SHALL BE REVIEWED ON A SITE SPECIFIC BASIS.

3. WALKWAY PIER FOOTING HAS BEEN CHECKED FOR D.S.A. BULLETIN 09-06 REV.

TESTING & INSPECTIONS REQUIREMENTS

1. INSPECTOR CLASS (MINIMUM REQUIREMENTS)
CLASS 2

2. SELECTION OF THE PROJECT INSPECTOR AND TESTING AGENCY
BY THE SCHOOL DISTRICT AND APPROVED BY D.S.A., A/E OF RECORD AND STRUCTURAL ENGINEER

3. COST OF THE PROJECT INSPECTOR (CA ADMIN. CODE 4-333(B)) AND TESTING AGENCY (CA ADMIN. CODE 4-333)
BY THE SCHOOL DISTRICT

4. COPIES OF THE REPORT TO
ARCHITECT; STRUCTURAL ENGINEER; SCHOOL DISTRICT; D.S.A. (ORIGINAL); IOR; MANUFACTURER

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316 (D & E) OF THE CALIFORNIA CODE OF REGULATIONS, THE DISTRICT SHALL HIRE AN ARCHITECT OR STRUCTURAL ENGINEER TO BE IN GENERAL RESPONSIBLE CHARGE OF SITE SPECIFIC PROJECT.

2. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.

3. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR A PORTION OF THE PROJECT AS DESIGNED BY THE ARCHITECT FOR INCORPORATION INTO THE PROJECT.

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GERARD HOMER & ASSOCIATES RESPONSIBILITY FOR SITE SPECIFIC PROJECT.

SHADE STRUCTURE TESTING & INSPECTION GUIDELINE

THE EXAMPLE FORM DSA-103 SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECT-SPECIFIC FORM DSA-103. A FORM DSA-103 IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND THE EXAMPLE FORM DSA-103 IS TO BE CROSSED OUT ON THIS DRAWING.

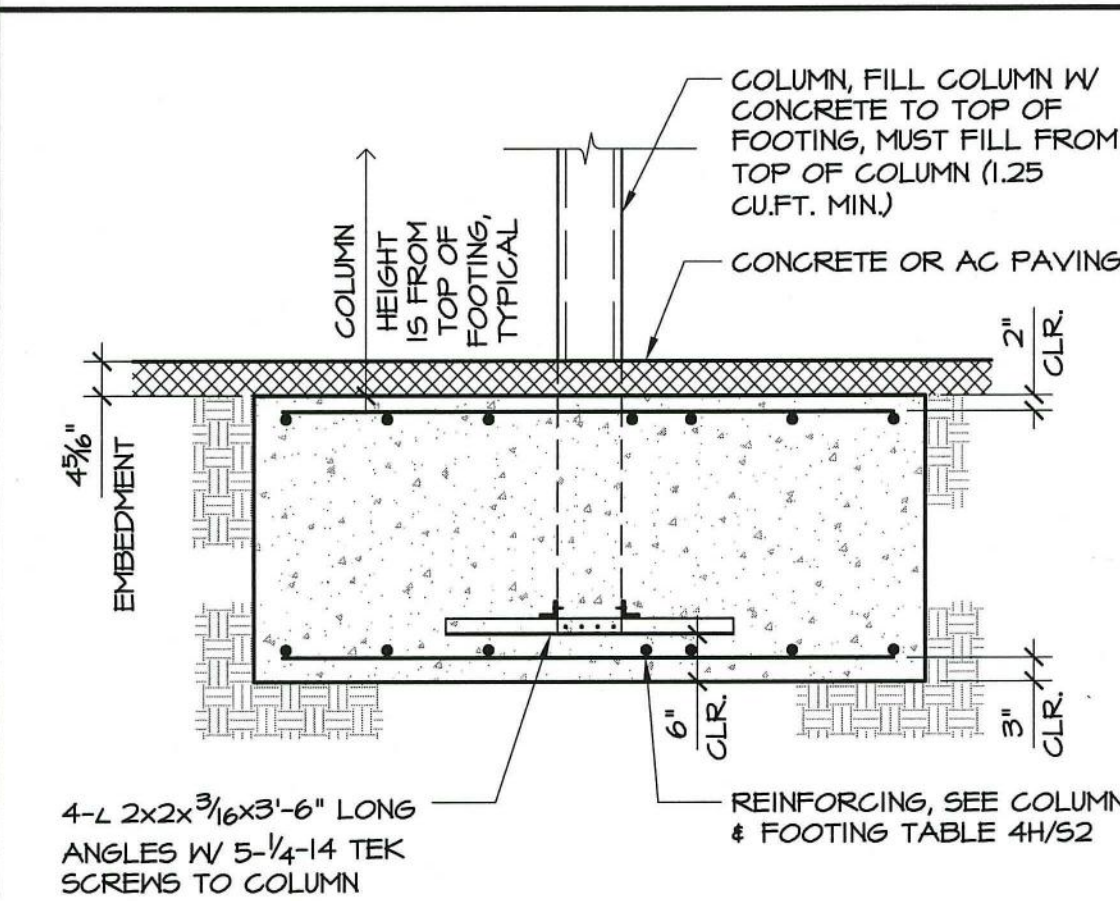
DSA-103 STATEMENT OF STRUCTURAL TESTS & SPECIAL INSPECTIONS - 2016 CBC

IMPORTANT: THIS FORM IS ONLY A SUMMARY LIST OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS REQUIRED FOR A PROJECT. THE ACTUAL TESTS AND INSPECTIONS MUST BE PERFORMED AS DETAILED ON THE DSA APPROVED DOCUMENTS. THE PROJECT INSPECTOR IS RESPONSIBLE FOR PROVIDING INSPECTION OF ALL FACETS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, SPECIAL INSPECTIONS NOT LISTED ON THIS FORM SUCH AS STRUCTURAL WOOD FRAMING, HIGH-LOAD WOOD DIAPHRAGMS, COLD-FORMED STEEL FRAMING, ANCHORAGE OF NON-STRUCTURAL COMPONENTS, ETC., PER TITLE 24, PART 2, CHAPTER 17A.

NOTE: REFERENCES ARE TO THE 2016 EDITION OF THE CALIFORNIA BUILDING CODE (CBC) UNLESS OTHERWISE NOTED.

TEST OR SPECIAL INSPECTION	TYPE	PERFORMED BY	CODE REFERENCE AND NOTES
SOILS			
1. GENERAL: TABLE 1705A.6			
A. VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF CONTROLLED FILL AND/OR EXCAVATIONS FOR FOUNDATIONS.	PERIODIC	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE
A. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH PIER.	CONTINUOUS	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE
B. VERIFY LOCATIONS OF PIERS.	CONTINUOUS	PI	
C. CONFIRM PIER DIAMETERS, PLUMBNESS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, AND EMBEDMENT INTO BEDROCK (IF APPLICABLE). RECORD CONCRETE OR GROUT VOLUMES.	CONTINUOUS	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE
E. CONCRETE PIERS.	PROVIDE TEST AND INSPECTIONS PER CONCRETE SECTION BELOW		
CONCRETE			
TABLE 1705A.3			
17. CAST IN PLACE CONCRETE			
MATERIAL VERIFICATION AND TESTING:			
A. VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	SI & PI	TO BE PERFORMED BY BATCH-PLANT SPECIAL INSPECTOR AND PROJECT INSPECTOR.
B. TEST REINFORCING STEEL.	TEST	LAB	1913A.2 (1913.2.6), ASTM A370, DSA IR17-10
C. PERFORM SLUMP, TEMPERATURE, AND (WHERE REQUIRED) AIR CONTENT TEST.	TEST	LAB	ASTM C172, ASTM C31
D. TEST CONCRETE (COMPRESSION).	TEST	LAB	ACI 318 SECTION 5.6 AND 1905A.1.2 (1913.3.1+), ASTM C39.
INSPECTION:			
E. BATCH PLANT INSPECTION	CONTINUOUS	SI	1705A.3.2. IF APPROVED BY DSA, BATCH PLANT INSPECTION BAT BE REDUCED TO PERIODIC IF PLANT COMPLIES WITH 1705A.3.3. ITEM 1, AND REQUIRES FIRST BATCH INSPECTION, WEIGHMASTER, AND BATCH TICKETS.
F. BATCH PLANT INSPECTION - DESIGN COMPLIES WITH 1705A.3.3 ITEM 2	PERIODIC	SI	1705A.3.3. ITEM 2. REQUIRES FIRST BATCH INSPECTION, WEIGHMASTER, AND BATCH TICKETS.
G. INSPECT PLACEMENT OF FORMWORK, REINFORCING STEEL, EMBEDDED ITEMS, AND CONCRETE. INSPECT CURING AND FORM REMOVAL.	CONTINUOUS	PI	MAY BE PERFORMED BY A SPECIAL INSPECTOR WHEN SPECIFICALLY APPROVED BY DSA.
STEEL			
TABLE 1705A.2.1, ASC 303-10, ASC 360-10, ASC 341-10, ASC 358-10, AISI S100-07/82-10			
17. STRUCTURAL STEEL AND COLD-FORMED STEEL USED FOR STRUCTURAL PURPOSES			
MATERIAL VERIFICATION:			
A. VERIFY THAT ALL MATERIALS ARE APPROPRIATELY MARKED AND THAT: MILL CERTIFICATES ARE AVAILABLE AND PROPERLY MATCH WITH REQUIREMENTS. MATERIAL SIZES, TYPES AND GRADES COMPLY WITH REQUIREMENTS.	PERIODIC	*	BY SPECIAL INSPECTOR WHEN PERFORMED OFF-SITE; BY PROJECT INSPECTOR FOR STEEL SHIPPED, DIRECTLY TO PROJECT SITE WITHOUT WELDING OR FABRICATION.
B. TEST UNIDENTIFIED MATERIALS	TEST	LAB	2203A.1 (2203.1+), ASTM A370
C. EXAMINE SEAM WELDS OF STRUCTURAL TUBES AND PIPES	PERIODIC	SI	DSA IR 17-3
INSPECTION:			
D. VERIFY MEMBER LOCATIONS, BRACING AND ALL DETAILS CONSTRUCTED IN THE FIELD.	CONTINUOUS	PI	
E. VERIFY STIFFENER LOCATIONS, CONNECTIONS TAB LOCATIONS AND ALL CONSTRUCTION DETAILS FABRICATED IN THE SHOP.	PERIODIC	SI	
KEY TO COLUMNS:			
1. TYPE -			
CONTINUOUS - INDICATES THAT A CONTINUOUS SPECIAL INSPECTION IS REQUIRED			
PERIODIC - INDICATES THAT A PERIODIC SPECIAL INSPECTION IS REQUIRED			
TEST - INDICATES THAT A TEST IS REQUIRED			
* SI - INDICATES THAT THE SPECIAL INSPECTION IS TO BE PERFORMED BY A SPECIAL INSPECTOR			
* IN THE CODE REFERENCE AND NOTES COLUMN, IT INDICATES DSA-SS/CC SECTIONS THAT MAY BE USED BY COMMUNITY COLLEGES, PER 2016 CBC SEC. 1.3.2.2.			

GENERAL NOTES



OPTIONAL SPREAD FOOTING
@ ADDITIONAL COST

SCALE 1/2" = 1'-0"

8B

SHEET INDEX & STRUCTURAL TESTS AND INSPECTIONS SHEET

4K

SITE SPECIFIC INFORMATION TABLE

SEISMIC COEFFICIENT, S_{ds}	SELECT OCCUPANT LOAD FACTOR (OLF)	DETERMINATION OF BAY SPACING FOR COVER NEXT TO RISK CATEGORY III & $S_{ds} > 2.0$
WIND SPEED (3 SEC. GUST) @ MPH	ASSEMBLY-CONCENTRATED (7 SQ. FT./OCCUPANT)	
WIND EXPOSURE CATEGORY	ASSEMBLY-NONCONCENTRATED (15 SQ. FT./OCCUPANT)	
WIDTH OF COVER AREA IN FT.	DETERMINE OCCUPANT LOAD = AREA/OLF	28' BAYS \times 2.0 = <input type="text"/> MAXIMUM BAY SPACING IN FEET *
LENGTH OF COVERED AREA IN FT.	DETERMINE RISK CATEGORY	
AREA IN SQ. FT.	RISK CATEGORY II OCCUPANTS \leq 300	
NUMBER OF BAYS	RISK CATEGORY III OCCUPANTS $>$ 300	
CLEAR HEIGHT OF EAVES IN FT.	RISK CATEGORY III ADJACENT TO RISK CATEGORY III BUILDING	
ROOF SLOPE IN/FT.	NOTE: DUE TO REDUNDANCY FACTOR THERE CAN BE NO REDUCTION IN S_{ds} PER CBC 1816A.1.12 FOR COVERS.	
CONCRETE EXPOSURE F0	CONCRETE EXPOSURE F2*	
GEOHAZARD REPORTS		LIQUEFABLE SOIL OR SITE CLASS F
GEOHAZARD REPORT	REQUIRED	NOT REQUIRED
	LIQUEFABLE SOILS	YES
		NO
GEOHAZARD REPORTS ARE NOT REQUIRED FOR CANTILEVERED COLUMN OPEN STRUCTURES PROVIDED THEY ARE CONSTRUCTED OF METAL, DO NOT EXCEED 4,000 ST. IN PLAN AREA AND ARE NOT LOCATED WITHIN STATE OR LOCAL ZONE. IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.		

* F2 IS REQUIRED FOR ANY SNOW LOAD 30 PSF OR GREATER.

SINGLE POLE WALKWAY OPTIONS TABLE

LOADING	COLUMN HEIGHT	ROOF PANEL OPTION	PROJECTION	NOTES:
20 PSF	12' HEIGHT	MELODY MEGA-RIB	0'-0"	1. SEE 6H/53 FOR ROOF DECK PROFILES & SECTION PROPERTIES
20 PSF	14' HEIGHT	AEP SPAN HR-36	10'-0"	2. LIVE LOAD
20 PSF	16' HEIGHT	3 P.S.F.*	12'-0"	3. SNOW LOAD
ROOF PITCH	COLUMN LOCATION	SEISMIC	DEAD LOAD	4. 9 P.S.F. MISCELLANEOUS DEAD LOAD NOT ALLOWED WITH SNOW LOAD
2/12 MAX.	OFF-SET	LOWER SEISMIC	YES	5. SEE SITE SPECIFIC TABLE.
4/12 MAX.	CENTERED	NO	"A"	CONFIGURATION PER 6K/54
			"B"	
			"C"	
			"D"	

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC-VC

IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES

APPL # 02-115686

AC TM F/LS SS ST

DATE 5/29/18

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

OPTIONS TABLE & SITE SPECIFIC INFORMATION TABLE

8K

2574 WEST WHITENDALE
VISALIA, CA 93277

(559) 734-6675
FAX (559) 734-5232
Email: ghomerse@gmail.com

Gerard
Homer and Associates
STRUCTURAL ENGINEERS

SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS

NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY: T.E.H.
CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE
FOUNDATION PLAN
GENERAL NOTES
DETAILS

SHEET NUMBER

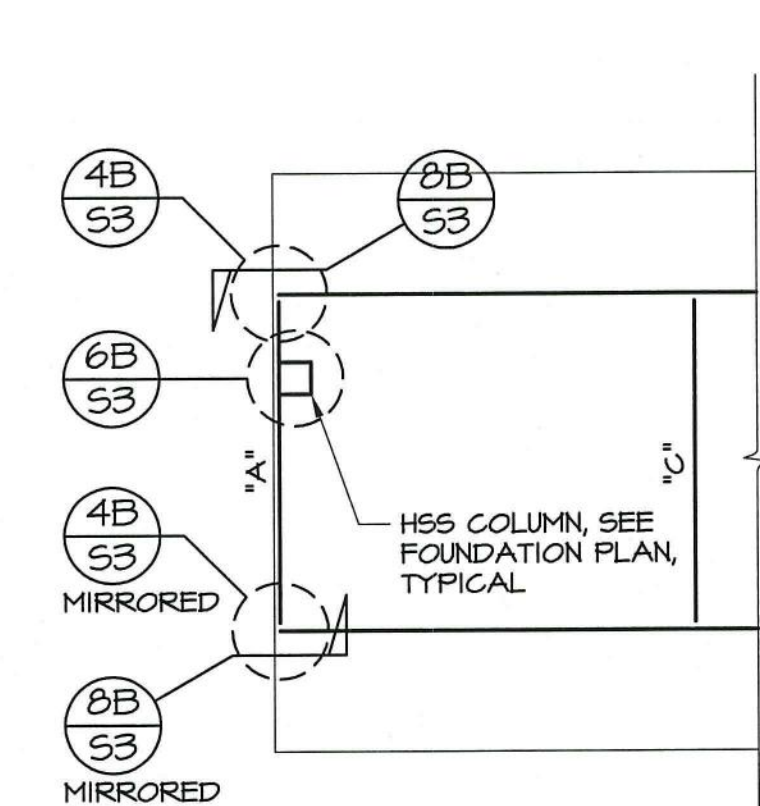
S1

OF 4 SHEETS

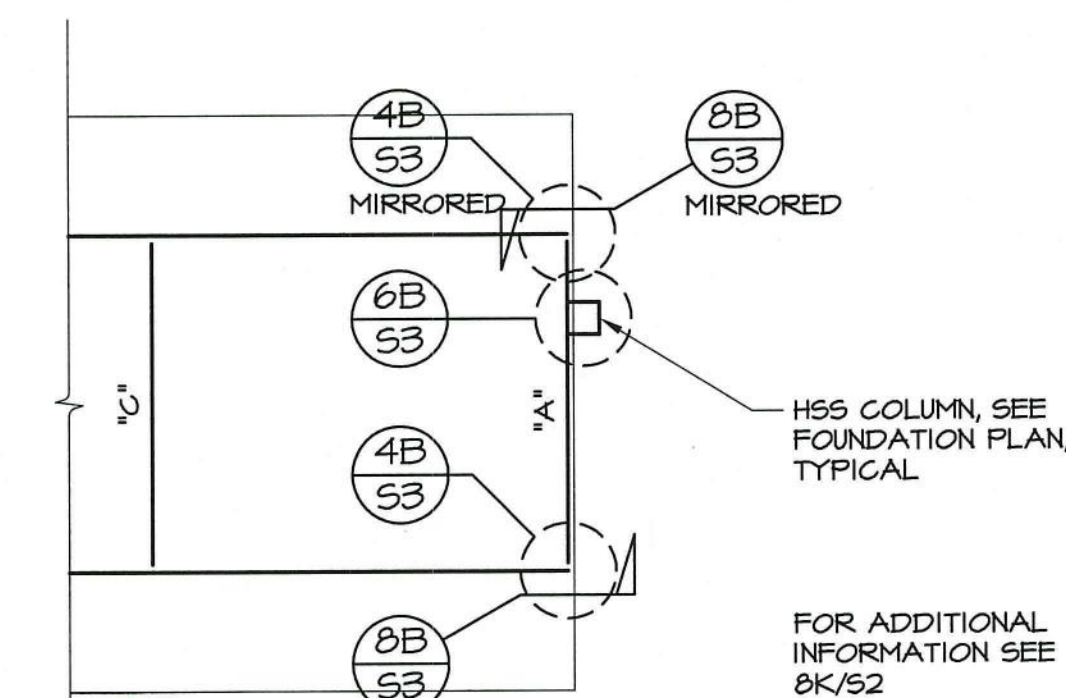
COLUMN & FOOTING TABLE (NOTE 6)														
COLUMN HEIGHT	ROOF PITCH	LOAD	LOWER SEISMIC (S _{DS} = 1.0 RISK CATEGORY II) (S _{DS} ≤ 0.8 RISK CATEGORY III)						HIGHER SEISMIC (S _{DS} = 2.5 RISK CATEGORY II) (S _{DS} ≤ 2.0 RISK CATEGORY III)					
			≤ 12'–0" PROJECTION			≤ 14'–0" PROJECTION			≤ 12'–0" PROJECTION			≤ 14'–0" PROJECTION		
			COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1
12' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x1/4	2'-0"φx10'-3" DEEP	9'-3"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx11'-9" DEEP	10'-4"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx11'-10" DEEP	10'-6"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx13'-2" DEEP	11'-4"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H550x8x3/16	2'-0"φx12'-2" DEEP	9'-8"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx13'-0" DEEP	10'-10"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx12'-2" DEEP		H550x8x3/16		
14' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x1/4	2'-0"φx10'-8" DEEP	9'-8"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx12'-2" DEEP	10'-10"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx12'-3" DEEP	11'-0"SQ.x2'-6" THICK (5)	H550x8x3/16	2'-0"φx13'-6" DEEP	11'-10"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H550x8x3/16	2'-0"φx12'-6" DEEP	9'-10"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx13'-2" DEEP	11'-0"SQ.x2'-6" THICK (5)	H550x8x3/16	2'-0"φx12'-6" DEEP		H550x8x3/16		
16' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x3/16	2'-0"φx11'-0" DEEP	10'-0"SQ.x2'-6" THICK (4)	H550x8x3/16	2'-0"φx12'-6" DEEP	11'-3"SQ.x2'-6" THICK (5)	H550x8x3/16	2'-0"φx12'-8" DEEP	11'-4"SQ.x2'-6" THICK (5)	H550x8x1/2	2'-0"φx14'-0" DEEP	12'-4"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H550x8x3/16	2'-0"φx12'-8" DEEP	10'-2"SQ.x2'-6" THICK (4)	H550x8x1/2	2'-0"φx13'-6" DEEP			2'-0"φx12'-8" DEEP				

NOTES:

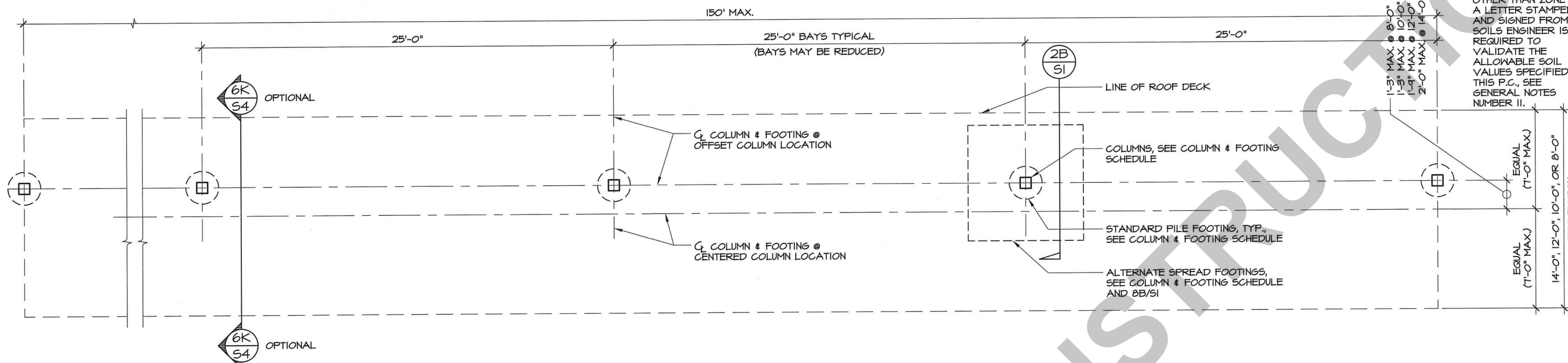
1. ALTERNATE SPREAD FOOTINGS ARE OPTIONAL. FOOTINGS @ AN ADDITIONAL COST.
2. NO SNOW LOAD ALLOWED W/ 3 P.S.F. FIRE SPRINKLER DEAD LOAD.
3. COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 3'-6" MAX. FROM BOTTOM. SEE SECTION 6K/53.
4. PROVIDE 8-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.
5. PROVIDE 10-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.
6. IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C. SEE GENERAL NOTES NUMBER II.



PLAN @ UPPER ROOF WHERE STEP OCCURS



PLAN @ LOWER ROOF WHERE STEP OCCURS



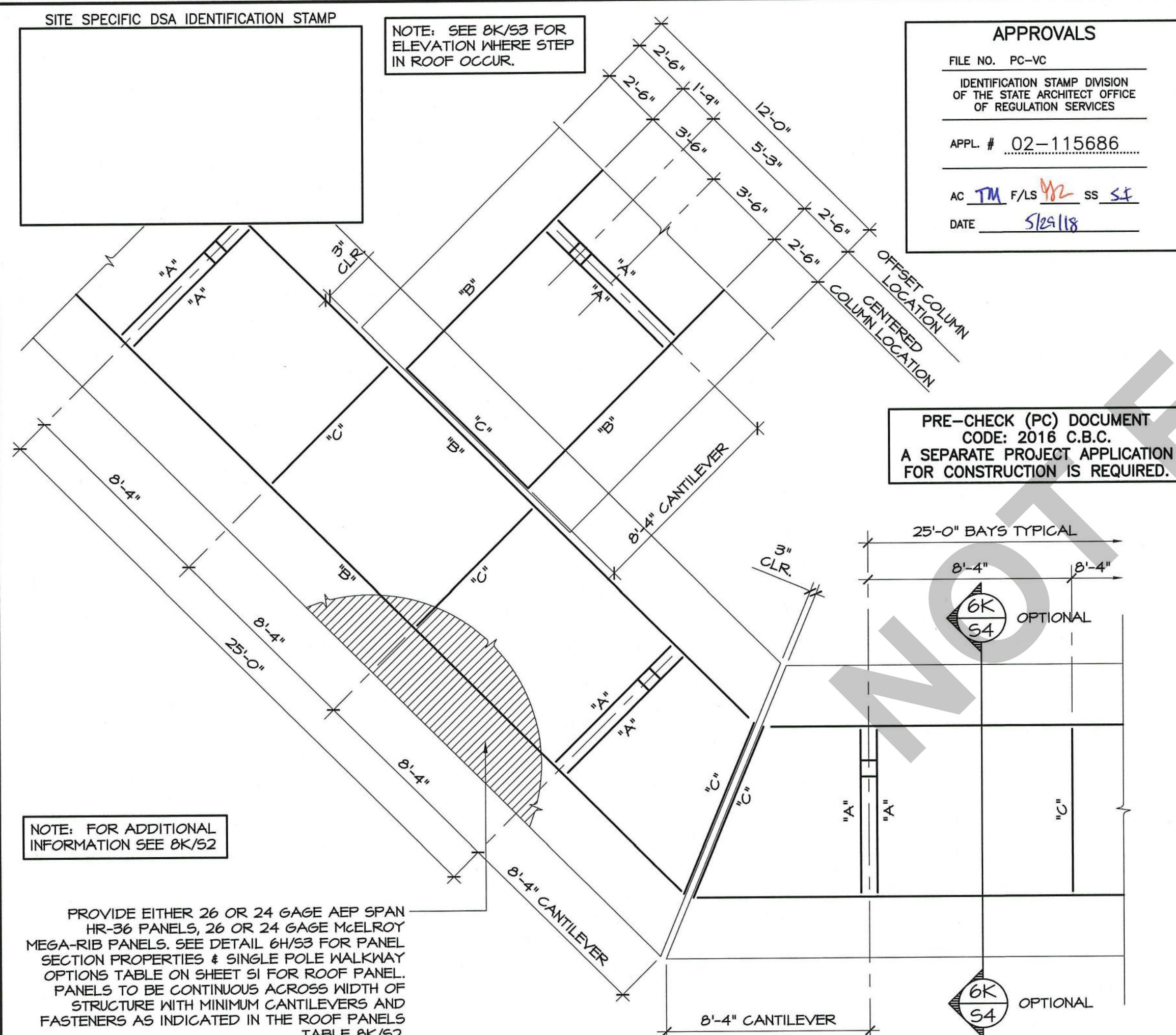
FOUNDATION PLAN

SCALE $\frac{1}{4}" = 1'-0"$

ROOF FRAMING PLAN

SCALE $\frac{1}{4}" = 1'-0"$

4K



ROOF FRAMING PLAN @ 45° OPTION

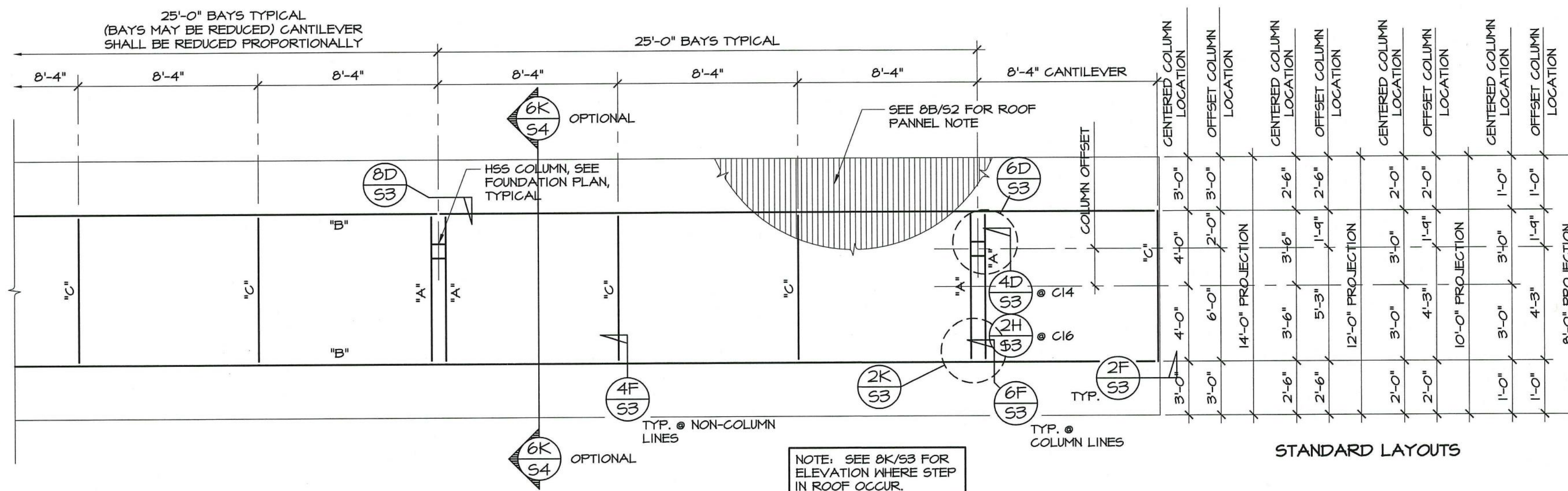
SCALE $\frac{1}{4}" = 1'-0"$

8B

ROOF FRAMING PLANS

ROOF PANEL TABLE					(1, 3, 4, & 6)
ROOF PITCH	LOAD	MINIMUM REQUIRED CANTILEVER LENGTHS @ 6'-0" CENTER SPAN (3)	MINIMUM REQUIRED CANTILEVER LENGTHS @ 7'-0" CENTER SPAN (3)	MINIMUM REQUIRED CANTILEVER LENGTHS @ 8'-0" CENTER SPAN (3 & 5)	SCREWS PER PANEL PER SUPPORT
2:12 MAX.	20psf LL/SL 30psf SL	NO CANTILEVER REQUIRED	1'-0" MIN. 1'-6" MIN.	0'-6" MIN. 1'-6" MIN.	(4)- #12-14x1" LONG W/ 1/2"Ø THINSEAL WASHERS
4:12 MAX.	20psf LL/SL 30psf SL	0'-8" MIN	1'-6" MIN. 2'-0" MIN.	1'-4" MIN. 2'-0" MIN.	(5)- #12-14x1" LONG W/ 1/2"Ø THINSEAL WASHERS

1. SEE 8K/SI FOR ROOF PANEL OPTIONS AND 6H/S3 FOR PROFILES & PROPERTIES.
2. $\frac{1}{4}$ IN 12 MINIMUM.
3. WHEN NOT USING STANDARD LAYOUTS SHOW ON ROOF FRAMING PLAN BELOW, THE ROOF PANELS MUST MEET THE MINIMUM REQUIRED CANTILEVER LENGTHS.
4. LENGTHS OF CANTILEVERS MUST BE EQUAL EACH SIDE, TYPICAL.
5. MUST USE 24 GAGE ROOF PANEL OPTIONS @ 8'-0" CENTER SPAN LAYOUT OPTION.
6. MAXIMUM ALLOWABLE CANTILEVER AT 24 GAGE ROOF PANELS IS 3'-0". IN NO CASE SHALL THE ROOF PANEL LENGTH EXCEED THE $\frac{1}{4}$ "-0" MAXIMUM ROOF PROJECTION.



STANDARD LAYOUTS

NOTE: SEE 8K/S3 FOR
ELEVATION WHERE STEP
IN ROOF OCCUR.

COLUMN LINES

erard
America

TYPE: SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

SUBJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

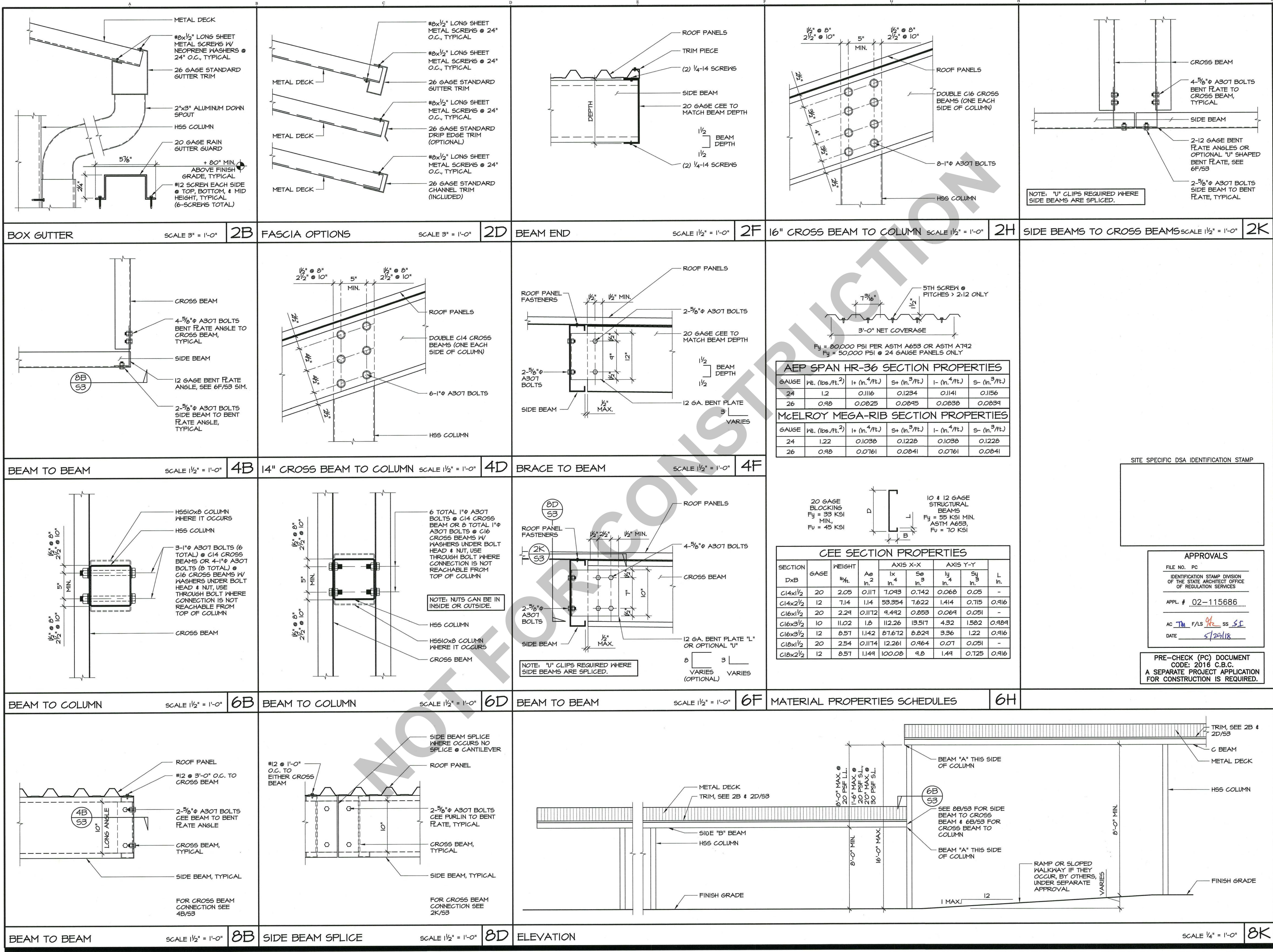
DWN BY: T.E.H.	CHKD BY: G.B.H.
DATE: 5/25/18	

PROJECT NO: 16320
DRAWING TITLE
ROOF FRAMING PLANS

SHEET NUMBER

S2

OF 4 SHEETS



SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE
SECTION
TYPICAL ELEVATION
DETAILS

SHEET NUMBER

S3

OF 4 SHEETS

2374 WEST WHITEDALE
VISALIA, CA 93277

Grand
Glover and Associates
STRUCTURAL ENGINEERS
(559) 734-6675
FAX (559) 734-5332
Email: gloverse@gmail.com

CT: SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY: T.E.H.	CHKD BY: G.B.H.
-------------------	--------------------

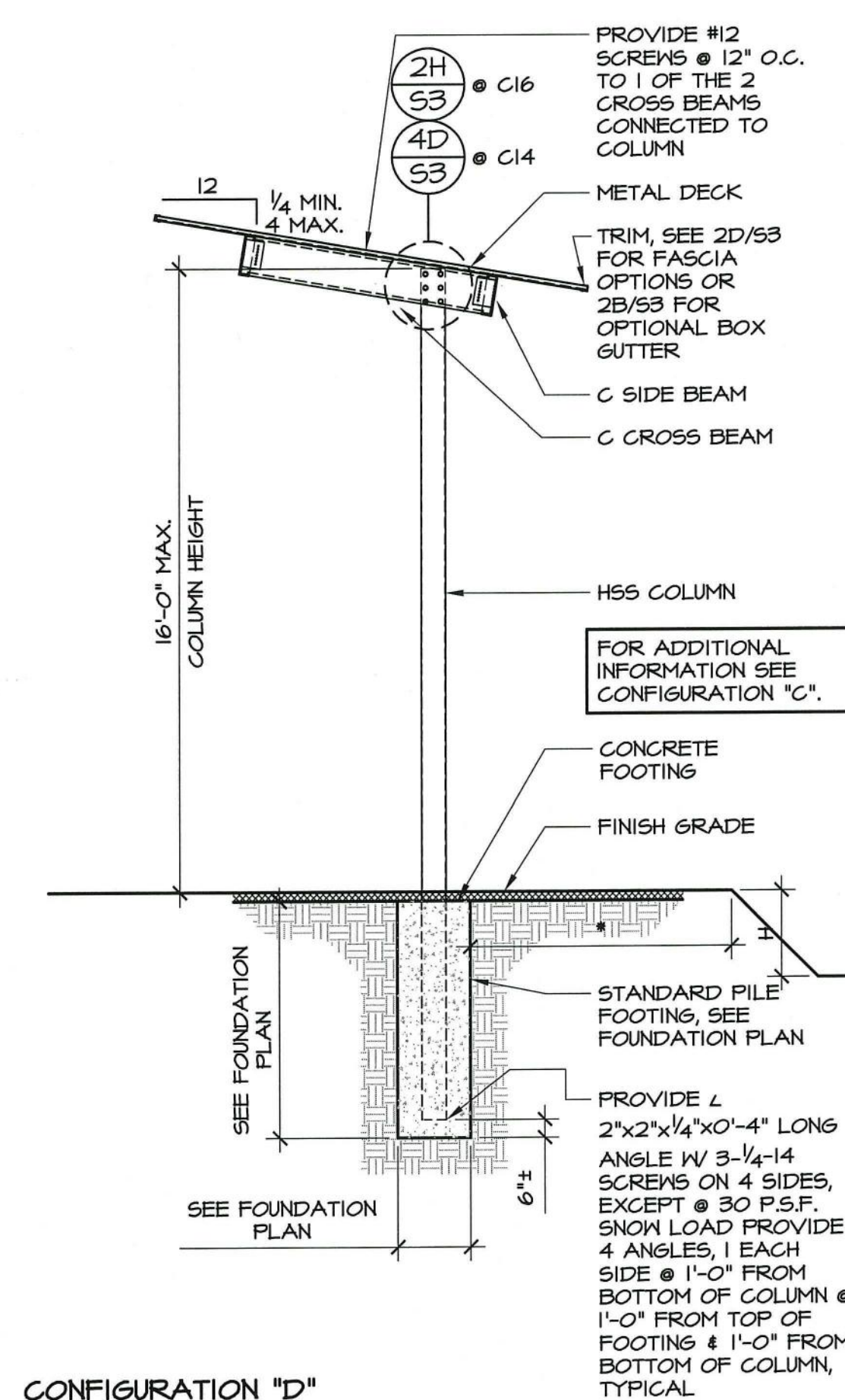
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PROJECT NO: 16320

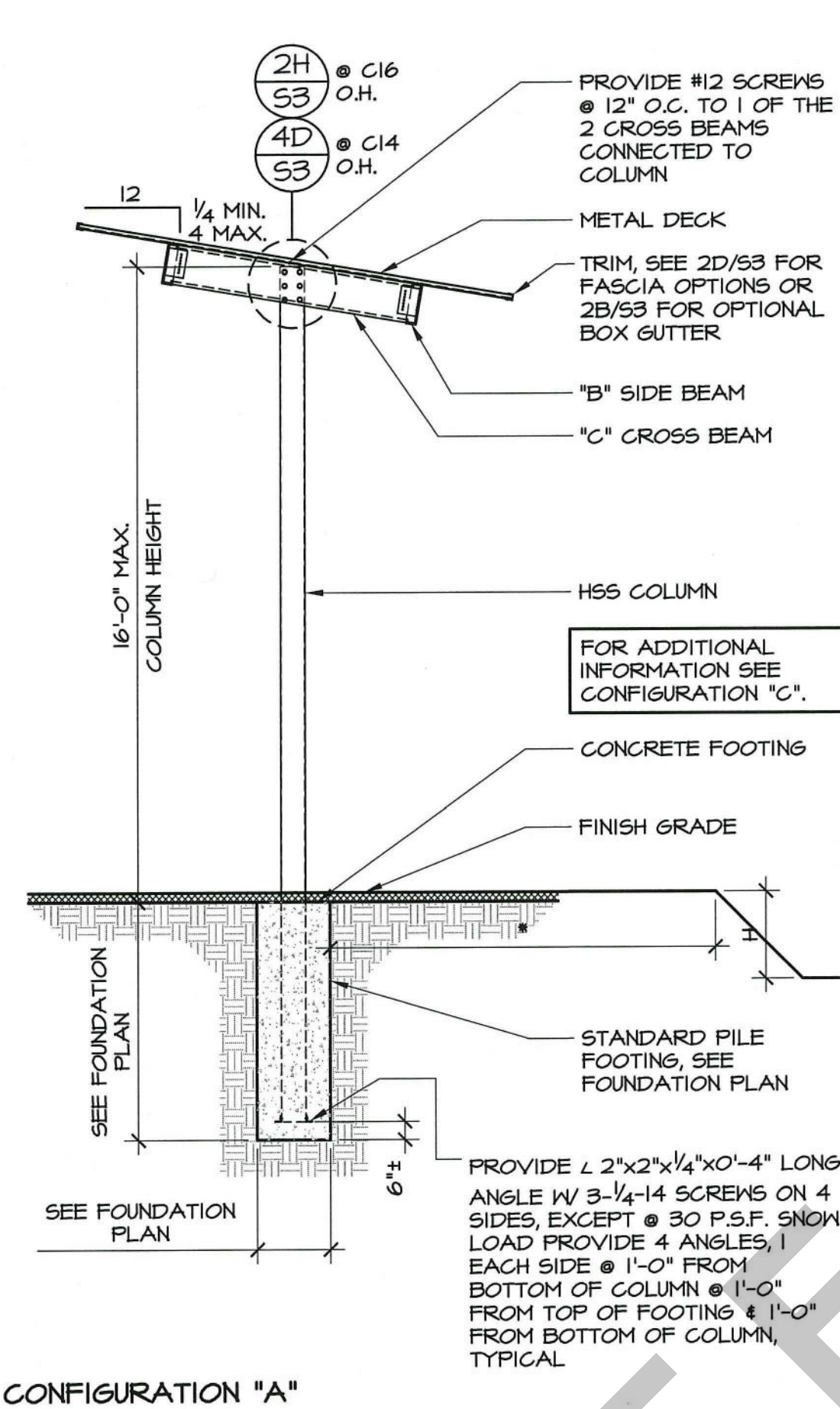
DRAWING TITLE

SECTION
TYPICAL ELEVATION
DETAILS

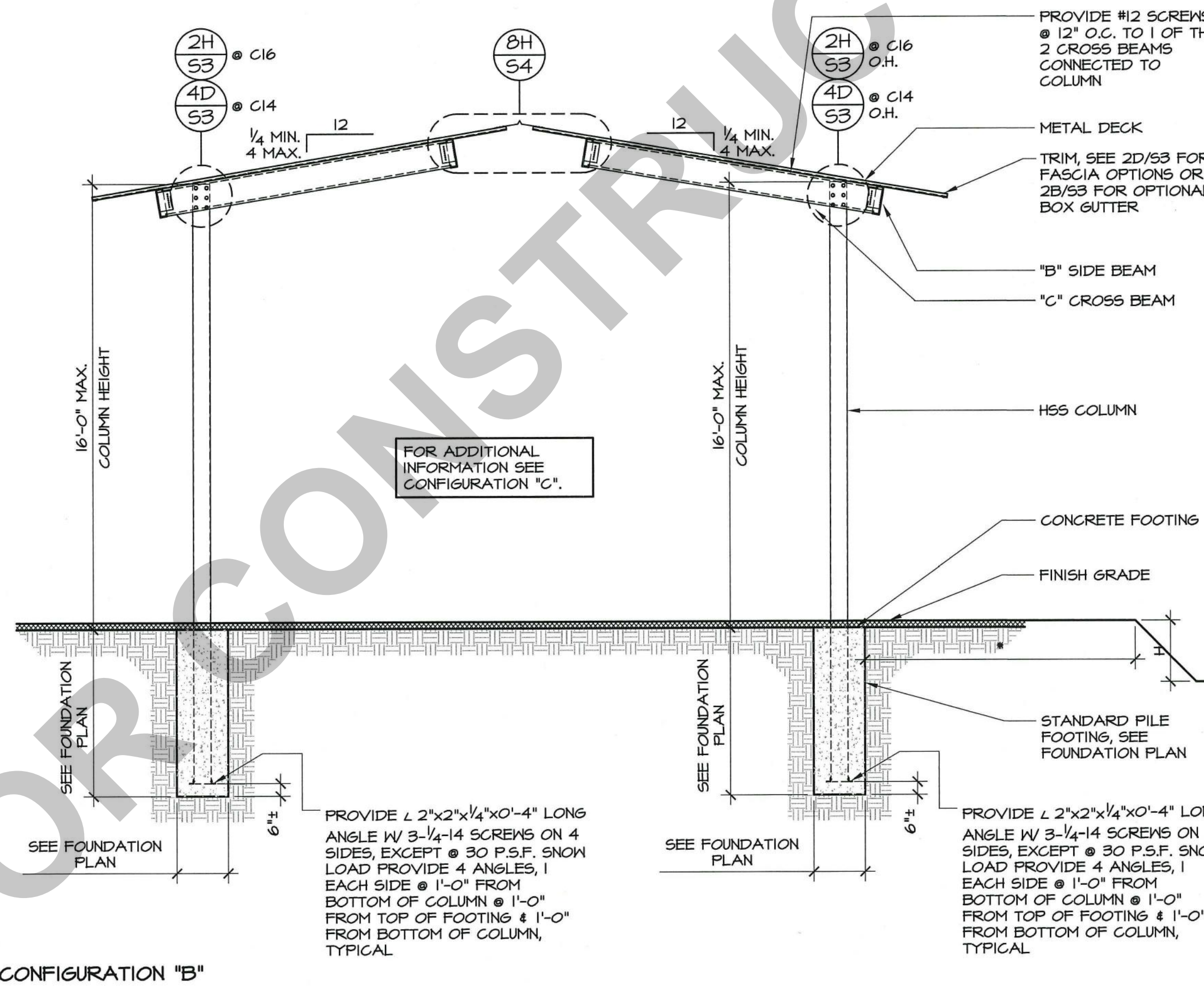
SHEET NUMBER
S4
OF 4 SHEETS



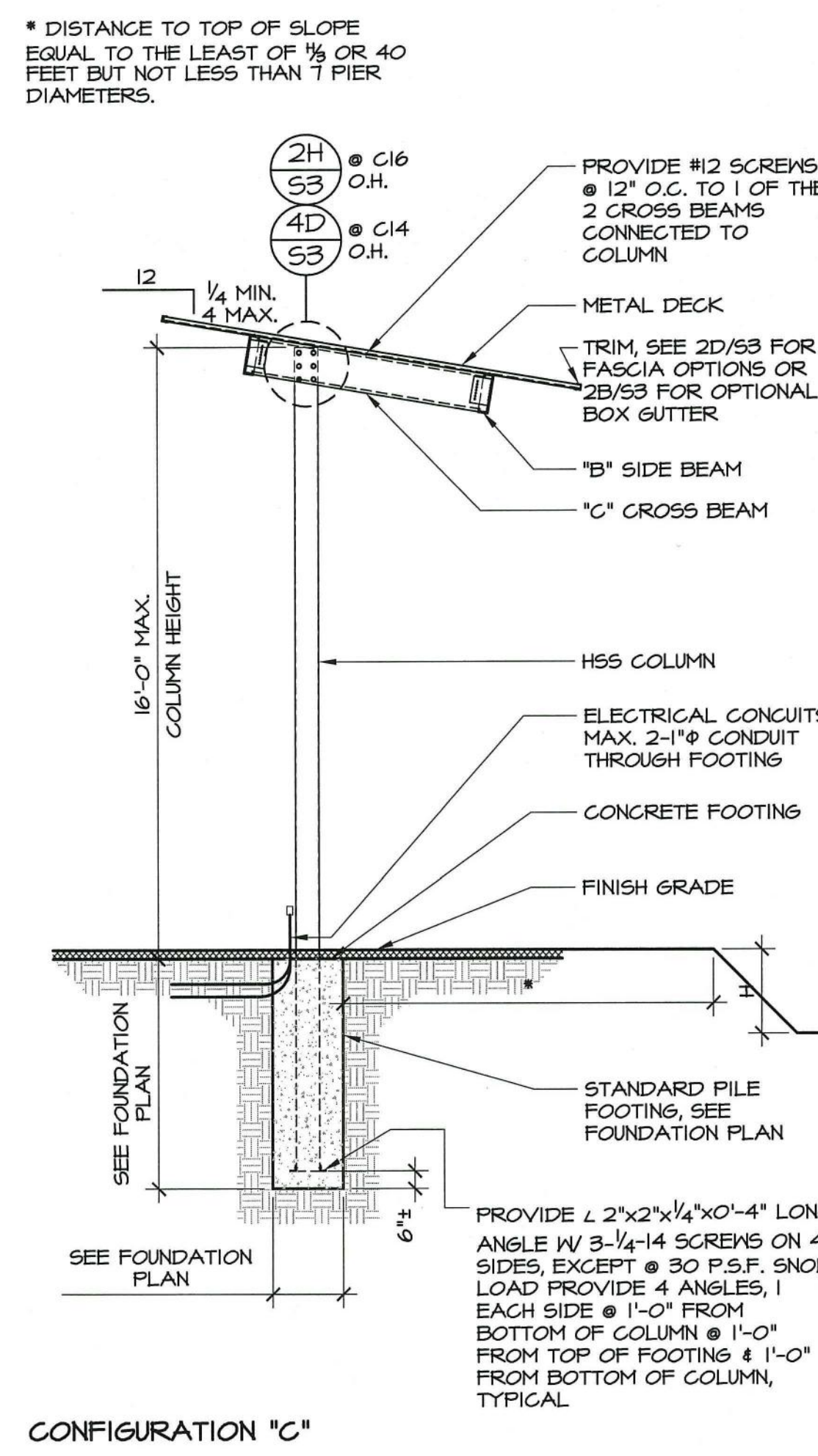
CONFIGURATION "D"



CONFIGURATION "A"



CONFIGURATION "B"



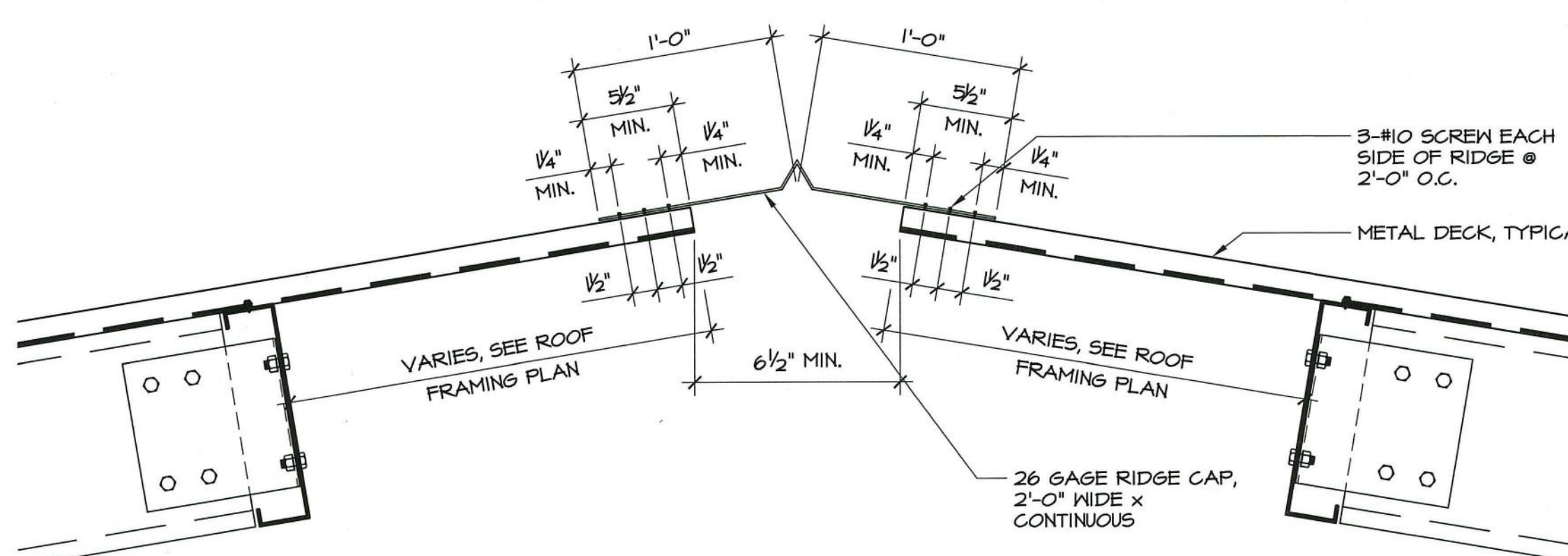
CONFIGURATION "C"

* DISTANCE TO TOP OF SLOPE
EQUAL TO THE LEAST OF $\frac{1}{3}$ OR 40
FEET BUT NOT LESS THAN 7 PIER
DIAMETERS.

SECTIONS

SCALE $\frac{1}{4}" = 1'-0"$ 6K

6K



RIDGE CAP

SCALE $1\frac{1}{2}" = 1'-0"$ 8t

" 84

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC

IDENTIFICATION STAMP DIVISION
OF THE STATE ARCHITECT OFFICE
OF REGULATION SERVICES

APPL # 02-115686

AC TM F/LS 9/2 SS _____
DATE _____

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION
FOR CONSTRUCTION IS REQUIRED

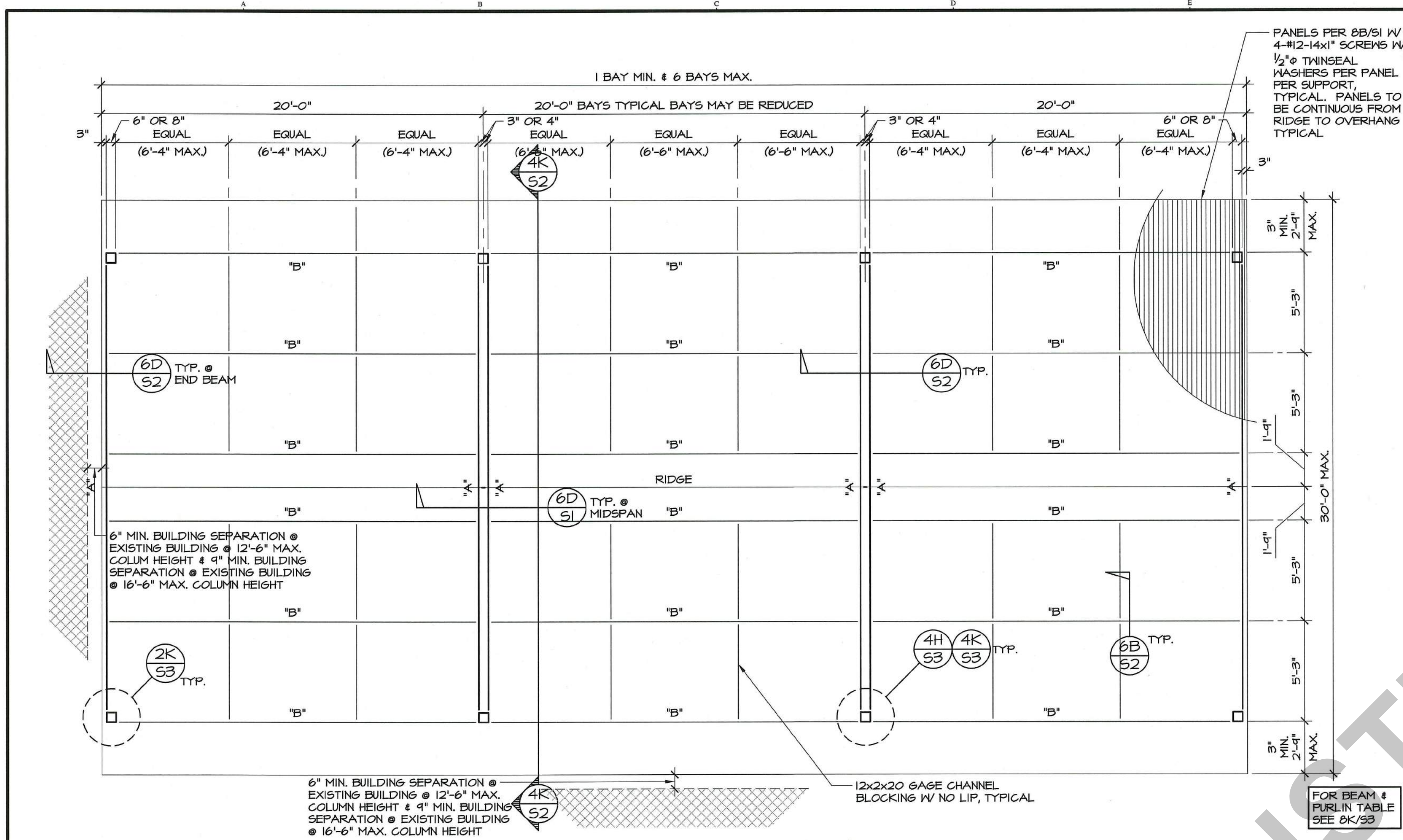
Gable Lunch Shelter Cover DSA Pre-Checked Plans NO STAMP

Stamped plans to be provided if approved equal is approved.

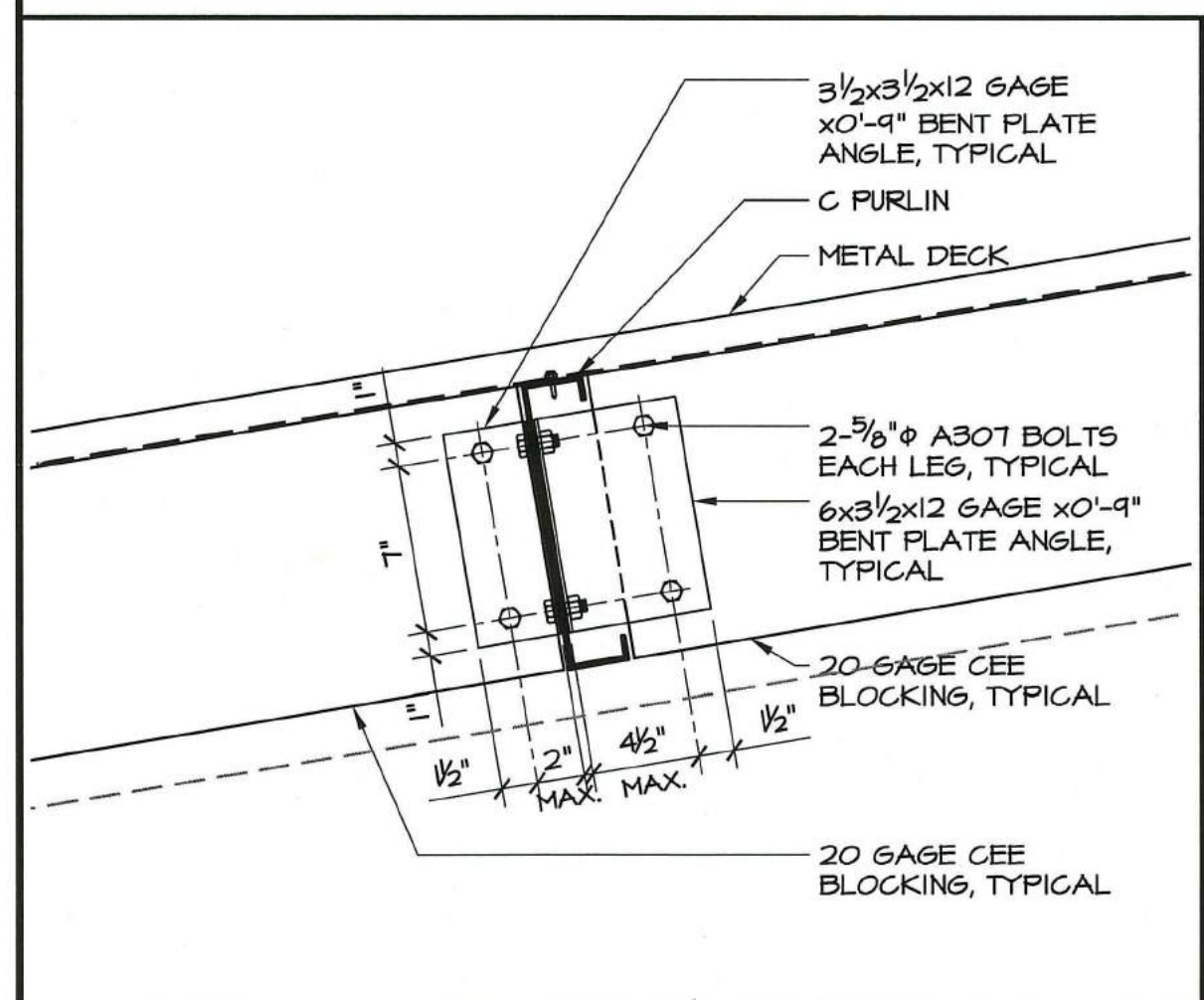
Gable Lunch Shelter Covers Used On:

- **Piedmont Elementary Structure 'A'**
-

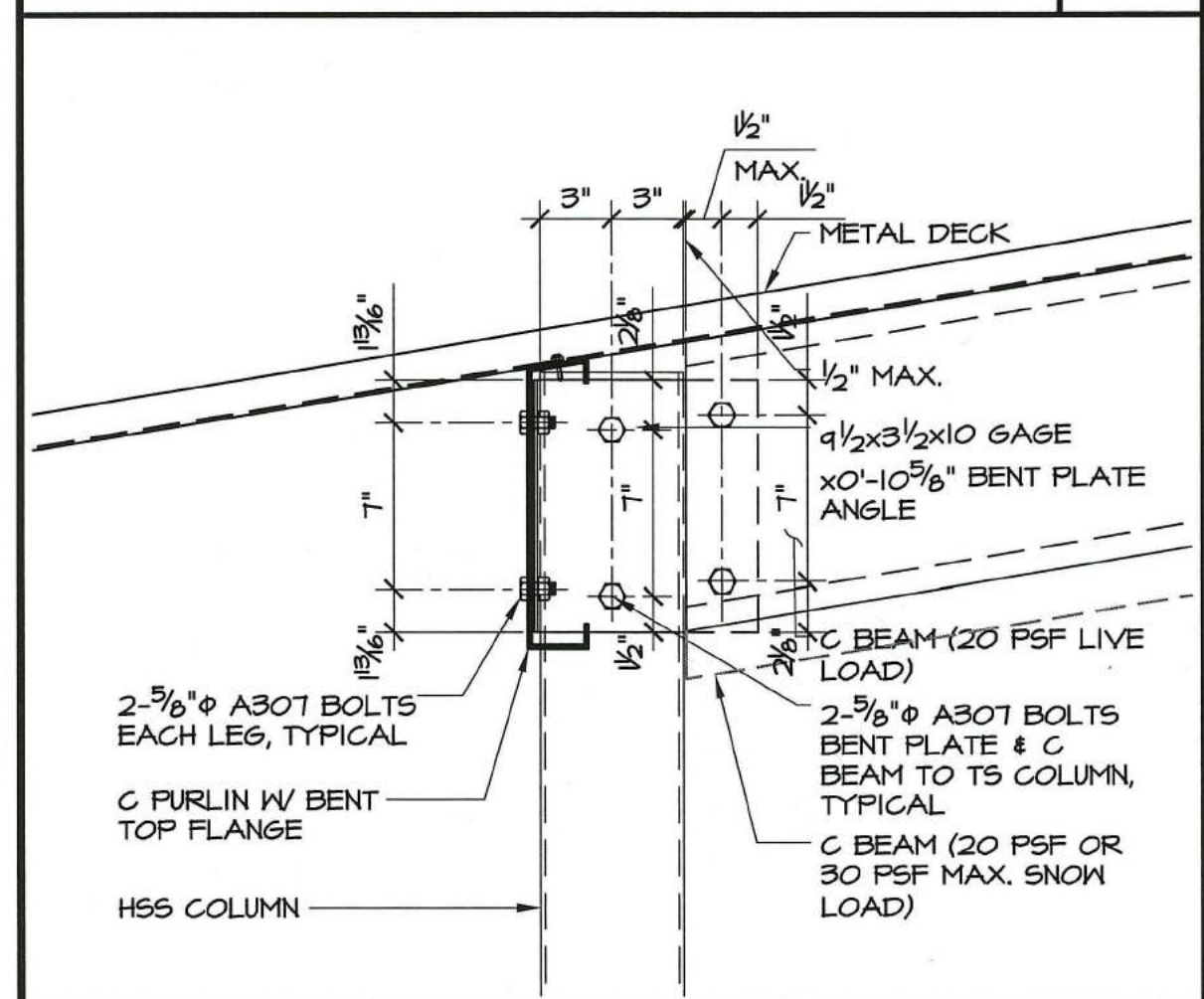
C:\Users\cod\1\Documents\Drawings\Projects\16324\S02Fndwg May 25 2018 11:12am Gable School Lunch Shelter - Valley School Shelters



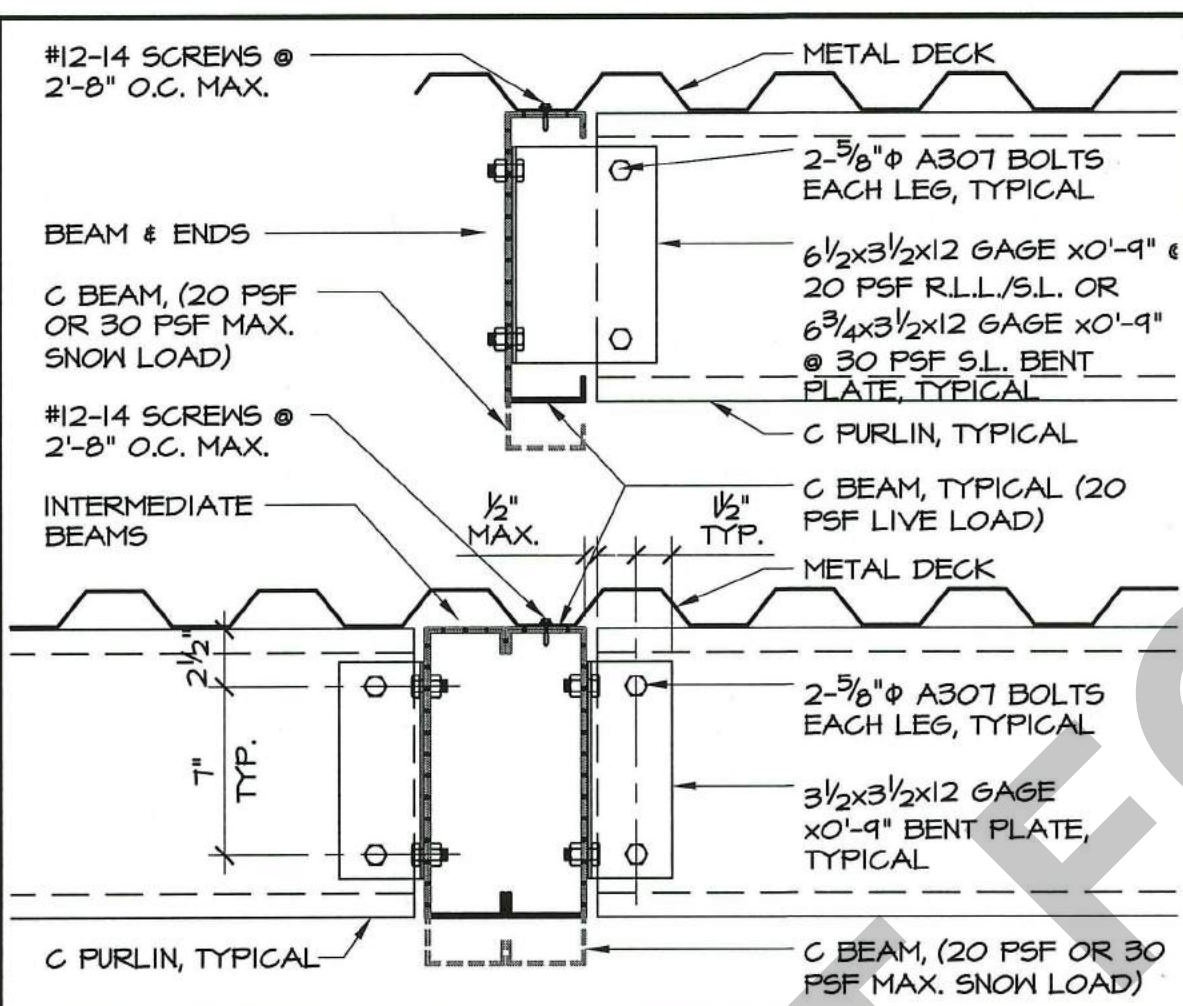
ROOF FRAMING PLAN



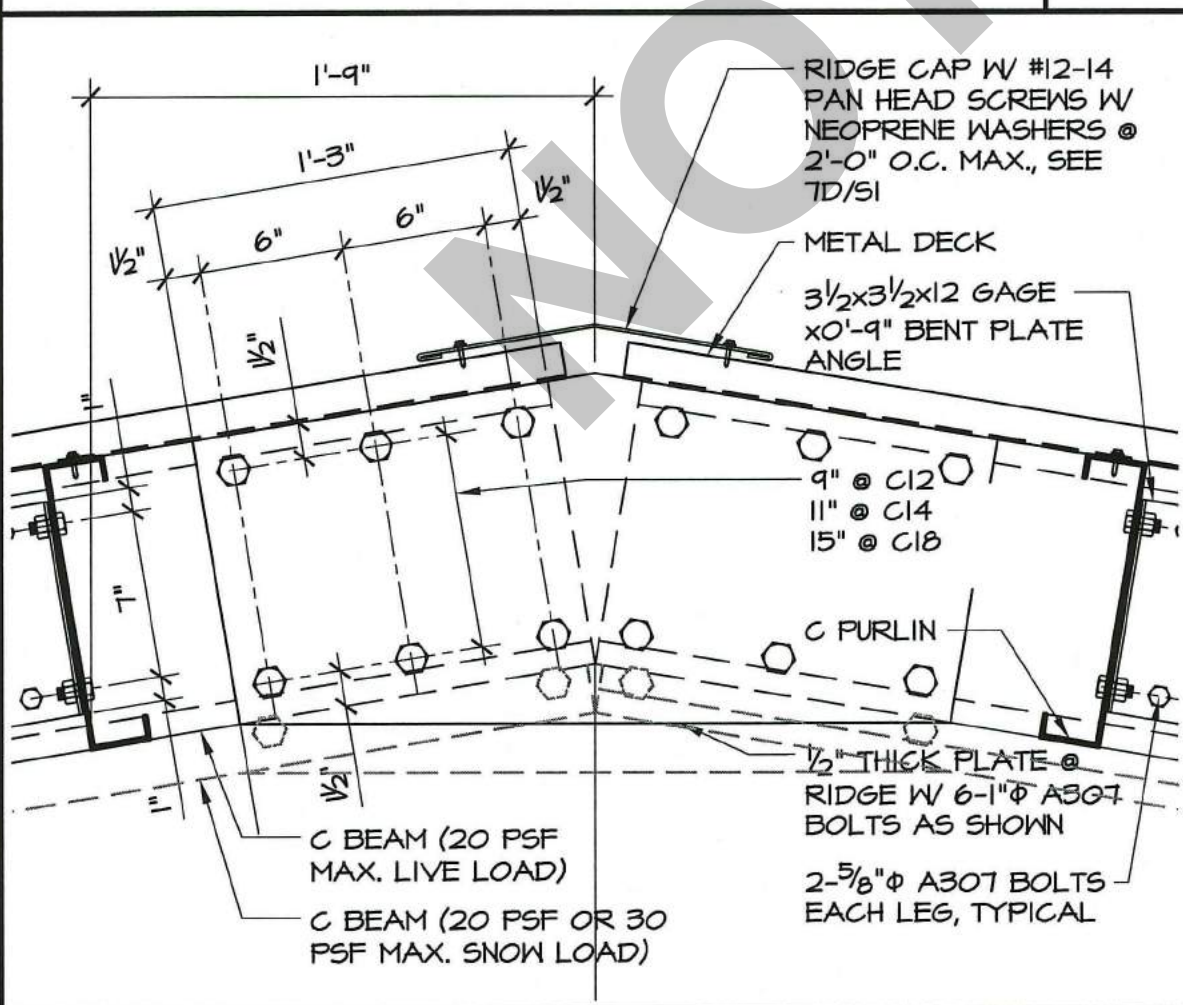
BLOCKING TO PURLIN



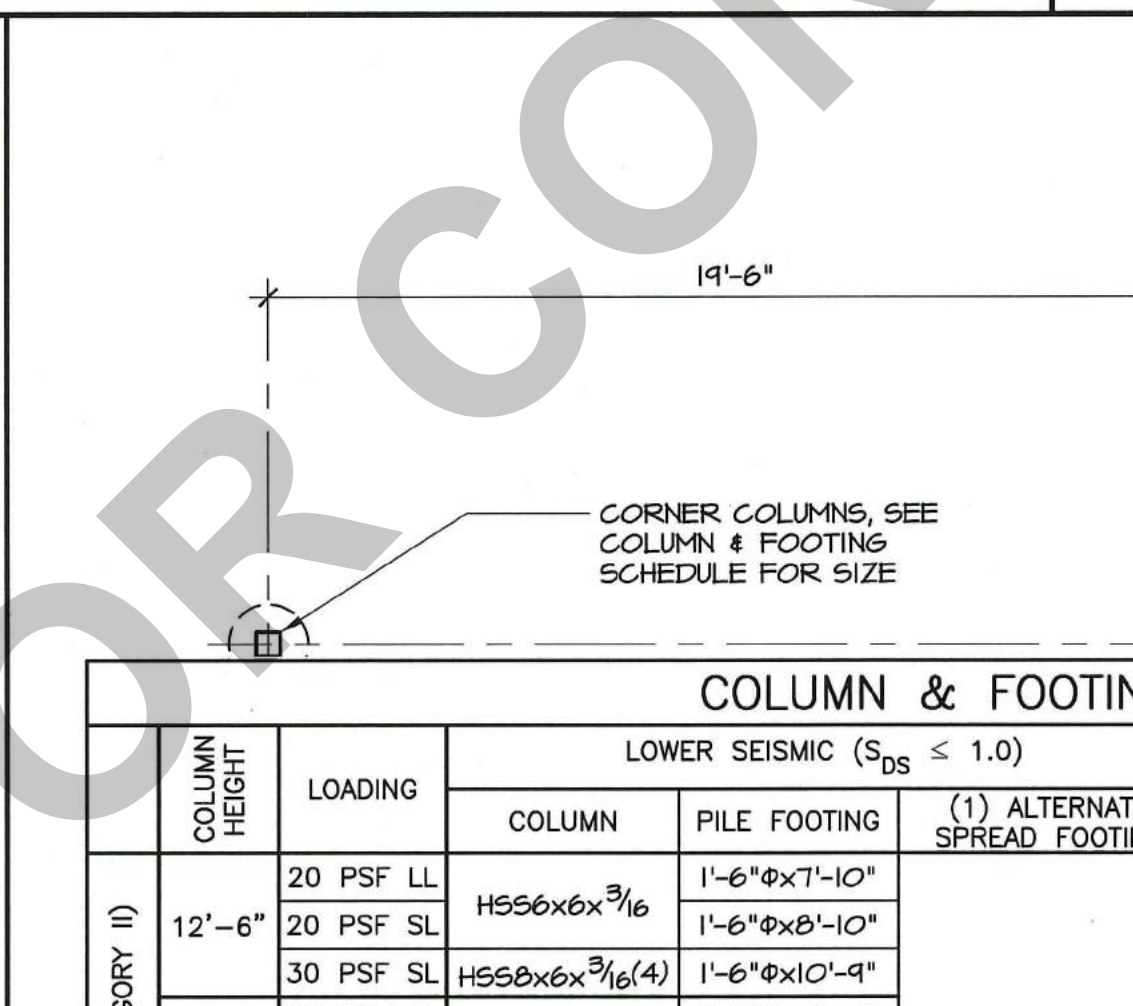
BEAM TO COLUMN



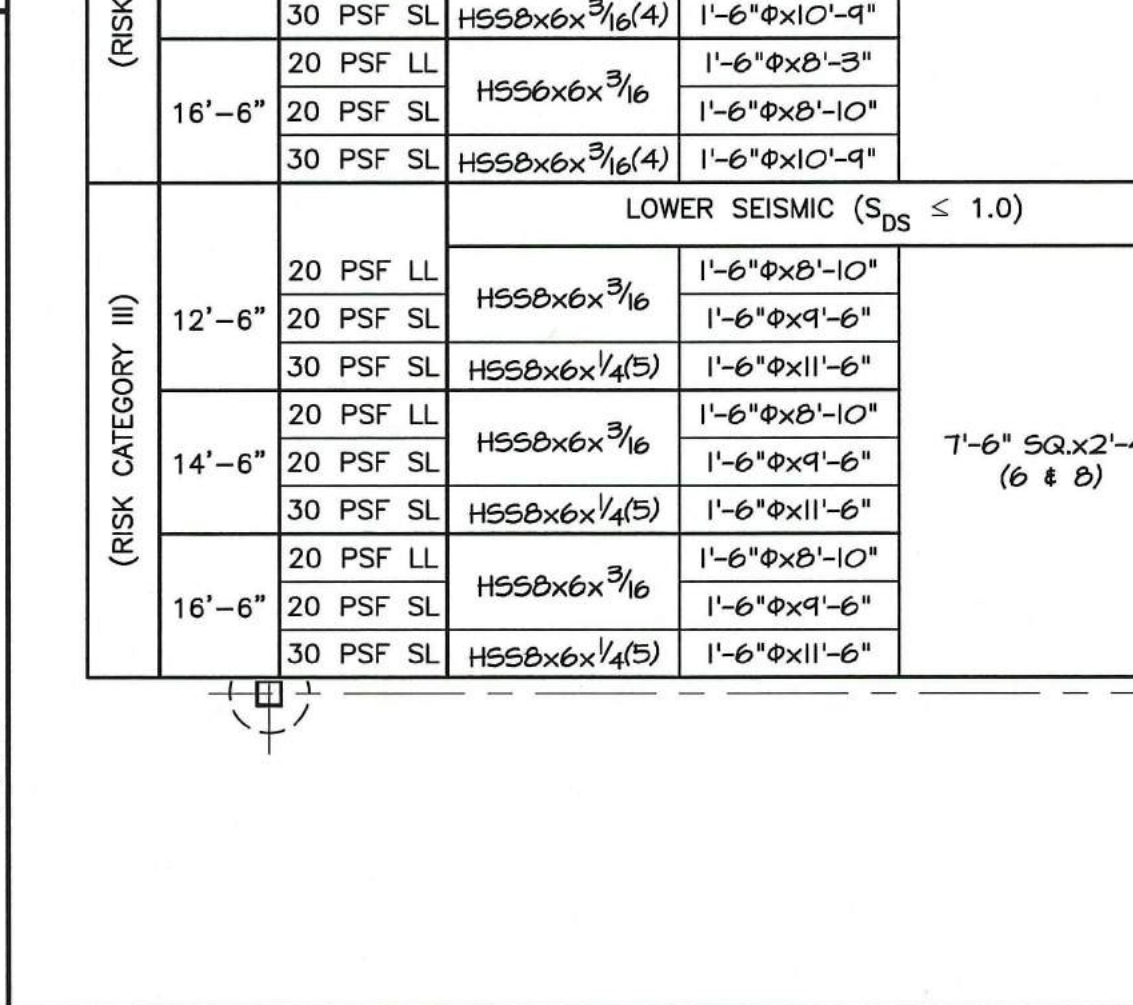
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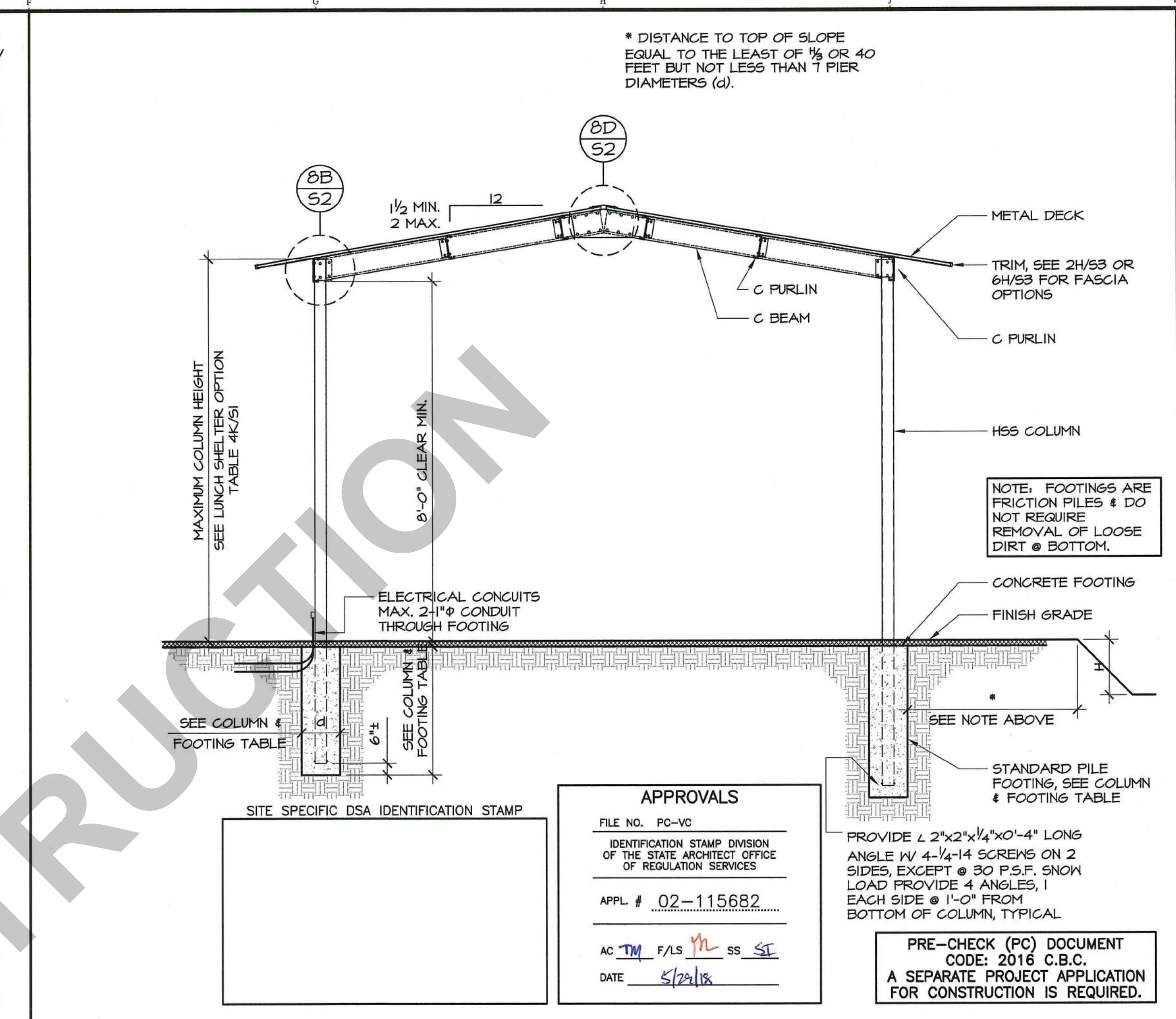
BEAM RIDGE CONNECTION



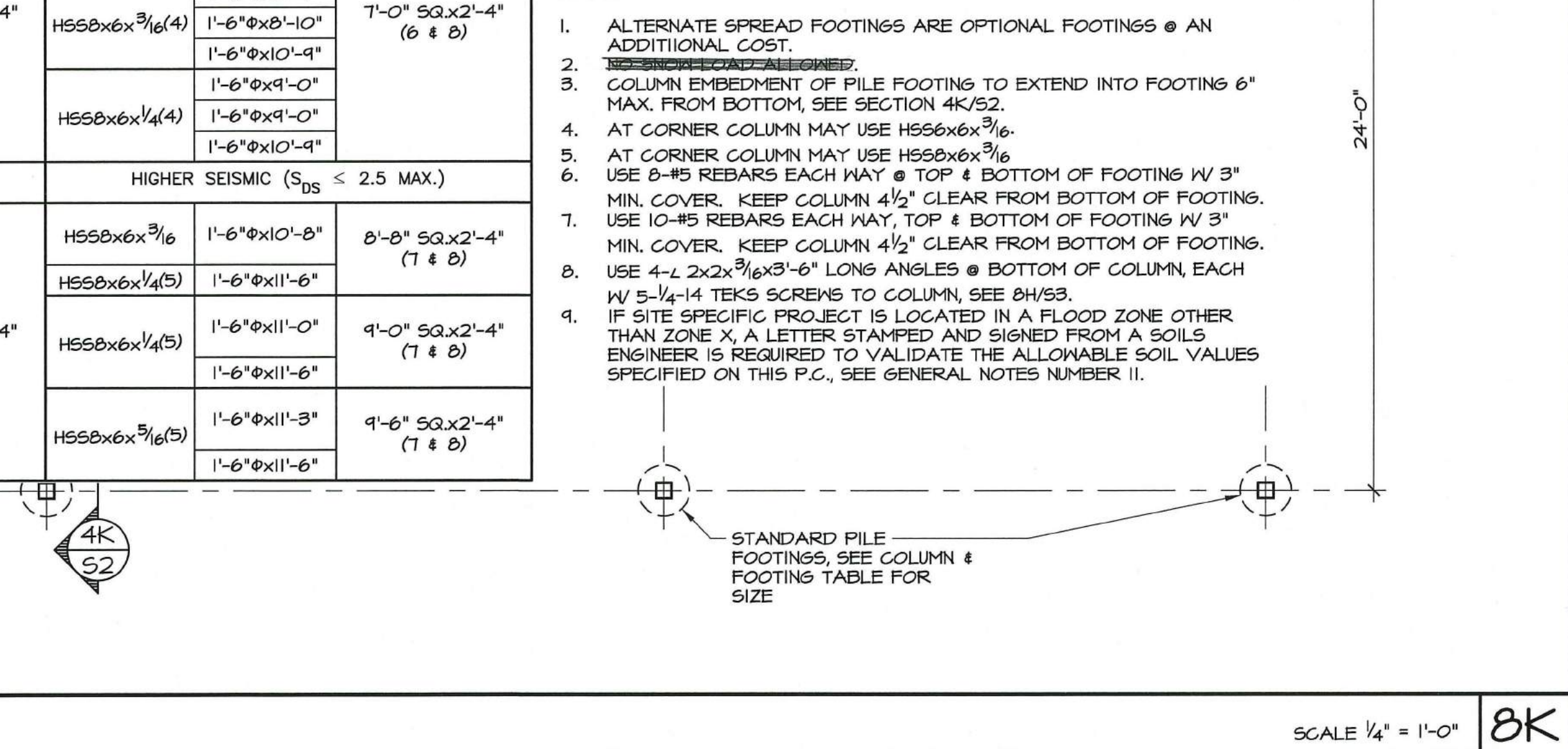
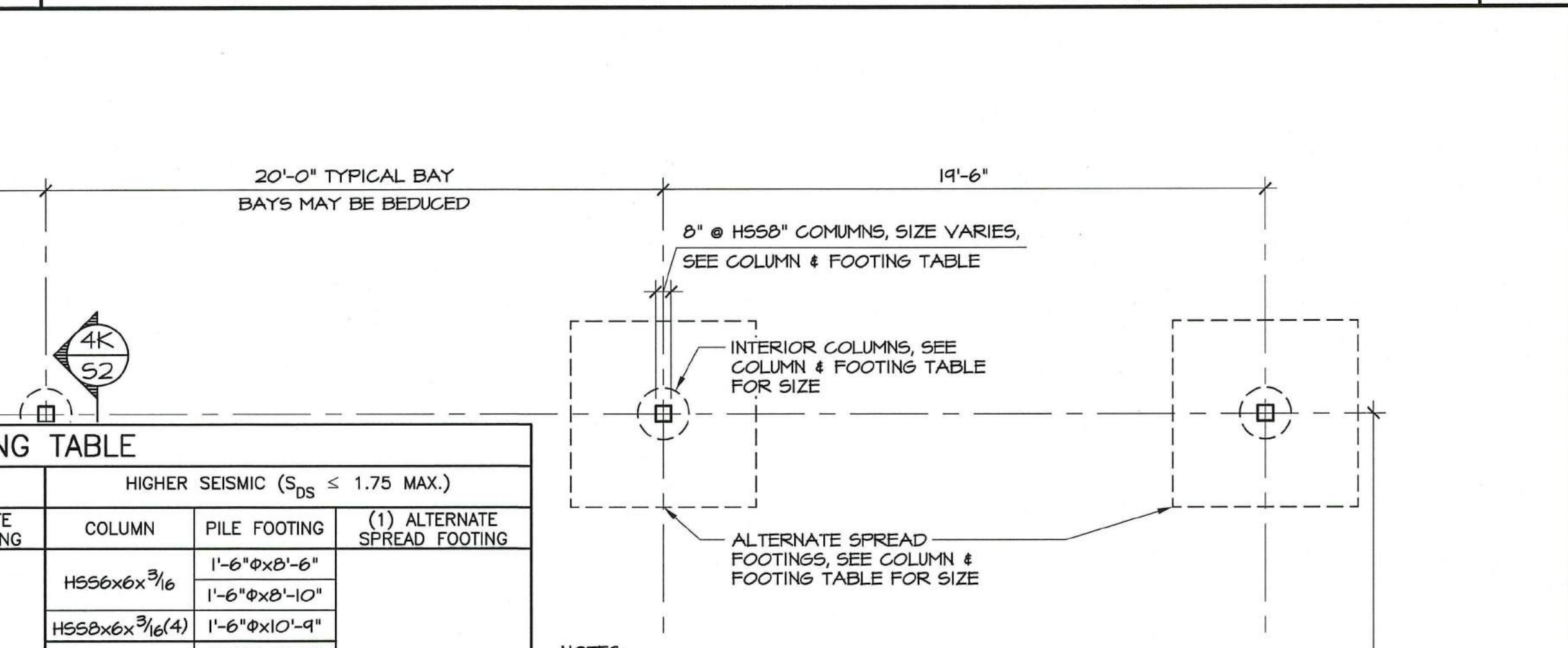
FOUNDATION PLAN



FOUNDATION PLAN



SECTION



SECTION

COLUMN & FOOTING TABLE						
RISK CATEGORY II	COLUMN HEIGHT	LOADING	LOWER SEISMIC ($S_{DS} \leq 1.0$)		HIGHER SEISMIC ($S_{DS} \leq 1.75$ MAX.)	
			COLUMN	PILE FOOTING	COLUMN	PILE FOOTING
12'-6"	20 PSF LL	20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
14'-6"	20 PSF LL	20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
16'-6"	20 PSF LL	20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
RISK CATEGORY III	12'-6"	20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
14'-6"	20 PSF LL	20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
16'-6"	20 PSF LL	20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF LL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		20 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"
		30 PSF SL	HSS6x6x3/16	1'-6" x 1'-10"	HSS6x6x3/16	1'-6" x 1'-10"

- NOTES:
1. ALTERNATE SPREAD FOOTINGS ARE OPTIONAL FOOTINGS @ AN ADDITIONAL COST.
 2. COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 6" MAX. FROM BOTTOM, SEE SECTION 4K/52.
 3. AT CORNER COLUMN MAY USE HSS6x6x3/16.
 4. AT CORNER COLUMN MAY USE HSS6x6x3/16.
 5. USE 8-#5 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 4 1/2" CLEAR FROM BOTTOM OF FOOTING. USE 10-#5 REBARS EACH WAY, TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 4 1/2" CLEAR FROM BOTTOM OF FOOTING.
 6. USE 4-L 2x2x3/16x3'-6" LONG ANGLES @ BOTTOM OF COLUMN, EACH W/ 5-1/4-14 TKS SCREWS TO COLUMN, SEE 8H/53.
 7. IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C., SEE GENERAL NOTES NUMBER II.

2374 WEST WHITEDALE
VISALIA, CA 93277

(559) 734-6675
FAX (559) 734-5232
Email ghomerstrudengr@sbglobal.net

Grand
Gomer and Associates
STRUCTURAL ENGINEERS

GABLE SCHOOL
LUNCH SHELTER
VALLEY SCHOOL SHELTERS

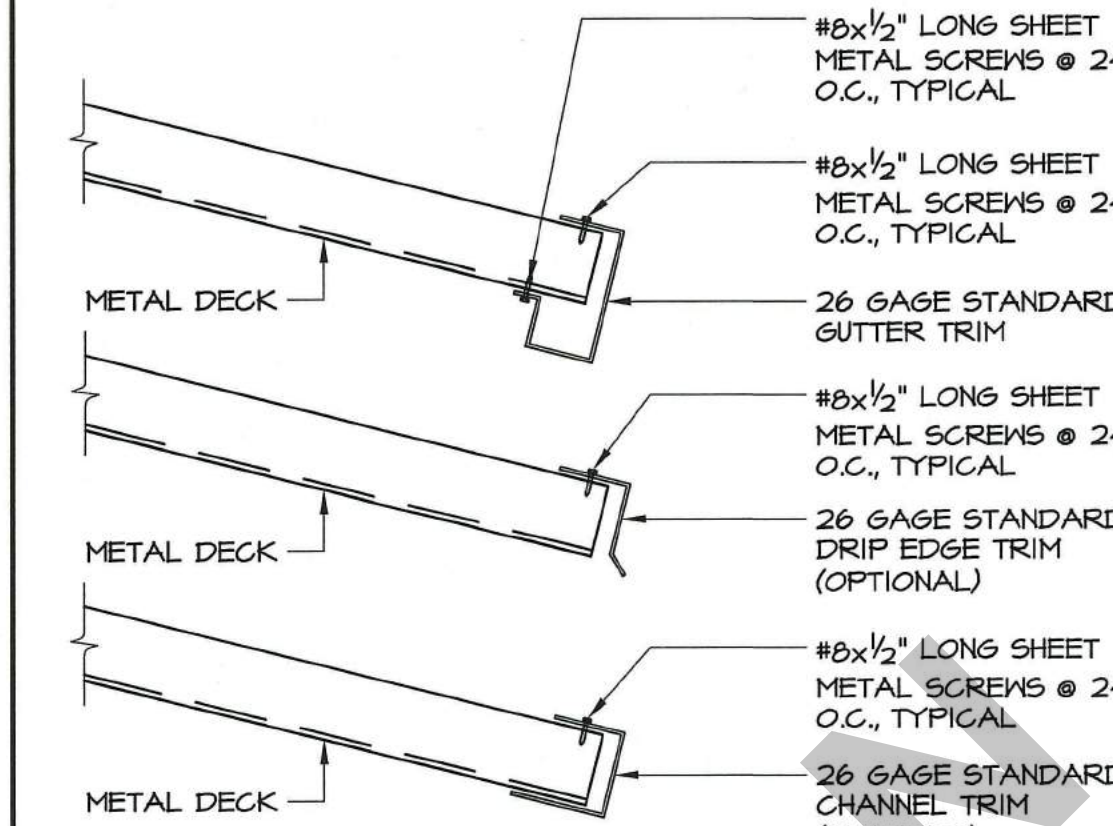
PROJECT:

REVISIONS

NO.	DATE	BY	DESCRIPTION

DWN BY: T.E.H. CHKD BY: G.B.H.
DATE: 5/25/18
PROJECT NO: 16324
DRAWING TITLE
FOUNDATION PLAN
ROOF FRAMING PLAN
SECTION
DETAILS
SHEET NUMBER
S2
OF 3 SHEETS

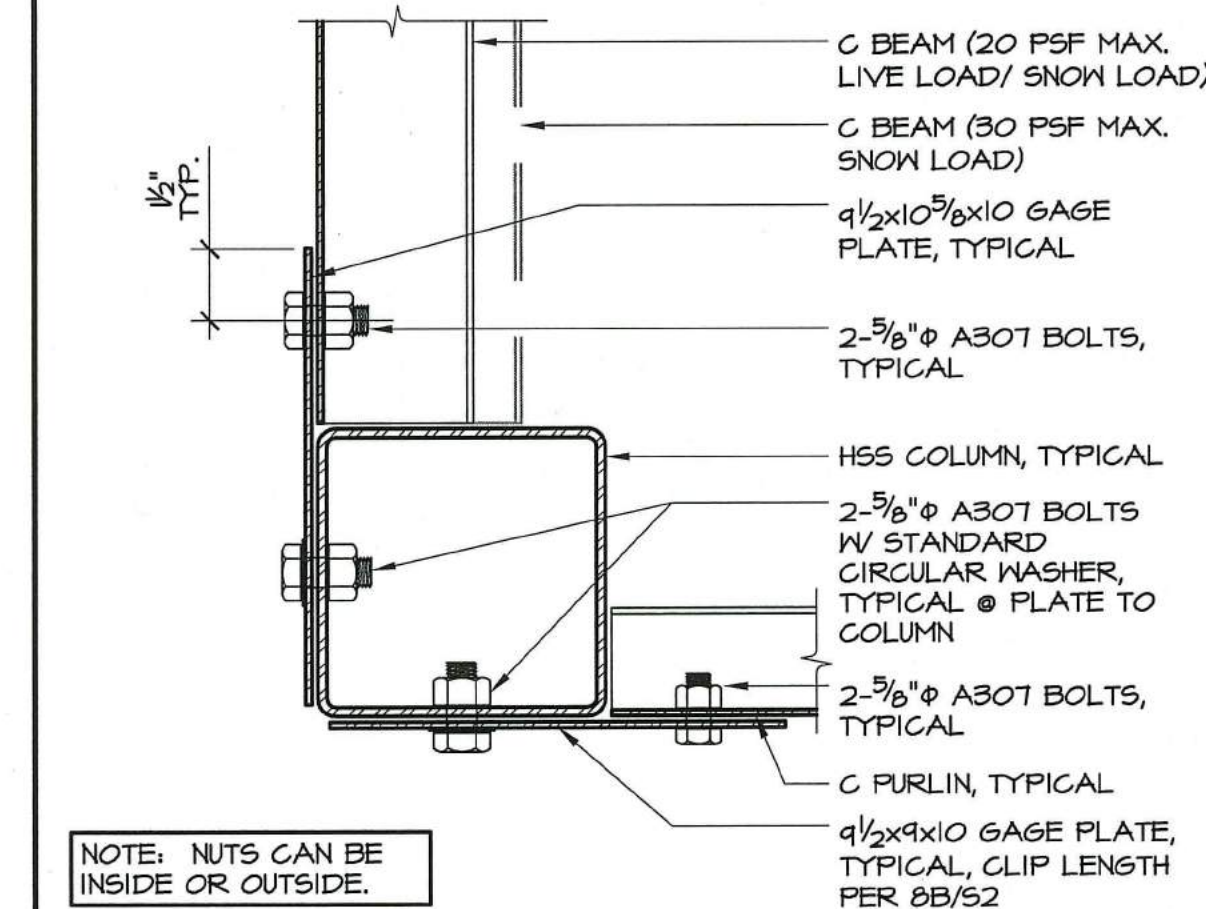
NOT FOR CONSTRUCTION



FASCIA OPTIONS

SCALE 3" = 1'-0"

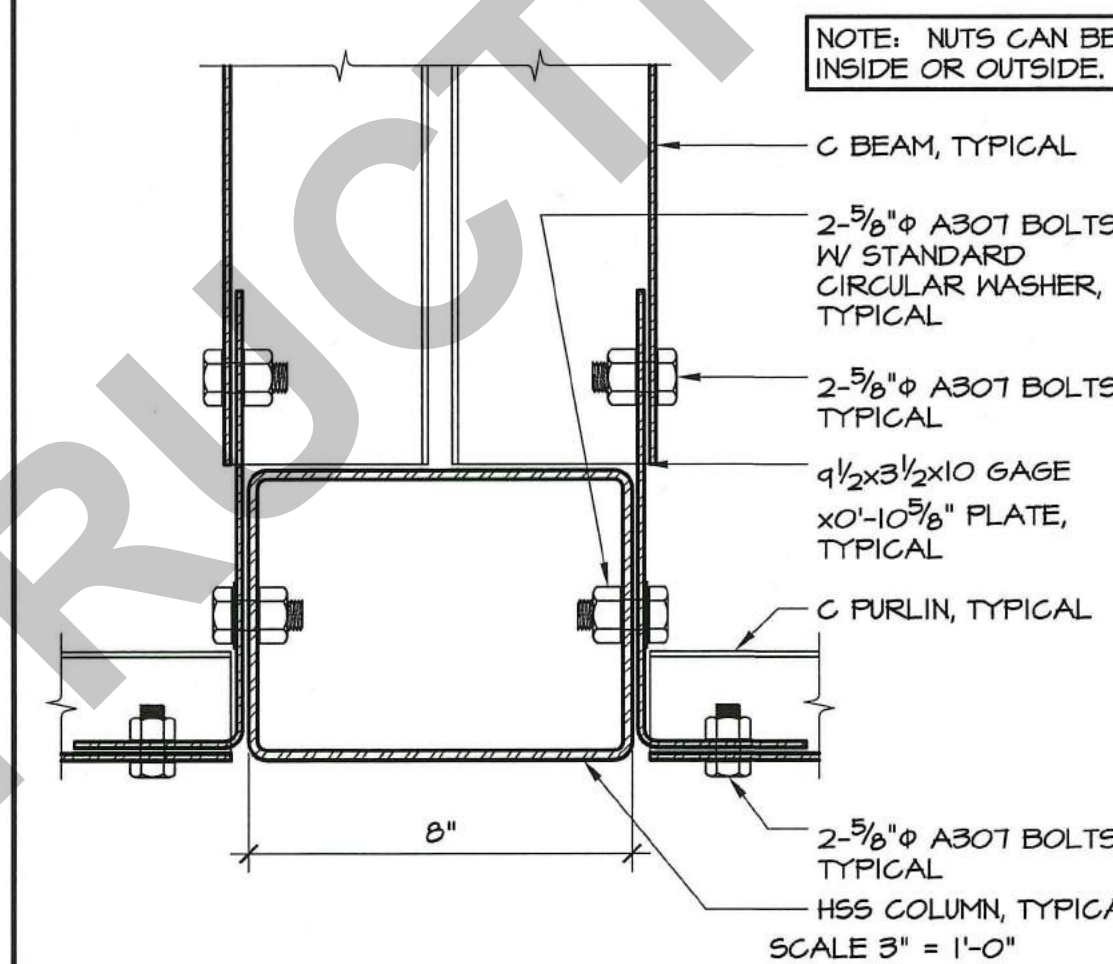
2H



BEAM TO COLUMN @ CORNERS

SCALE 3" = 1'-0"

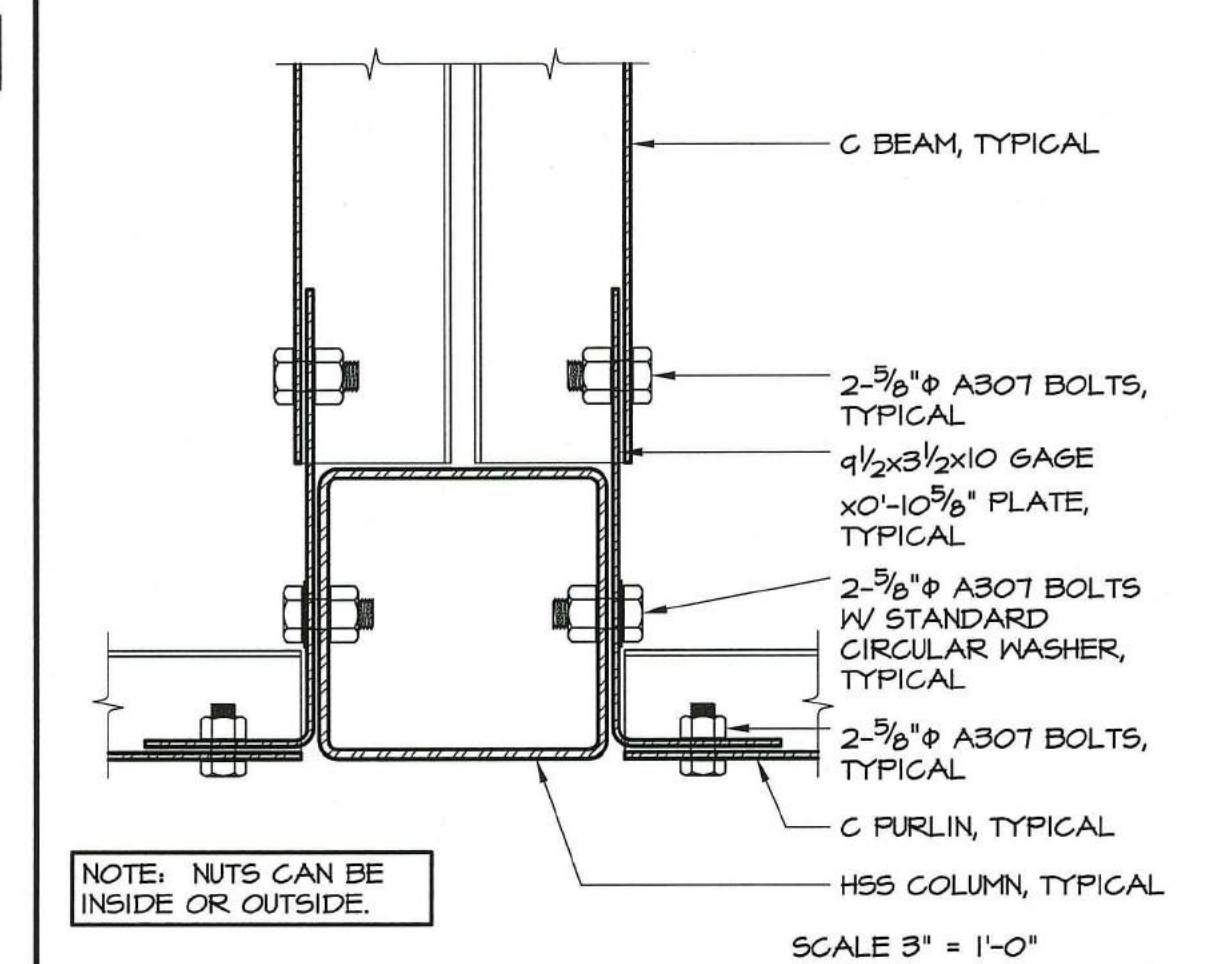
2K



BEAM TO COLUMN @ INTERMEDIATE LOCATIONS

8" COLUMNS

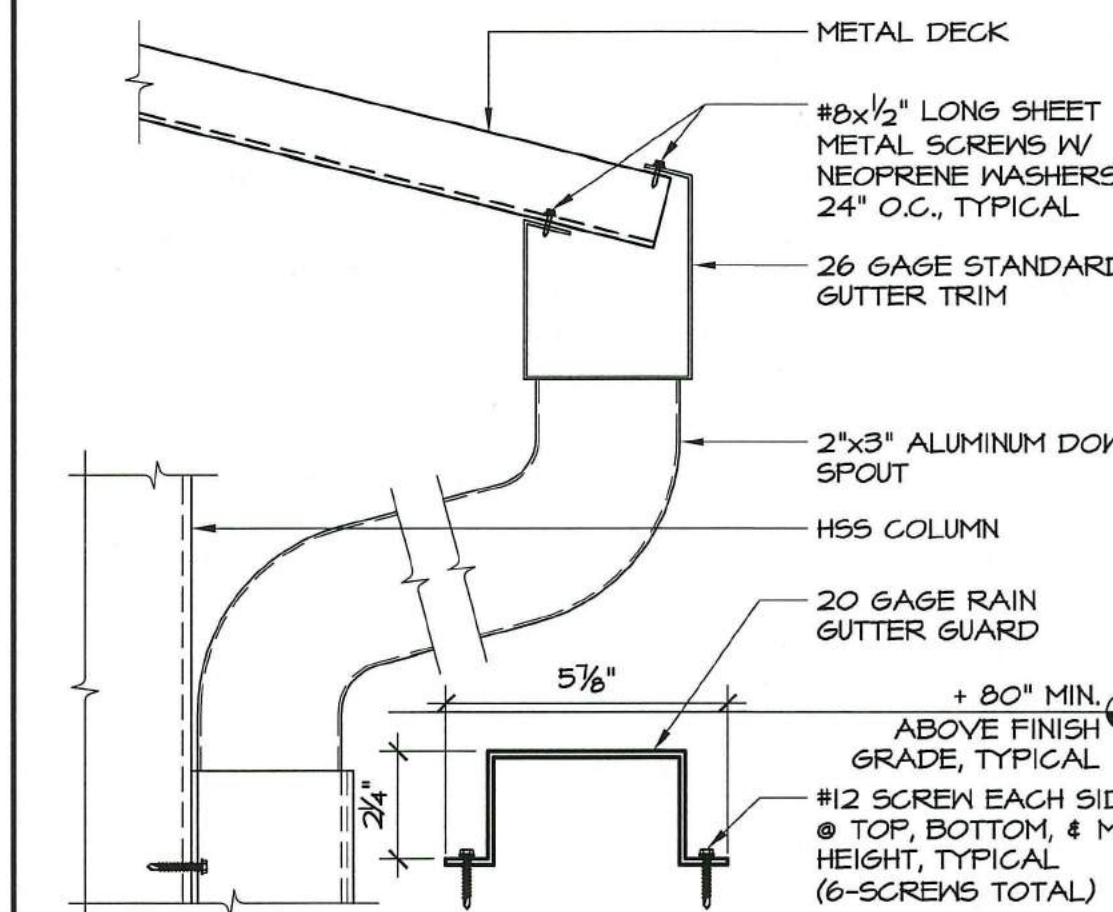
4H



BEAM TO COLUMN @ INTERMEDIATE LOCATIONS

6" COLUMNS

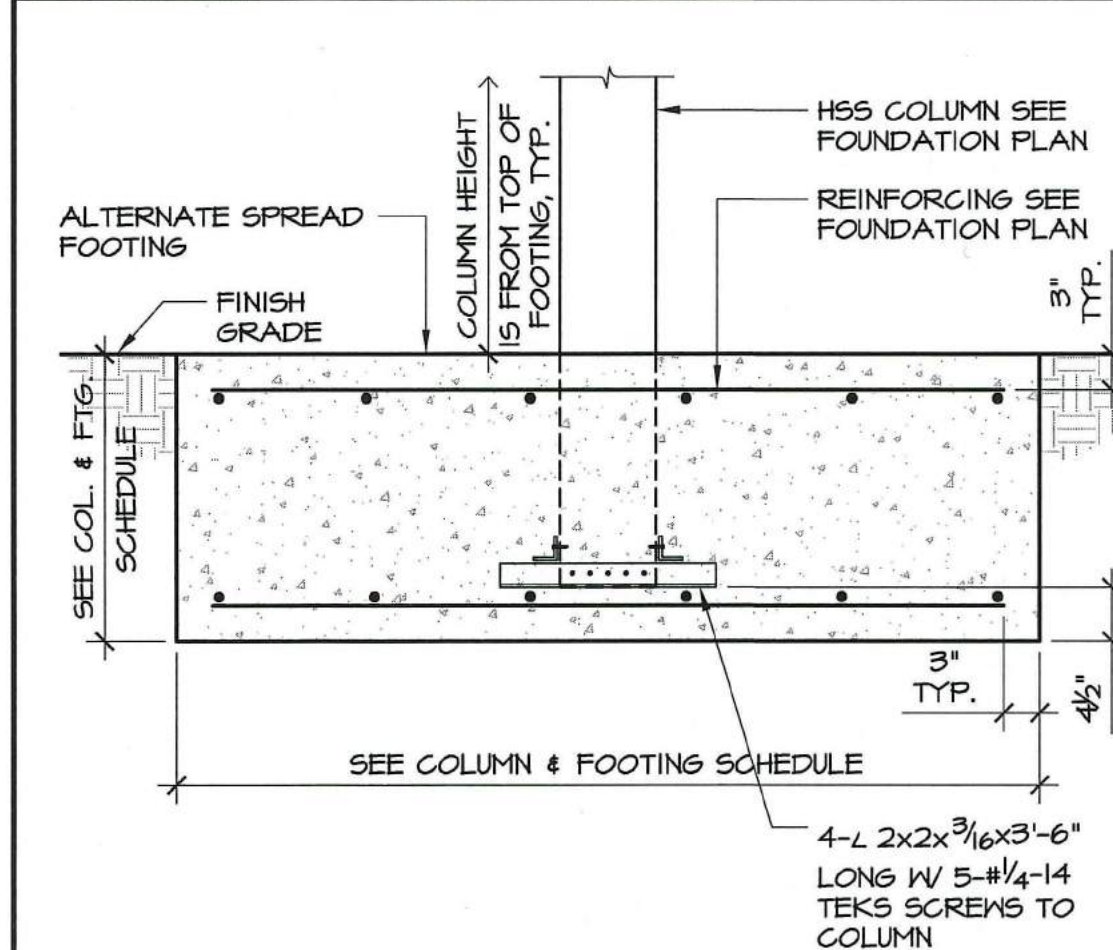
4K



BOX GUTTER

SCALE 3" = 1'-0"

6H



OPTIONAL SPREAD FOOTING @ ADDITIONAL COST

SCALE 3/4" = 1'-0"

8H

BEAM & PURLIN TABLE			
(RISK CATEGORY)	LOADING	BEAM "A"	PURLIN "B"
(RISK CATEGORY I) SEE NOTE 3	20 PSF LL	C12x3x12 GAGE	C12x2 1/4x14 GAGE
	20 PSF SL	C14x3x12 GAGE	C12x2 1/4x14 GAGE
	30 PSF SL	C18x3 1/4x12 GAGE	C12x2 1/4x12 GAGE
(RISK CATEGORY II) SEE NOTE 3	20 PSF LL/SL	C14x3x12 GAGE	C12x2 1/4x12 GAGE
	30 PSF SL	C18x3 1/4x12 GAGE	C12x2 1/4x12 GAGE

- NOTES:
- RISK CATEGORY IS PER CBC 1604A.5.
 - SEE 8B/5I FOR SECTION PROPERTIES OF BEAMS.
 - SEE SITE SPECIFIC TABLE TO DETERMINE RISK CATEGORY.

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC-VC
IDENTIFICATION STAMP DIVISION
OF THE STATE ARCHITECT OFFICE
OF REGULATION SERVICES

APPL. # 02-115682

AC TM F/LS 1/2 SS 5/25/18
DATE 5/25/18

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION
FOR CONSTRUCTION IS REQUIRED.

BEAM & PURLIN TABLE

8K

PROJECT:
GABLE SCHOOL
LUNCH SHELTER
VALLEY SCHOOL SHELTERS

REVISIONS			
NO.	DATE	BY	DESCRIPTION

DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16324

DRAWING TITLE
TABLE DETAILS

SHEET NUMBER

S3

OF 3 SHEETS

2374 WEST WHITENDALE
VISALIA, CA 93277

Grand
Gomer and Associates
STRUCTURAL ENGINEERS
Email: ghomerstructural@gmail.com

(559) 734-6675
FAX (559) 734-5232

Phone (559) 329-8830
Fax (559) 329-8807

CA Lic. 981366

LABOR AND / OR MATERIALS WARRANTY/GUARANTEE

Warranty for that certain project known as <School Name>, located at: <School Address>, installed for:

<School District>

We hereby warrant that the labor and / or materials which we have provided for the above project have been completed in accordance with the requirements of specifications sections for lunch shelters and walkway covers, and the Contract Documents.

We agree to repair or replace any or all of our work, together with any other adjacent work which may be displaced by so doing, that may prove to be defective in its workmanship or material, within a period of one year from the date of final acceptance by Owner or from the Date of Certificate of Occupancy, whichever is the earlier.

We also agree to repair any and all damages resulting from such defects, all without additional expense to the Owner. Ordinary wear and tear and unusual abuse or neglect accepted.

In the event of our failure to comply with the above mentioned conditions within 30 days after being notified in writing by the Owner, we collectively or separately do hereby authorized the Owner to proceed to have such defective work repaired or replaced and made good at our expense, and we will honor and pay the costs and charges therefrom upon demand.

Signed: _____ Date: <Date>
(Name/Title) Michael Messerschmidt (Owner)

Insurance Company:
Street Address:
City/State/Zip:

For maintenance, repair, or replacement service, contact:
Valley School Shelters
PO BOX 177
Tulare, CA 93275
(559) 329-8830

Contact person: Michael Messerschmidt



Valley School Shelters

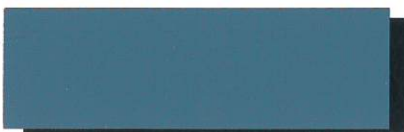
P.O. Box 1499
Tulare, CA 93275-1499
559-329-8830
www.valleyschoolshelters.com



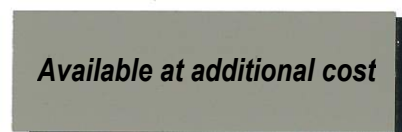
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McELROY METAL



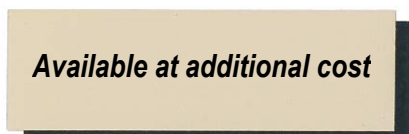
SP / REGAL WHITE



ROMAN BLUE



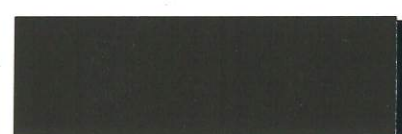
CLAY



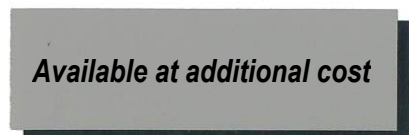
IVORY



SURREY BEIGE



PATRICIAN BRONZE



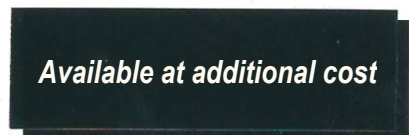
ASH GRAY



LIGHT STONE



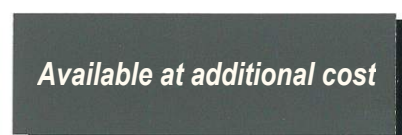
AUTUMN RED



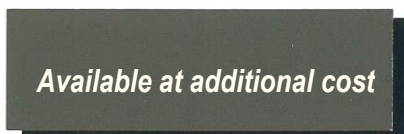
MATTE BLACK



TUDOR BROWN



CHARCOAL



TERRATONE
KYNAR 500® ONLY



EVERGREEN



BRANDYWINE



HARTFORD GREEN

- ADDITIONAL COST WILL APPLY FOR BRANDYWINE AND HARTFORD GREEN.
- AVAILABLE IN KYNAR 500® ONLY.

COLOR SELECTOR



ALL KYNAR 500® COLORS ARE ENERGY STAR COMPLIANT.

MM210CA



SUBMITTAL

For APPROVED EQUAL -
Ruskin ES - Berryessa USD



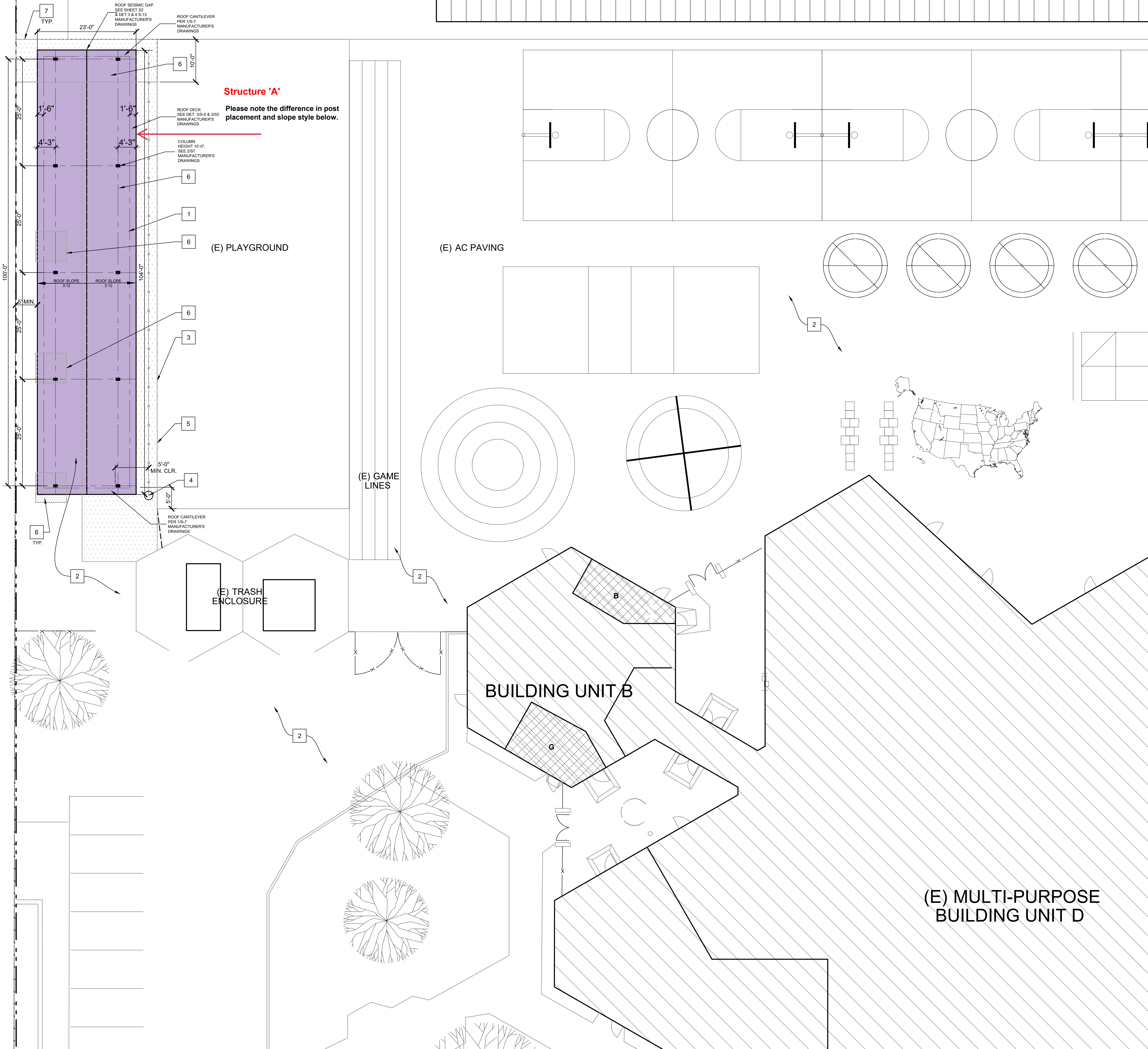
Valley School Shelters & Covered Walkways
Submitted by:



1555 Tahoe Court
Redding . California . 96003

Toll Free: 877-473-7619
Facsimile: 530-246-0518

RUSKIN ELEMENTARY SCHOOL



GENERAL NOTES

- A. CONTRACTOR TO VERIFY ALL BARRIERS IN P.O.T. HAVE BEEN REMOVED.
- B. CONTRACTOR TO REMOVE ALL EXISTING ITEMS TO ALLOW THE NEW WORK, INCLUDING BUT NOT LIMITED TO TREES, SHRUBS, ASPHALT PAVING, FENCING.
- B. GENERAL CONTRACTOR SHALL SURVEY THE AREA OF NEW CONSTRUCTION FOR UNDERGROUND UTILITIES PRIOR TO THE BEGINNING OF CONSTRUCTION, AND REROUTE/CA/P ALL EXISTING UTILITIES RUNNING BELOW THE AREA OF THE NEW SHADE STRUCTURES IF IT CONFLICTS WITH NEW SHADE STRUCTURE FOOTING.
- C. REFER TO SHADE STRUCTURES MANUFACTURER'S DRAWINGS FOR CONC. FOOTINGS DESIGN REQUIREMENTS.
- D. NEW SHADE STRUCTURES IS O.F.C.I. GENERAL CONTRACTOR IS RESPONSIBLE FOR THE FOUNDATION AND INSTALLATION/COORDINATION OF THE PRE-MANUFACTURED SHADE STRUCTURE.
- E. CONTRACTOR TO BACKFILL TRENCHES AND PATCH (E) PAVING AS REQUIRED TO MATCH EXISTING PAVING.
- K. PROVIDE TEMPORARY FENCING DURING CONSTRUCTION. SEE DETAIL 11A0.4
- H. TIE IN (N) SHADE STRUCTURE DOWNSPOUTS TO THE NEAREST CLEAN OUT.

NEW SITE PLAN NOTES

1. (N) SHADE STRUCTURE PC # 04-117117, SEE MANUFACTURER'S DRAWINGS.
2. (E) AC PAVING, PATCH AND REPAIR AS NEEDED AFTER INSTALLATION OF SHADE STRUCTURE. PROVIDE MAX 2% SLOPE IN ALL DIRECTIONS.
3. ASSUMED DIRECTION OF (E) STORM DRAIN LINE. CONTRACTOR TO VERIFY PRIOR TO BEGINNING OF CONSTRUCTION.
4. APPROXIMATE LOCATION OF (E) STORM INLET, VERIFY IN FIELD. ELEVATE AS NEEDED TO BE FLUSH WITH THE SURFACE OF (N) AC PAVING.
5. (E) PERIMETER BOARD OF PAVING AREA TO REMAIN.
6. NEW AC PAVING TO CONFORM WITH EXISTING, SEE DETAIL 2/A04. PROVIDE MAX 2% SLOPE IN ALL DIRECTIONS.
7. (N) HEADER/BOARD, TYP. SEE DET. 3/A-04

GRAPHIC KEY

- | | |
|----------|------------------------------|
| | EXISTING PROPERTY LINE |
| | ROOF OVERHANG |
| | CHAINLINK FENCE |
| | WOOD FENCE |
| | DECORATIVE FENCE |
| | NEW SHADE STRUCTURE |
| | EXISTING BUILDING |
| | EXISTING RESTROOMS |
| | (E) DRY STAND PIPE |
| | DRINKING FOUNTAIN |
| | (E) FIRE HYDRANT |
| | (E) SIGN |
| M | (E) MENS TOILET ROOM |
| W | (E) WOMENS TOILET ROOM |
| G | (E) GIRLS TOILET ROOM |
| B | (E) BOYS TOILET ROOM |
| U | (E) UNISEX TOILET ROOM |
| K | (E) KINDERGARTEN TOILET ROOM |

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 01-118968 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 06/12/2020



NEW ENLARGED SITE PLAN

SHADE STRUCTURE
RUSKIN ELEMENTARY SCHOOL
1401 TURLOCK LN., SAN JOSE, CA 95132
BERRYESSA UNION SCHOOL DISTRICT

REVISIONS		
NO.	ITEM	DATE

DRAWN BY:	MK
CHECKED BY:	NJ
SFA JOB NO:	DATE:
10066	08/17/2010

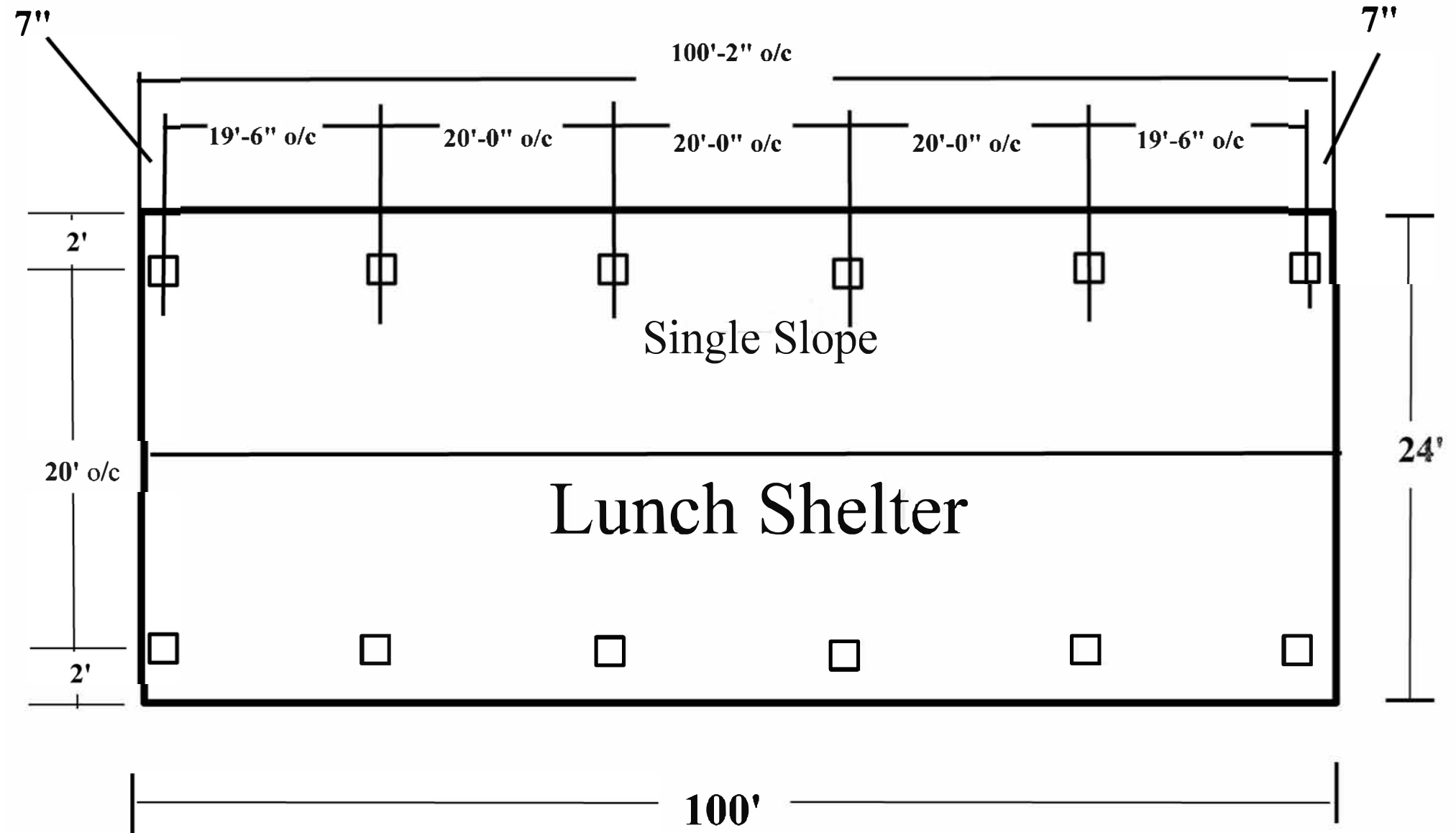
A0.2

Valley School Shelters

PO BOX 177 Tulare, CA 93275

Non Standard 24 x 100'-2" Single Slope Lunch Shelter w / 2:12 pitch

Ruskin ES Structure 'A'



NTS

Single Slope Lunch Shelter Cover DSA Pre-Checked Plans NO STAMP

Stamped plans to be provided if approved equal is approved.

Single Slope Lunch Shelter Covers Used On:

- **Ruskin Elementary Structure 'A'**

GENERAL NOTES

1. ALL DIMENSIONS, CONDITIONS AND ELEVATIONS ARE TO BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORK OR FABRICATION. IF ANY DISCREPANCIES ARE FOUND OR IF ANY CONDITION EXISTS NOT AS SHOWN ON THE DRAWINGS THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

2. THE CONTRACTOR WILL HOLD HARMLESS, INDEMNIFY AND DEFEND THE OWNER, THE ENGINEER, AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS, FROM ANY AND ALL LIABILITY CLAIMS, LOSSES OR DAMAGE ARISING OR ALLEGED TO ARISE FROM THE PERFORMANCE OF THE WORK DESCRIBED HEREIN, BUT NOT INCLUDING THE SOLE NEGLIGENCE OF THE OWNER, THE ENGINEER AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS.

3. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE DESIGN INTENT. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES.

IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.

ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION.

4. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON, OR NEAR THE CONSTRUCTION SITE.

5. ANY SUPPORT SERVICES PERFORMED BY THE ENGINEER OR HIS REPRESENTATIVES DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER OF MATERIAL OR WORK AND WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION ARE PERFORMED SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATION, BUT THEY DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

6. ANY CHANGES TO THE APPROVED SET OF PLANS WITHOUT NOTIFYING THE ENGINEER PRIOR TO SUCH CHANGES ASSUMES SAID ENGINEER FROM ANY AND ALL RESPONSIBILITY WITH RESPECT TO LIABILITY, DAMAGE OR EXTRA WORK RESULTING FROM SAID CHANGES.

7. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.

8. THE TYPICAL DETAILS SHOWN ON THESE SHEETS SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY SHOWN OTHERWISE. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS SHOWN FOR OTHER SIMILAR WORK.

9. DESIGN LOADS: RISK CATEGORY II OR RISK CATEGORY III
ROOF LIVE LOAD: 20 PSF ROOF DEAD LOAD: 3 PSF
SNOW LOAD: $P_g = F_s = 20 \text{ PSF} + 30 \text{ PSF}$ $C_e = 1.1$ $C_t = 1.2$ $C_s = 1.0$
 $I_s = 1.0$ RISK CATEGORY II $I_s = 1.0$ RISK CATEGORY III

SEISMIC: $I_e = 1.0$ AT RISK CATEGORY II OR $I_e = 1.25$ AT RISK CATEGORY III

SEISMIC DESIGN CATEGORY = D
 $S_{DS} = 1.75$ & $S_{DS} = 2.5$

STRUCTURE IS A STEEL ORDINARY CANTILEVERED COLUMN SYSTEM (G2 PER ASCE7-10)
 $R = 1.25/0.7$ (FOR WORKING STRESS)
 $C_d = 1.25$ $C_d = 1.25$
SOIL SITE CLASS = D
FOR 12.5' COLUMN HEIGHT $T = 0.49$
 $T > 0.5$ SEC. FOR COLUMN
HEIGHTS $> 12.5'$
BASE SHEAR: $V = C_u \times W$
RISK CATEGORY II $V = 0.56W$ (ASD) FOR $S_{DS} = 1.0$
RISK CATEGORY II $V = 0.96W$ (ASD) FOR $S_{DS} = 1.75$
RISK CATEGORY III $V = 0.71W$ (ASD) FOR $S_{DS} = 1.0$
RISK CATEGORY III $V = 1.75W$ (ASD) FOR $S_{DS} = 2.5$

WIND LOAD: 135 MPH, EXPOSURE C RISK CATEGORY II OR RISK CATEGORY III
 $K_d = 0.85$ $K_z = 0.90$ $K_{zt} = 1.0$
 $q_h = 21.42$ ($q_h = 0.00256 \times K_z \times K_{zt} \times K_d \times V^2 \times 0.6$ (FOR WORKING STRESS))
MAXIMUM BASIC WIND LOAD FOR PROJECT LOCATED IN SPECIAL WIND REGIONS SHALL BE EQUAL TO OR LESS THAN 135 mph AND CONFORM WITH THE ADOPTED ORDINANCE OF THE CITY, COUNTY OR CITY AND COUNTY IN WHICH THE PROJECT SITE IS LOCATED AND SHALL BE APPROVED BY DSA-SS.

10. GOVERNING CODE: 2016 CBC

11. ALLOWABLE SOIL BEARING IS BASED ON 1500 PSF & 100 PCF2X PASSIVE PRESSURE PER CBC TABLE 1806A.2 & SECTION 1806A.3.4. SKIN FRICTION IS PER 1810A.3.3.1.4 & IS EQUAL TO 1500/4=375 PSF. FOR UPLIFT SKIN FRICTION SHALL BE 125 PSF (S.F. OF 2).

12. ALL WORK TO BE PERFORMED UNDER THE CONTINUOUS INSPECTION OF A D.S.A. APPROVED INSPECTOR

13. ALL WORK SHALL CONFORM TO TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

14. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDA OR A CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR AND THE ENGINEER.

15. A PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE OFFICE OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

FOUNDATION NOTES

1. ALL FOOTINGS SHALL EXTEND TO FIRM BEARING IN UNDISTURBED SOIL OR ENGINEERED FILL.

2. NOMINAL TOP OF FLOOR SLAB ELEVATION = DATUM +0'-0" UNLESS OTHERWISE NOTED.

3. ANY EXISTING FILL AT THE BUILDING PAD SHALL MEET THE 92% COMPACTION REQUIREMENTS. ALL ORGANIC MATERIAL, RUBBLE, OR OTHER DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SITE.

4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR WALKWAYS.

5. ALL REINFORCING STEEL, ANCHOR BOLTS, AND OTHER EMBEDDED ITEMS SHALL BE SECURELY POSITIONED IN THE FORMS PRIOR TO POURING OF CONCRETE.

6. SHORING AND BRACING: IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, AND FORM WORK AS REQUIRED FOR THE CONSTRUCTION OF THIS BUILDING. PROVIDE TEMPORARY BRACING AS REQUIRED TO HOLD THE VARIOUS ELEMENTS IN PLACE UNTIL FINAL SUPPORT IS SECURELY ANCHORED.

7. EXCAVATION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL EXCAVATION PROCEDURES AND FOR PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.

8. BACKFILL: DO NOT BACKFILL AROUND THE EXTERIOR PERIMETER WALL UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. IF THE FLOOR SLABS ARE CONCRETE, DO NOT BACKFILL UNTIL 7 DAYS MINIMUM AFTER COMPLETION OF THE FLOOR SLABS. DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF DAMP-PROOFING.

CONCRETE NOTES

1. ALL MOLDS, ORNAMENTS, GROOVES, ETC. SHOWN ON THE ARCHITECTURAL DRAWINGS SHALL BE PROVIDED FOR IN THE FORM WORK BEFORE THE CONCRETE IS POURED.

2. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER EMBEDS SHALL BE IN PLACE AND SECURED TO FORM WORK PRIOR TO POURING OF CONCRETE.

3. REFER TO BOTH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF PLUMBING FIXTURES.

4. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE WALLS OR STRUCTURAL SLABS UNLESS SPECIFICALLY DETAILED.

5. CONSTRUCTION JOINTS NOT INDICATED ON THE DRAWINGS SHALL BE 50 MADE AND LOCATED AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. PROVISION SHALL BE MADE FOR TRANSFER OF SHEAR AND OTHER FORCES THROUGHOUT THE JOINTS. THE CONTRACTOR SHALL OBTAIN THE ARCHITECT'S APPROVAL OF CONSTRUCTION JOINT LOCATION IN ALL STRUCTURAL SLAB, BEAMS AND SHEAR WALLS.

6. SIDES OF FOOTINGS MAY BE POURED AGAINST STABLE EARTH.

7. THE QUALITY AND DESIGN OF CONCRETE SHALL COMPLY WITH TITLE 24 PART 2 EXCEPT ITEMS NOT SPECIFICALLY COVERED THEREIN. SHALL CONFORM TO ACI 318.

8. ALL REINFORCING SHALL BE NEW STOCK DEFORMED BARS CONFORMING TO ASTM A615.
A. #4 BARS AND SMALLER..... GRADE 40 OR 60
B. #5 BARS AND LARGER..... GRADE 60
C. SEPARATE BARS 1-1/2" DIAMETERS CLEAR OR 1-1/2" CLEAR, WHICHEVER IS LARGER.

9. MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:
CAST AGAINST EARTH (EXCEPT SLABS ON GRADE)..... 3"
EXPOSED TO EARTH OR WEATHER..... 1-1/2"
#5 BARS AND SMALLER..... 1-1/2"
#6 BARS AND LARGER..... 2"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
SLABS, WALLS, JOISTS..... 3/4"
#11 BARS AND SMALLER..... 3/4"
#14 AND #18 BARS..... 1-1/2"
BEAMS, GIRDERS, COLUMNS
PRINCIPAL REINFORCING TIES
STIRRUPS, OR SPIRALS..... 1-1/2"
SHELLS AND FOLDED PLATE MEMBERS..... 1-1/2"
#5 BARS AND SMALLER..... 1-1/2"
#6 BARS AND LARGER..... 3/4"

10. CONCRETE SHALL HAVE FOLLOWING MINIMUM REQUIREMENTS.
F1- 3000 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.5.
F2- 4500 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.45 & MINIMUM AIR CONTENT OF 7%.

11. THE ENGINEER DOES NOT PROVIDE CONTRACT ADMINISTRATION FOR THE PROJECT, INCLUDING REVIEW OF CONCRETE MIXES.

12. PER ACI SECTION 19.3.2.1. FOOTING SHALL NOT BE EXPOSED TO FREEZING AND THAWING CYCLES. SHALL NOT BE EXPOSED TO WATER-SOLUBLE SULFATE IN SOIL BY PERCENT OF MASS 20.10%, SHALL NOT BE EXPOSED TO EXTERNAL SOURCE OF CHLORIDES.

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL EXCEPT W SHAPES SHALL CONFORM TO ASTM A-36 AND SHALL BE FABRICATED AND ERRECTED AS PER AISC SPECIFICATIONS FOR BUILDINGS. W SHAPES SHALL CONFORM TO ASTM A992.

2. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE "B" AND STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500 GRADE "B", $F_y = 46 \text{ KSI}$.

3. ALL LIGHT GAGE STEEL TO CONFORM TO ASTM A583 GRADE 55 FOR ALL STRUCTURAL SHAPES, A583 GRADE 33 FOR ALL BLOCCING, FLASHINGS, MISCELLANEOUS CONNECTION PLATES, AND ANGLES.

4. ALL UNFINISHED BOLTS SHALL BE ASTM A-307 UNLESS NOTED OTHERWISE.

5. USE AISC USUAL GAGES FOR BOLT HOLES IN ALL STEEL SECTIONS UNLESS OTHERWISE NOTED.

6. THE STEEL FABRICATOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR ERECTION.

7. ALL BOLT HOLES ARE TO BE 1/16" OVERSIZED. ALL BOLTS SHALL HAVE WASHERS INSTALLED UNDER BOTH HEAD & NUT.

8. ALL STEEL SHALL BE PROTECTED FROM WEATHER AS FOLLOWS: STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED (MINIMUM ASTM A123 OR A153, CLASS D) OR PAINTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT; OR EQUIVALENT PAINT SYSTEM. COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM-ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION.

ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT-DIP GALVANIZED (ASTM A153, CLASS D MINIMUM), OR PROTECTED WITH CORROSION-PROTECTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT. (EXAMPLE: PROPRIETARY COATINGS THAT DO COMPLY WITH THE 1,000 HOUR REQUIREMENT INCLUDE BUT ARE NOT NECESSARILY LIMITED TO: QUIK GUARD BY SIMPSON, KWIK-COTE BY HILTI, STALGARD BY ELO, VISTACORR BY SFS INTEC, ETC.)

GOVERNING CODES:

1. 2016 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE (PART 1, TITLE 24, CCR).

2. 2016 CALIFORNIA BUILDING CODE, VOLUMES 1 & 2 (PART 2, TITLE 24, CCR).

3. 2016 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR).

4. 2016 CALIFORNIA FIRE CODE (PART 9, TITLE 24, CCR).

NOTES

1. COVERS ARE NOT DESIGNED TO BE ENCLOSED OR FOR STORAGE OF COMBUSTIBLE MATERIALS.

2. WALKWAY COVER HAS BEEN CHECKED FOR OBSTRUCTED WIND FLOW CONDITION & CAN BE WITHIN 6" MIN. FROM AN EXISTING BUILDING. SHALL BE REVIEWED ON A SITE SPECIFIC BASIS.

3. WALKWAY PIER FOOTING HAS BEEN CHECKED FOR D.S.A. BULLETIN 09-06 REV..

TESTING & INSPECTIONS REQUIREMENTS

1. INSPECTOR CLASS (MINIMUM REQUIREMENTS)
CLASS 2

2. SELECTION OF THE PROJECT INSPECTOR AND TESTING AGENCY
BY THE SCHOOL DISTRICT AND APPROVED BY D.S.A., A/E OF RECORD AND STRUCTURAL ENGINEER

3. COST OF THE PROJECT INSPECTOR (CA ADMIN. CODE 4-333(B) AND TESTING AGENCY (CA ADMIN. CODE 4-335)
BY THE SCHOOL DISTRICT

4. COPIES OF THE REPORT TO
ARCHITECT; STRUCTURAL ENGINEER; SCHOOL DISTRICT; D.S.A. (ORIGINAL); IOR; MANUFACTURER

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316 (D & E) OF THE CALIFORNIA CODE OF REGULATIONS, THE DISTRICT SHALL HIRE AN ARCHITECT OR STRUCTURAL ENGINEER TO BE IN GENERAL RESPONSIBLE CHARGE OF SITE SPECIFIC PROJECT.

2. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.

3. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR A PORTION OF THE PROJECT AS DESIGNED BY THE ARCHITECT FOR INCORPORATION INTO THE PROJECT.

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GERARD HOMER & ASSOCIATES RESPONSIBILITY FOR SITE SPECIFIC PROJECT.

SHADE STRUCTURE TESTING & INSPECTION GUIDELINE

THE EXAMPLE FORM DSA-103 SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECT-SPECIFIC FORM DSA-103. A FORM DSA-103 IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND THE EXAMPLE FORM DSA-103 IS TO BE CROSSED OUT ON THIS DRAWING.

DSA-103 STATEMENT OF STRUCTURAL TESTS & SPECIAL INSPECTIONS - 2016 CBC

IMPORTANT: THIS FORM IS ONLY A SUMMARY LIST OF STRUCTURAL TESTS AND SPECIAL INSPECTIONS REQUIRED FOR E PROJECT. THE ACTUAL TESTS AND INSPECTIONS MUST BE PERFORMED AS DETAILED ON THE DSA APPROVED DOCUMENTS. THE PROJECT INSPECTOR IS RESPONSIBLE FOR PROVIDING INSPECTION OF ALL PARTS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, SPECIAL INSPECTIONS NOT LISTED ON THIS FORM SUCH AS STRUCTURAL WOOD FRAMING, HIGH-LOAD WOOD DIAPHRAGMS, COLD-FORMED STEEL FRAMING, ANCHORAGE OF NON-STRUCTURAL COMPONENTS, ETC., PER TITLE 24, PART 2, CHAPTER 17A.

NOTE: REFERENCES ARE TO THE 2016 EDITION OF THE CALIFORNIA BUILDING CODE (CBC) UNLESS OTHERWISE NOTED.

TEST OR SPECIAL INSPECTION	TYPE	PERFORMED BY	CODE REFERENCE AND NOTES
SOILS			
1. GENERAL: TABLE 1705A.6			
A. VERIFY THAT: SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF CONTROLLED FILL AND/OR EXCAVATIONS FOR FOUNDATIONS. FOUNDATION EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL AND MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE
4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS): TABLE 1705A.7			
A. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH PIER.	CONTINUOUS	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE
C. CONFIRM PIER DIAMETERS, PLUMBNESS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, AND EMBEDMENT INTO BEDROCK (IF APPLICABLE). RECORD CONCRETE OR GROUT VOLUMES.	CONTINUOUS	GE	BY GEOTECHNICAL ENGINEER OR HIS OR HER QUALIFIED REPRESENTATIVE
E. CONCRETE PIERS: PROVIDE TEST AND INSPECTIONS PER CONCRETE SECTION BELOW			
CONCRETE: TABLE 1705A.3 ACI 318-14 SECTIONS 26.12 & 26.13			
7. CAST IN PLACE CONCRETE MATERIAL VERIFICATION AND TESTING:			
A. VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	SI & PI	TO BE PERFORMED BY BATCH-PLANT SPECIAL INSPECTOR AND PROJECT INSPECTOR.
B. IDENTIFY, SAMPLE & TEST REINFORCING STEEL.	TEST	LAB	1813A.2 (1913.2.6), ASTM A370, DSA R17-10
C. PERFORM SLUMP, TEMPERATURE, AND (WHERE REQUIRED) AIR CONTENT TEST.	TEST	LAB	ASTM C172, ASTM C31.
D. TEST CONCRETE (COMPRESSION). f_c	TEST	LAB	ACI 318 SECTION 5.6 AND 1905A.1.2 (1913.3.1+), ASTM C39.
E. BATCH PLANT INSPECTION: CONTINUOUS SI			
F. BATCH PLANT INSPECTION - DESIGN COMPLIES WITH 1705A.3.3 ITEM 2	PERIODIC	SI	1705A.3.3, ITEM 2. REQUIRES FIRST BATCH INSPECTION, WEIGHMASTER, AND BATCH TICKETS.
G. INSPECT PLACEMENT OF FORMWORK, REINFORCING STEEL, EMBEDDED ITEMS, AND CONCRETE. INSPECT CURING AND FORM REMOVAL.	CONTINUOUS	PI	MAY BE PERFORMED BY A SPECIAL INSPECTOR WHEN SPECIFICALLY APPROVED BY DSA.
STEEL TABLE 1705A.2.1, AISC 303-10, AISC 360-10, AISC 341-10, AISC 358-10, AISI S100-07/52-10			
17. STRUCTURAL STEEL AND COLD-FORMED STEEL USED FOR STRUCTURAL PURPOSES MATERIAL VERIFICATION:			
A. VERIFY THAT ALL MATERIALS ARE APPROPRIATELY MARKED AND THAT: MILL CERTIFICATES INDICATE MATERIAL PROPERTIES THAT COMPLY WITH REQUIREMENTS. MATERIAL SIZES, TYPES AND GRADES COMPLY WITH REQUIREMENTS.	PERIODIC	*	BY SPECIAL INSPECTOR WHEN PERFORMED OFF-SITE; BY PROJECT INSPECTOR FOR STEEL SHIPPED DIRECTLY TO PROJECT SITE WITHOUT WELDING OR FABRICATION.
B. TEST UNIDENTIFIED MATERIALS.	TEST	LAB	2203A.1 (2203.1+), ASTM A370
C. EXAMINE SEAM WELDS OF STRUCTURAL TUBES AND PIPES	PERIODIC	SI	DSA IR 17-3
INSPECTION:			
F. VERIFY STIFFENER LOCATIONS, CONNECTIONS TAB LOCATIONS AND ALL CONSTRUCTION DETAILS FABRICATED IN THE SHOP.	PERIODIC	SI	
KEY TO COLUMNS:			
1. TYPE:	2. PERFORMED BY:		
CONTINUOUS - INDICATES THAT A CONTINUOUS SPECIAL INSPECTION IS REQUIRED	GE - INDICATES THAT THE SPECIAL INSPECTION IS TO BE PERFORMED BY A REGISTERED GEOTECHNICAL ENGINEER OR HIS OR HER AUTHORIZED REPRESENTATIVE		
PERIODIC - INDICATES THAT A PERIODIC SPECIAL INSPECTION IS REQUIRED	LAB - INDICATES THAT THE TEST OR INSPECTION IS TO BE PERFORMED BY A TESTING LABORATORY ACCEPTED IN THE DSA LABORATORY EVALUATION AND ACCEPTANCE (LEA) PROGRAM. SEE SECTION 4-335, 2013 CCR TITLE 24, PART 1.		
TEST - INDICATES THAT A TEST IS REQUIRED	PI - INDICATES THAT THE SPECIAL INSPECTION IS TO BE PERFORMED BY THE PROJECT INSPECTOR		
SI - INDICATES THAT THE SPECIAL INSPECTION IS TO BE PERFORMED BY A SPECIAL INSPECTOR			
+ IN THE CODE REFERENCE AND NOTES COLUMN, IT INDICATES DSA-SS/CC SECTIONS THAT MAY BE USED BY COMMUNITY COLLEGES, PER 2016 CBC SEC. 1.9.2.2.			

GENERAL NOTES

SHEET INDEX & STRUCTURAL TESTS AND INSPECTIONS SHEET

4K

SINGLE SLOPE LUNCH SHELTER OPTIONS TABLE											
ROOF PANEL OPTION		3 P.S.F. SPRINKLER DEAD LOAD		NUMBER OF BAYS			NOTES				
AEP SPAN HR-26				ONE	THREE	FIVE	SEE 8K/9 FOR ROOF DECK PROFILES & SECTION PROPERTIES. ALL PANELS CAN BE USED WITH ALL LOADING OPTIONS.				
MELEROY MEGA-RIB				YES	NO	SIX					
LOADING		WIND SPEED (1)		SEISMIC			1. ROOF FRAMING MEMBERS DEPENDENT ON WIND SPEED, RISK CATEGORY AND FITCH. THE HIGHER WIND SPEED, THE LESS FITCH IS ALLOWED AT CATEGORY II ROOF FRAMING. ROOF FRAMING MEMBERS AT RISK CATEGORY III DESIGNED FOR BOTH MAX. WIND LOAD AND FITCH. 2. 30 PSF FRAMING REQUIRED W/ 3 P.S.F. OPTIONAL SPRINKLER LOAD & 20 P.S.F. LL OR SNOW LOAD. 3. 3 P.S.F. OPTIONAL SPRINKLER LOAD NOT ALLOWED WITH 30 P.S.F. SNOW LOAD. 4. FOR SNOW LOAD, IF THE HORIZONTAL SEPARATION DISTANCE BETWEEN ADJACENT STRUCTURES, s , IS LESS THAN 20 FT. (6.1 M) AND LESS THAN SIX TIMES THE VERTICAL SEPARATION DISTANCE ($6s$), THEN THE REQUIREMENTS FOR THE LEeward DRIFT OF SECTION T.7.1 SHALL BE USED TO DETERMINE THE DRIFT LOAD ON THE LOWER STRUCTURE. IF THE GARCH DRIFT ANALYSIS IS REQUIRED IT SHALL BE PROVIDED BY THE PC APPLICANT & THE SITE APPLICATION IS NOT ELIGIBLE FOR OVER THE COUNTER SUBMITTAL. <th data-kind="ghost"></th> <th data-kind="ghost"></th> <th data-kind="ghost"></th> <th data-kind="ghost"></th>				
20 PSF LL		MPH		R.C.	PITCH	LOWER SEISMIC $S_{DS} \leq 1.0$					
20 PSF SL		≤ 116		II	15 IN 12 OR LESS	HIGHER SEISMIC $S_{DS} \leq 1.75$					
30 PSF SL		≤ 123		II	1 IN 12 MAX.	LOWER SEISMIC $S_{DS} \leq 1.00$					
		≤ 129		II	0.5 IN 12 MAX.	HIGHER SEISMIC $S_{DS} \leq 2.5$					
		≤ 130		III	15 IN 12 OR LESS	R.C. II $S_{DS} \leq 1.75$					

SITE SPECIFIC INFORMATION TABLE							
SEISMIC COEFFICIENT, S_{ds}		SELECT OCCUPANT LOAD FACTOR (OLF)		DETERMINE SEISMIC DESIGN FOR $S_{ds} > 1.75$			
WIND SPEED (3 SEC. GUST) @ MPH		ASSEMBLY-CONCENTRATED (CHAIRS NON-FIXED) 7 S.F./OCCUPANT		FOR RISK CATEGORY II (USE $S_{ds} = 1.75$ REQUIREMENTS)			
WIND EXPOSURE CATEGORY		ASSEMBLY-NONCONCENTRATED (FIXED TABLES & CHAIRS) 15 S.F./OCCUPANT		25' BAYS $\times 1.75$ = <input type="text"/> MAXIMUM BAY SPACING IN FEET *			
WIDTH OF COVER AREA IN FT.		DETERMINE OCCUPANT LOAD = AREA/OLF		FOR RISK CATEGORY III WITH $I_e = 1.25$ & $S_{ds} > 2.5$			
LENGTH OF COVERED AREA IN FT.		DETERMINE RISK CATEGORY		25' BAYS $\times 1.75$ = <input type="text"/> MAXIMUM BAY SPACING IN FEET *			
AREA IN SQ. FT.		RISK CATEGORY II OCCUPANTS ≤ 300		* BAY SPACING ON PLAN VIEWS TO BE REVISED TO MATCH OR FOR RISK CATEGORY II STRUCTURES CAN USE RISK CATEGORY III COLUMNS & FOOTINGS.			
NUMBER OF BAYS		RISK CATEGORY III OCCUPANTS > 300					
CLEAR HEIGHT OF EAVES IN FT.		RISK CATEGORY III ADJACENT TO RISK CATEGORY II BUILDING					
ROOF SLOPE IN./FT.		NOTE: DUE TO THE PERIOD OF THE STRUCTURE, ONLY 12'-6" TALL OR LESS CAN HAVE S_{ds}					
CONCRETE EXPOSURE F0	CONCRETE EXPOSURE F2*						
GEOHAZARD REPORTS			LIQUEFIABLE SOILS OR SITE CLASS F				
REQUIRED			LIQUEFIABLE SOILS	YES	NO		
GEOHAZARD REPORTS ARE NOT REQUIRED FOR CANTILEVERED COLUMN OPEN STRUCTURES PROVIDED THEY ARE CONSTRUCTED OF METAL. DO NOT EXCEED 4,000 ST. IN PLAN AREA AND ARE NOT LOCATED WITHIN STATE OR LOCAL GEOHAZARD ZONES. THE STRUCTURES MAY BE SPLIT INTO MULTIPLE SEISMICALLY SEPARATED STRUCTURES TO STAY BELOW THE 4,000 ST. TRIGGER.			IF STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE-SPECIFIC GEOTECHNICAL REPORT IDENTIFIES SUCH HAZARD.				

* F2 IS REQUIRED FOR ANY SNOW LOAD 30 PSF OR GREATER.

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC-YC
IDENTIFICATION STAMP DIVISION OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES

APPL. # 02-115683

AC TML F/LS SS SE

DATE 5/25/18

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

SINGLE SLOPE SCHOOL LUNCH SHELTER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS

NO.	DATE	BY	DESCRIPTION

DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16323

DRAWING TITLE

GENERAL NOTES
SHEET INDEX & STRUCTURAL TESTS AND INSPECTIONS SHEET
OPTIONS TABLE & SITE SPECIFIC INFORMATION TABLE

SHEET NUMBER

S1

OF 3 SHEETS

SCALE 1/4" = 1'-0"

8K

OPTIONS TABLE & SITE SPECIFIC INFORMATION TABLE

BEAM & PURLIN TABLE			
(RISK CATEGORY)	LOADING	BEAM "A"	PURLIN "B"
(RISK CATEGORY II) SEE NOTE 3	20 PSF LL/SL	C17x3x12 GA6E	C12x2 1/4x14 GA6E
	30 PSF SL	C17x3x10 GA6E	C12x2 1/4x12 GA6E
(RISK CATEGORY III) SEE NOTE 3	20 PSF LL/SL	C18x3x12 GA6E	C12x2 1/4x12 GA6E
	30 PSF SL	C17x3x10 GA6E	C12x2 1/4x12 GA6E

- NOTES:
- RISK CATEGORY IS PER CBC 1604A.5.
 - SEE 0K/53 FOR SECTION PROPERTIES OF BEAMS.
 - SEE SITE SPECIFIC TABLE TO DETERMINE RISK CATEGORY.

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC-VC

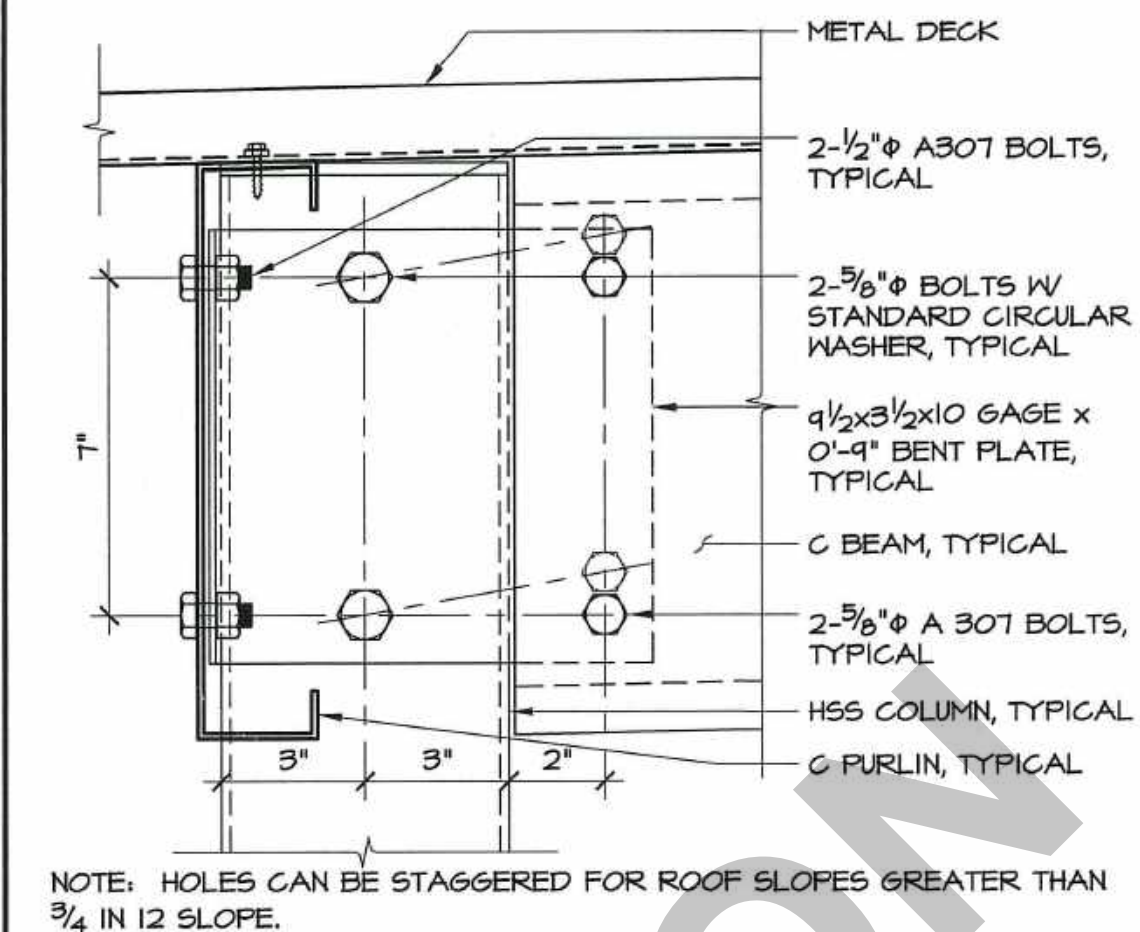
IDENTIFICATION STAMP, DIVISION OF THE STATE ARCHITECT OFFICE OF REGULATION SERVICES

APPL. # 02-115683

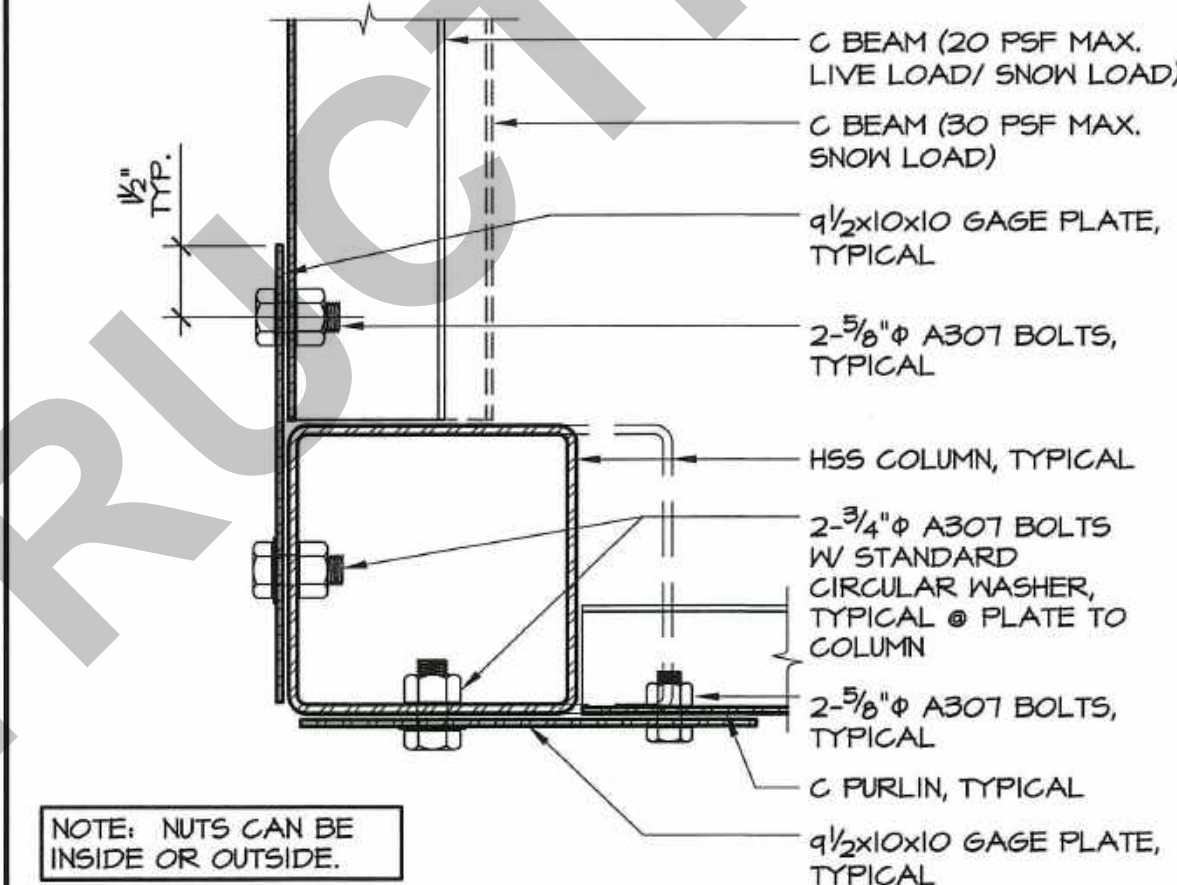
AC TM F/LS ML SS SS

DATE 5/23/18

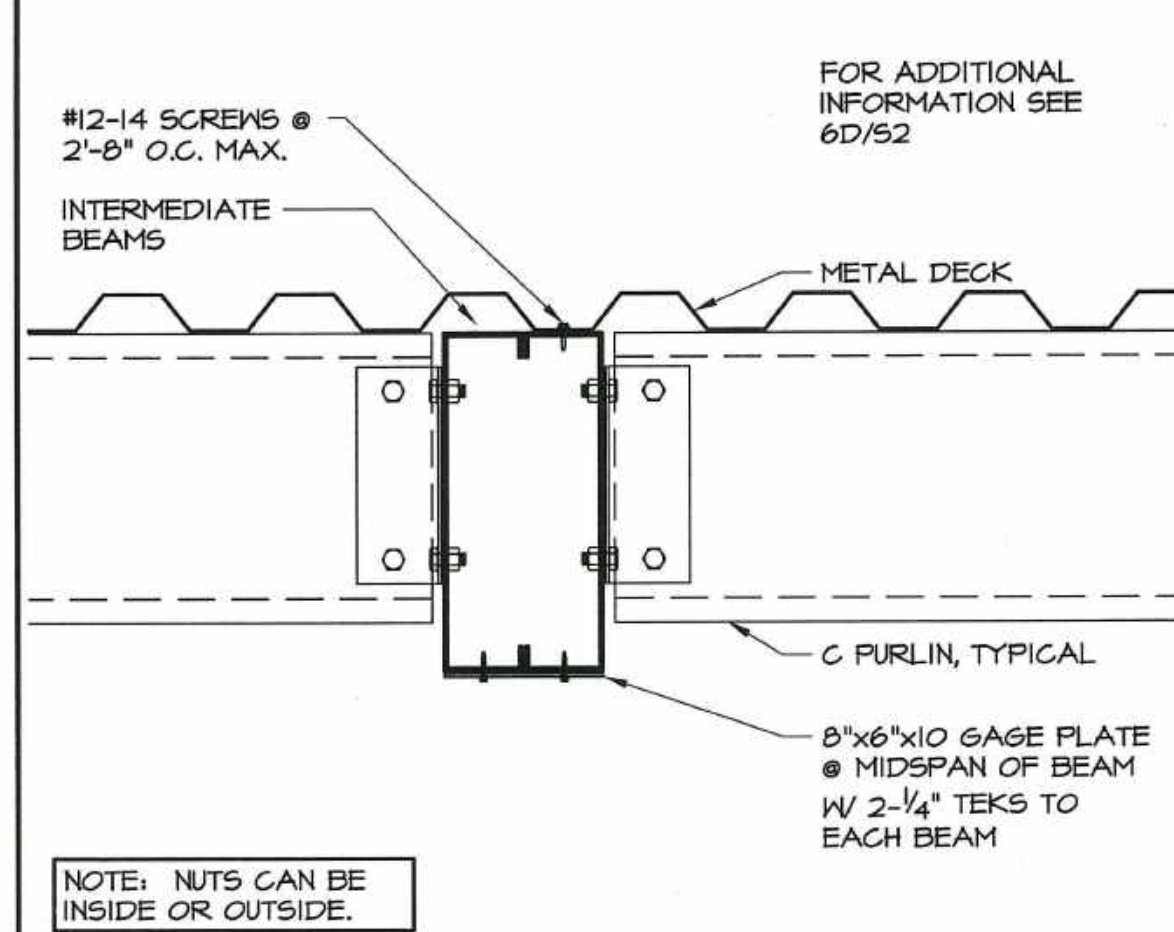
PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION
FOR CONSTRUCTION IS REQUIRED.



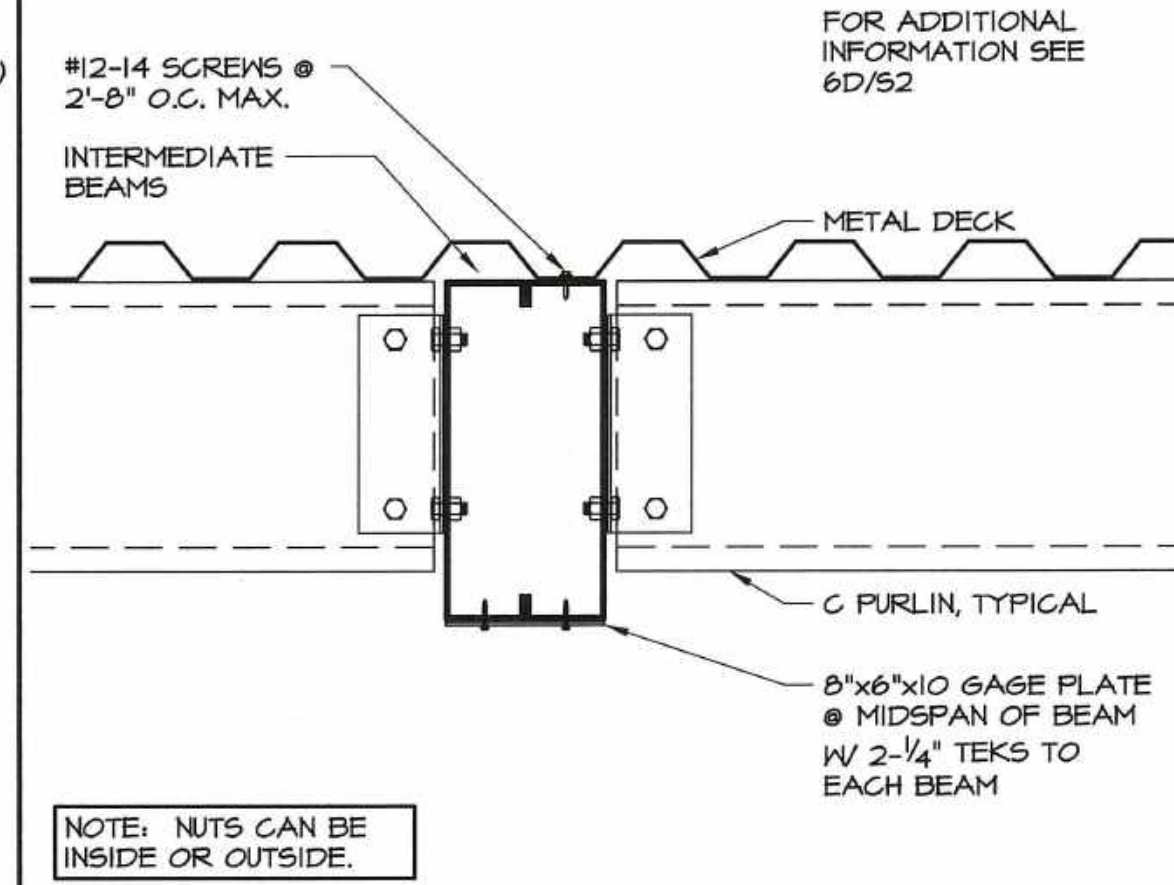
BEAM TO COLUMN SCALE 3" = 1'-0" 2H



BEAM TO COLUMN @ CORNER SCALE 3" = 1'-0" 4H



BEAM TIE @ MIDSPAN SCALE 1 1/2" = 1'-0" 2K



BEAM TO COLUMN SCALE 1 1/2" = 1'-0" 4K

BEAM & PURLIN TABLE

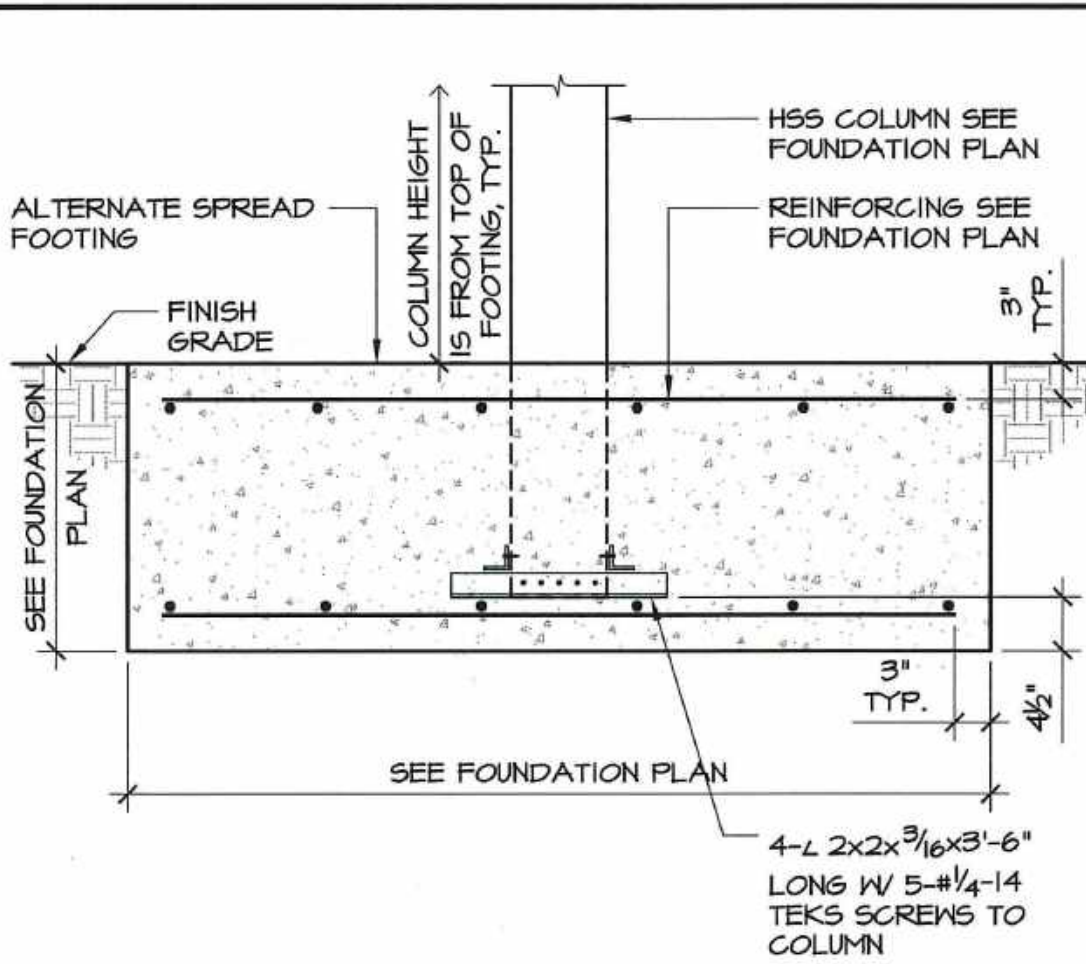
4D STAMPS

4F

COLUMN & FOOTING TABLE							
COLUMN HEIGHT	LOADING	LOWER SEISMIC (S _{DS} ≤ 1.0)			HIGHER SEISMIC (S _{DS} ≤ 1.75 MAX.)		
		COLUMN	PILE FOOTING	1 & 10 ALTERNATE SPREAD FOOTING	COLUMN	PILE FOOTING	1 & 10 ALTERNATE SPREAD FOOTING
12'-6"	20 PSF LL	H556x6x 3/16(4)	1'-6"φx8'-0"	6'-4" SQx2'-0"(8)	H556x6x 1/4(5)	1'-6"φx9'-4"	7'-0" SQx2'-4"(9)
	20 PSF SL	H556x6x 3/16(4)	1'-6"φx8'-4"	6'-4" SQx2'-0"(8)		1'-6"φx10'-8"	7'-0" SQx2'-4"(9)
	30 PSF SL	H556x6x 3/16(4)	1'-6"φx10'-8"	7'-0" SQx2'-4"(9)		1'-6"φx11'-4"	7'-4" SQx2'-4"(9)
	OPTIONAL SPRINKLER LOAD	H556x6x 3/16(4)	1'-6"φx9'-3"	7'-0" SQx2'-4"(9)		1'-6"φx11'-4"	7'-4" SQx2'-4"(9)
14'-6"	20 PSF LL	H556x6x 3/16(4)	1'-6"φx8'-3"	6'-4" SQx2'-0"(8)	H556x6x 1/4(5)	1'-6"φx10'-2"	7'-3" SQx2'-4"(9)
	20 PSF SL	H556x6x 3/16(4)	1'-6"φx8'-4"	6'-4" SQx2'-0"(8)		1'-6"φx10'-8"	7'-3" SQx2'-4"(9)
	30 PSF SL	H556x6x 3/16(4)	1'-6"φx10'-8"	7'-0" SQx2'-4"(9)		1'-6"φx11'-4"	8'-2" SQx2'-4"(9)
	OPTIONAL SPRINKLER LOAD	H556x6x 3/16(4)	1'-6"φx9'-1"	7'-0" SQx2'-4"(9)		1'-6"φx11'-4"	8'-2" SQx2'-4"(9)
16'-6"	20 PSF LL	H556x6x 3/16(4)	1'-6"φx8'-6"	6'-4" SQx2'-0"(8)	H556x6x 1/4(5)	1'-6"φx10'-8"	7'-6" SQx2'-4"(9)
	20 PSF SL	H556x6x 3/16(4)	1'-6"φx8'-4"	6'-4" SQx2'-0"(8)		1'-6"φx10'-8"	7'-6" SQx2'-4"(9)
	30 PSF SL	H556x6x 3/16(4)	1'-6"φx10'-8"	7'-0" SQx2'-4"(9)		1'-6"φx12'-0"	8'-6" SQx2'-4"(10)
	OPTIONAL SPRINKLER LOAD	H556x6x 3/16(4)	1'-6"φx9'-4"	7'-0" SQx2'-4"(9)		1'-6"φx12'-0"	8'-6" SQx2'-4"(10)
(RISK CATEGORY III)			LOWER SEISMIC (S _{DS} ≤ 1.0)		HIGHER SEISMIC (S _{DS} ≤ 2.5 MAX.)		
	20 PSF LL	H556x6x 3/16(5)	1'-6"φx8'-4"	6'-10" SQx2'-0"(8)	H556x6x 3/16(5)	1'-6"φx12'-3"	9'-0" SQx2'-4"(10)
	20 PSF SL	H556x6x 3/16(5)	1'-6"φx9'-4"	6'-10" SQx2'-0"(8)		1'-6"φx14'-3"	9'-10" SQx2'-6"(11)
	30 PSF SL	H556x6x 1/4(5)	1'-6"φx11'-4"	7'-6" SQx2'-4"(9)	H556x6x 1/2(7)	1'-6"φx14'-3"	9'-10" SQx2'-6"(11)
	OPTIONAL SPRINKLER LOAD	H556x6x 3/16(5)	1'-6"φx10'-0"	7'-6" SQx2'-4"(9)		1'-6"φx14'-3"	9'-10" SQx2'-6"(11)
	20 PSF LL	H556x6x 3/16(5)	1'-6"φx8'-4"	6'-10" SQx2'-0"(8)	H556x6x 3/8(6)	1'-6"φx12'-8"	9'-3" SQx2'-4"(10)
	20 PSF SL	H556x6x 3/16(5)	1'-6"φx9'-4"	6'-10" SQx2'-0"(8)		1'-6"φx14'-3"	10'-4" SQx2'-6"(11)
	30 PSF SL	H556x6x 1/4(5)	1'-6"φx11'-4"	7'-6" SQx2'-4"(9)	H556x6x 1/2(6)	1'-6"φx13'-0"	9'-8" SQx2'-4"(10)
	OPTIONAL SPRINKLER LOAD	H556x6x 3/16(5)	1'-6"φx10'-0"	7'-6" SQx2'-4"(9)		2'-0"φx13'-6"	10'-10" SQx2'-6"(11)
	20 PSF LL	H556x6x 3/16(5)	1'-6"φx8'-4"	6'-10" SQx2'-0"(8)	H556x6x 1/2(7)	2'-0"φx13'-6"	10'-10" SQx2'-6"(11)
	20 PSF SL	H556x6x 3/16(5)	1'-6"φx9'-4"	6'-10" SQx2'-0"(8)		2'-0"φx13'-6"	10'-10" SQx2'-6"(11)
	30 PSF SL	H556x6x 1/4(5)	1'-6"φx11'-4"	7'-6" SQx2'-4"(9)	H556x6x 1/2(7)	2'-0"φx13'-6"	10'-10" SQx2'-6"(11)
	OPTIONAL SPRINKLER LOAD	H556x6x 3/16(5)	1'-6"φx10'-0"	7'-6" SQx2'-4"(9)		2'-0"φx13'-6"	10'-10" SQx2'-6"(11)

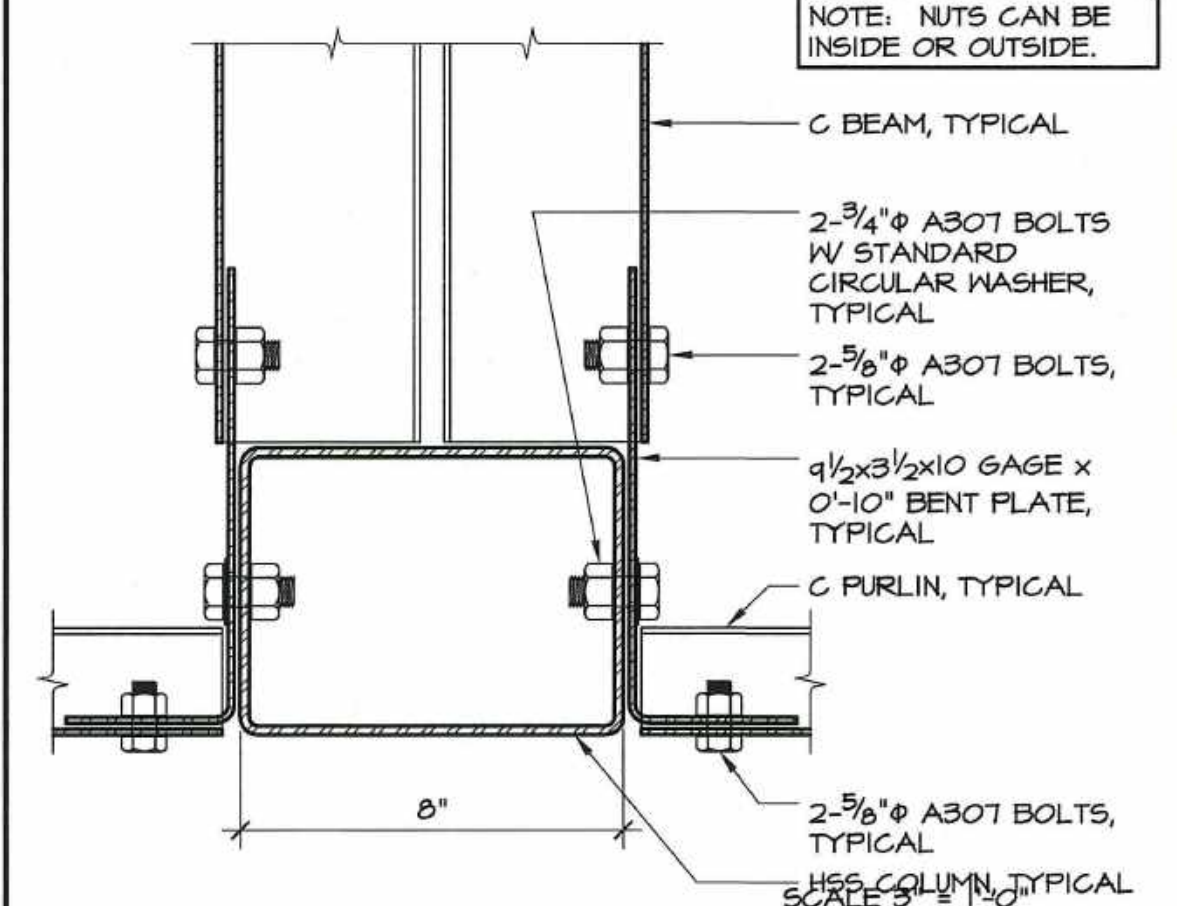
MOVE COLUMN & FOOTING IN 1" FROM COLUMN LINE SO OUTSIDE FACE OF COLUMN STILL ALIGN W/ CORNER COLUMN

- NOTES:
- ALTERNATE SPREAD FOOTINGS ARE OPTIONAL FOOTINGS @ AN ADDITIONAL COST.
 - NO SNOW LOAD ALLOWED W/ 3 P.S.F. MISCELLANEOUS DEAD LOAD.
 - COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 6" MAX. FROM BOTTOM, SEE SECTION 4K/52.
 - AT CORNER COLUMN MAY USE H556x6x 3/16.
 - AT CORNER COLUMN MAY USE H556x6x 3/16.
 - AT CORNER COLUMN MAY USE H556x6x 1/4.
 - AT CORNER COLUMN MAY USE H556x6x 3/16.
 - USE 6-#5 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 1/2" CLEAR FROM BOTTOM OF FOOTING.
 - USE 8-#5 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 1/2" CLEAR FROM BOTTOM OF FOOTING.
 - USE 10-#5 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 1/2" CLEAR FROM BOTTOM OF FOOTING.
 - USE 12-#5 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING W/ 3" MIN. COVER. KEEP COLUMN 1/2" CLEAR FROM BOTTOM OF FOOTING.
 - USE 4-#4 2x2x 3/16x3'-6" LONG ANGLES @ BOTTOM OF COLUMN, EACH W/ 5-#1/4-14 TEK SCREWS TO COLUMN, SEE 0F/53.
 - IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C., SEE GENERAL NOTES NUMBER II.

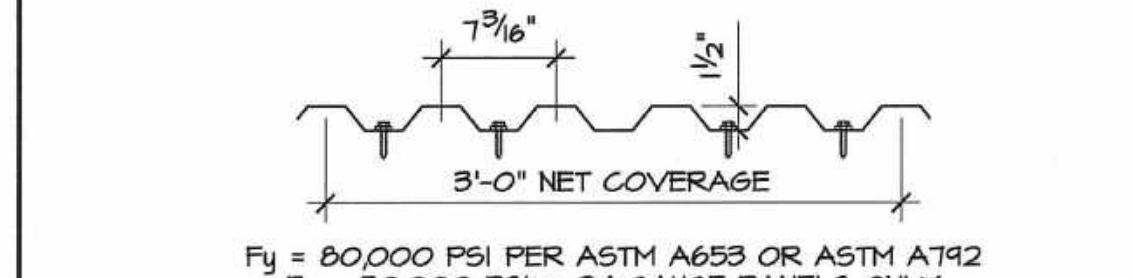


OPTIONAL SPREAD FOOTING @ ADDITIONAL COST SCALE 3/4" = 1'-0" 8D

BEAM TO COLUMN @ INTERMEDIATE LOCATION (20 PSF MAX. LIVE LOAD/SNOW LOAD) 6H

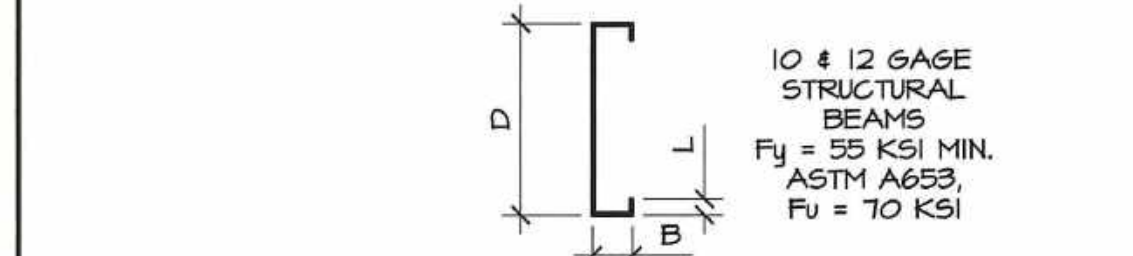


BEAM TO COLUMN @ INTERMEDIATE LOCATION (30 PSF MAX. SNOW LOAD) 8H



AEP SPAN HR-36 SECTION PROPERTIES						
GAUGE	Wt. (lbs./ft.)	I _x (in. ⁴ /ft.)	S _x (in. ³ /ft.)	I _y (in. ⁴ /ft.)	S _y (in. ³ /ft.)	
24	1.2	0.1116	0.1234	0.1141	0.1156	
26	0.98	0.0825	0.0895	0.0838	0.0839	

McELROY MEGA-RIB SECTION PROPERTIES						
GAUGE	Wt. (lbs./ft.)	I _x (in. ⁴ /ft.)	S _x (in. ³ /ft.)	I _y (in. ⁴ /ft.)	S _y (in. ³ /ft.)	
24	1.22	0.1038	0.1228	0.1038	0.1228	
26	0.98	0.0761	0.0841	0.0761	0.0841	



CEE SECTION PROPERTIES									
SECTION	GAUGE	WEIGHT	AXIS X-X			AXIS Y-Y			L
DxB		Wt.	A ₂ in.	I _x in. ⁴	S _x in. ³	I _y in. ⁴	S _y in. ³		
C12x2 1/4	14	4.28	0.629	24.153	3.524	0.807	0.469	1.081	
C12x2 1/4	12	6.43	1.134	35.934	5.989	1.2	0.705	1.166	
C17x3	12	8.57	1.185	83.417	8.837	2.32	0.461	0.916	
C17x3	10	10.98	1.762	113.65	12.634	2.98	1.242	0.989	
C18x3	12	9.02	1.256	98.693	9.909	2.5	1.035	1.042	

COLUMN & FOOTING TABLE

OPTIONAL SPREAD FOOTING @ ADDITIONAL COST

BEAM TO COLUMN @ INTERMEDIATE LOCATION

MATERIAL PROPERTIES TABLES

8K

Phone (559) 329-8830
Fax (559) 329-8807

CA Lic. 981366

LABOR AND / OR MATERIALS WARRANTY/GUARANTEE

Warranty for that certain project known as <School Name>, located at: <School Address>, installed for:

<School District>

We hereby warrant that the labor and / or materials which we have provided for the above project have been completed in accordance with the requirements of specifications sections for lunch shelters and walkway covers, and the Contract Documents.

We agree to repair or replace any or all of our work, together with any other adjacent work which may be displaced by so doing, that may prove to be defective in its workmanship or material, within a period of one year from the date of final acceptance by Owner or from the Date of Certificate of Occupancy, whichever is the earlier.

We also agree to repair any and all damages resulting from such defects, all without additional expense to the Owner. Ordinary wear and tear and unusual abuse or neglect accepted.

In the event of our failure to comply with the above mentioned conditions within 30 days after being notified in writing by the Owner, we collectively or separately do hereby authorized the Owner to proceed to have such defective work repaired or replaced and made good at our expense, and we will honor and pay the costs and charges therefrom upon demand.

Signed: _____ Date: <Date>
(Name/Title) Michael Messerschmidt (Owner)

Insurance Company:
Street Address:
City/State/Zip:

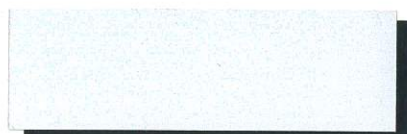
For maintenance, repair, or replacement service, contact:
Valley School Shelters
PO BOX 177
Tulare, CA 93275
(559) 329-8830

Contact person: Michael Messerschmidt

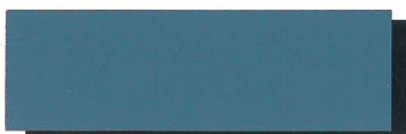


Valley School Shelters
P.O. Box 1499
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559-329-8830
www.valleyschoolshelters.com

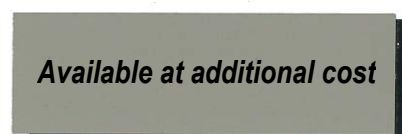
M
COLORS BY
McELROY METAL



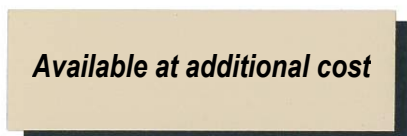
SP / REGAL WHITE



ROMAN BLUE



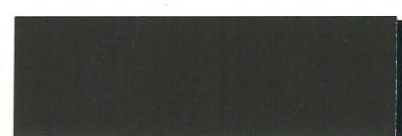
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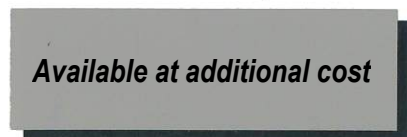
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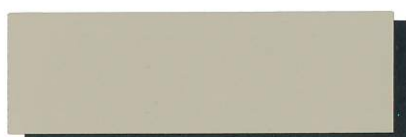
SURREY BEIGE



PATRICIAN BRONZE



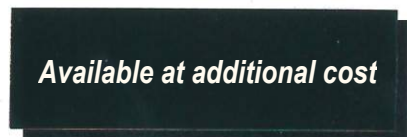
ASH GRAY



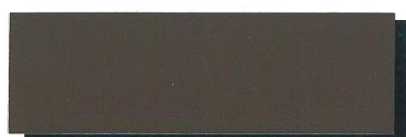
LIGHT STONE



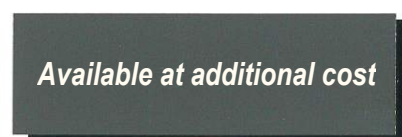
AUTUMN RED



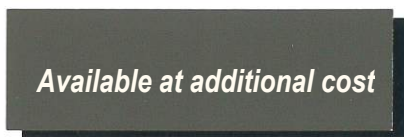
MATTE BLACK



TUDOR BROWN



CHARCOAL



TERRATONE
KYNAR 500® ONLY



EVERGREEN



BRANDYWINE



HARTFORD GREEN

• ADDITIONAL COST WILL APPLY FOR BRANDYWINE AND HARTFORD GREEN.
• AVAILABLE IN KYNAR 500® ONLY.

COLOR SELECTOR



ALL KYNAR 500® COLORS ARE ENERGY STAR COMPLIANT.

MM210CA



SUBMITTAL

For APPROVED EQUAL -
Vinci ES - Berryessa USD



Valley School Shelters & Covered Walkways
Submitted by:



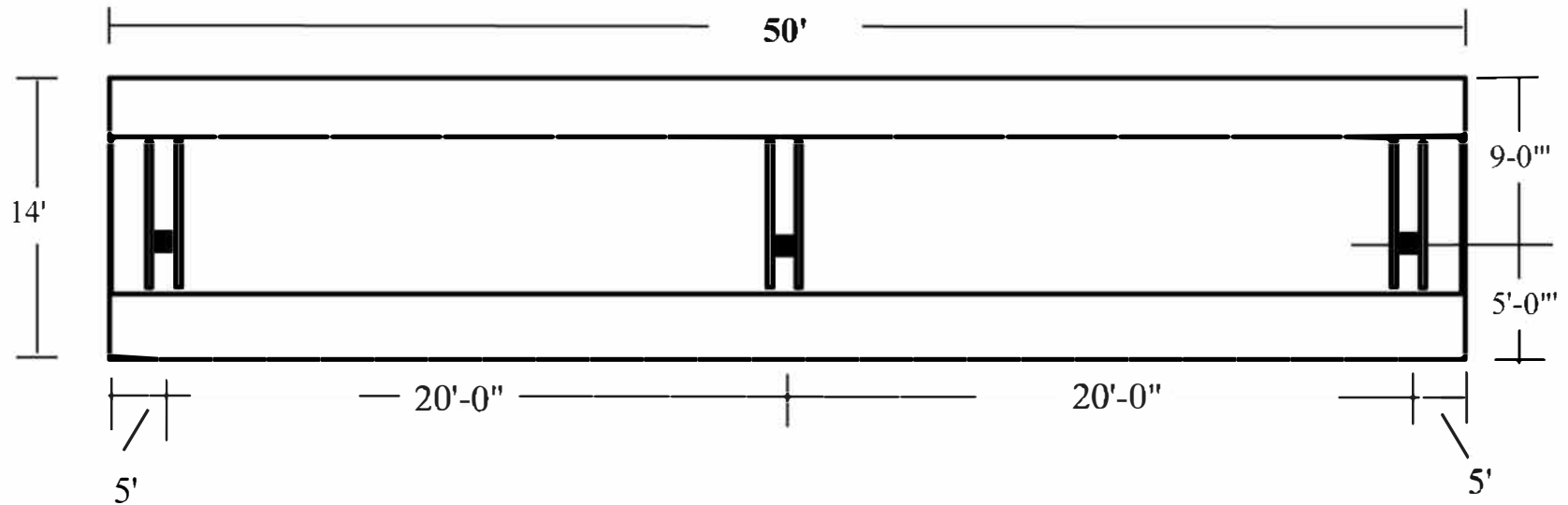
1555 Tahoe Court
Redding . California . 96003

Toll Free: 877-473-7619
Facsimile: 530-246-0518

Single Post Walkway Cover

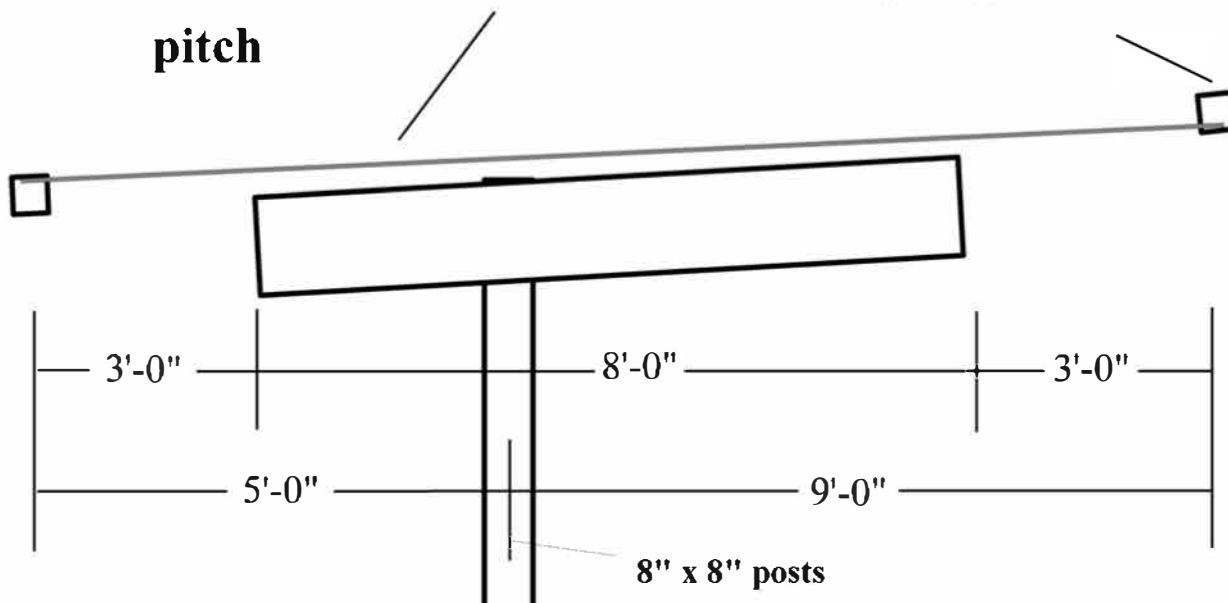
14' x 50' with 3 posts

Vinci ES Structure 'A'



**Up to 2:12
pitch**

Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included



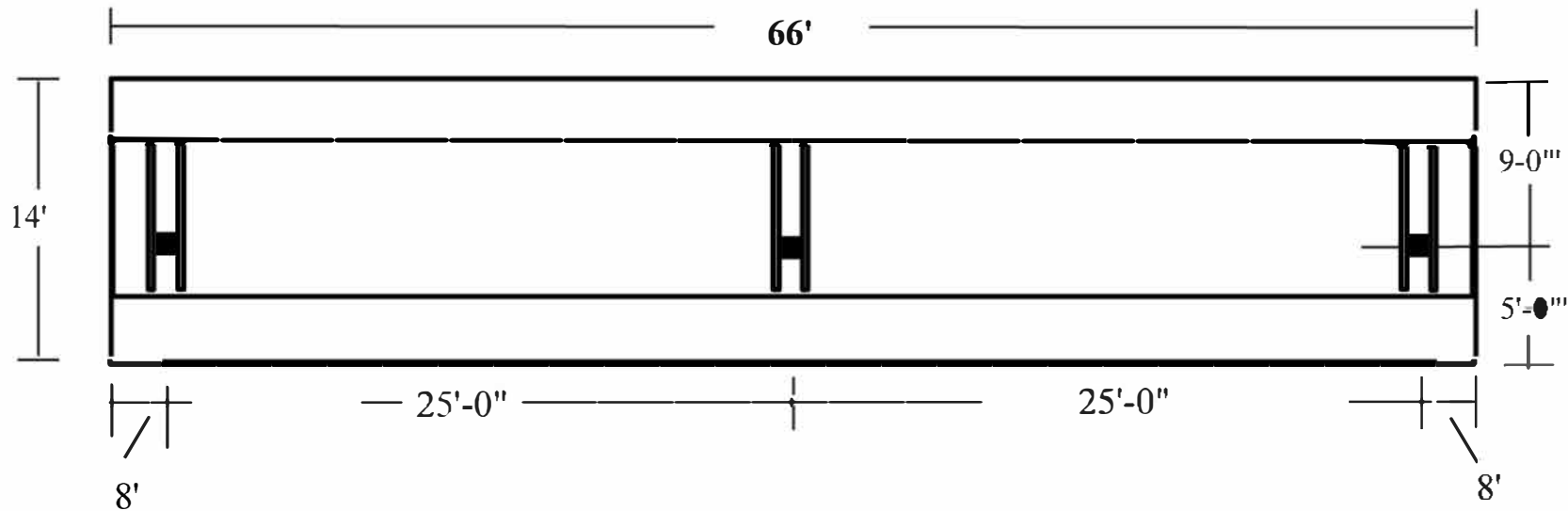
**Valley School Shelters
P O Box 177
Tulare, CA 93275**

nts

Single Post Walkway Cover

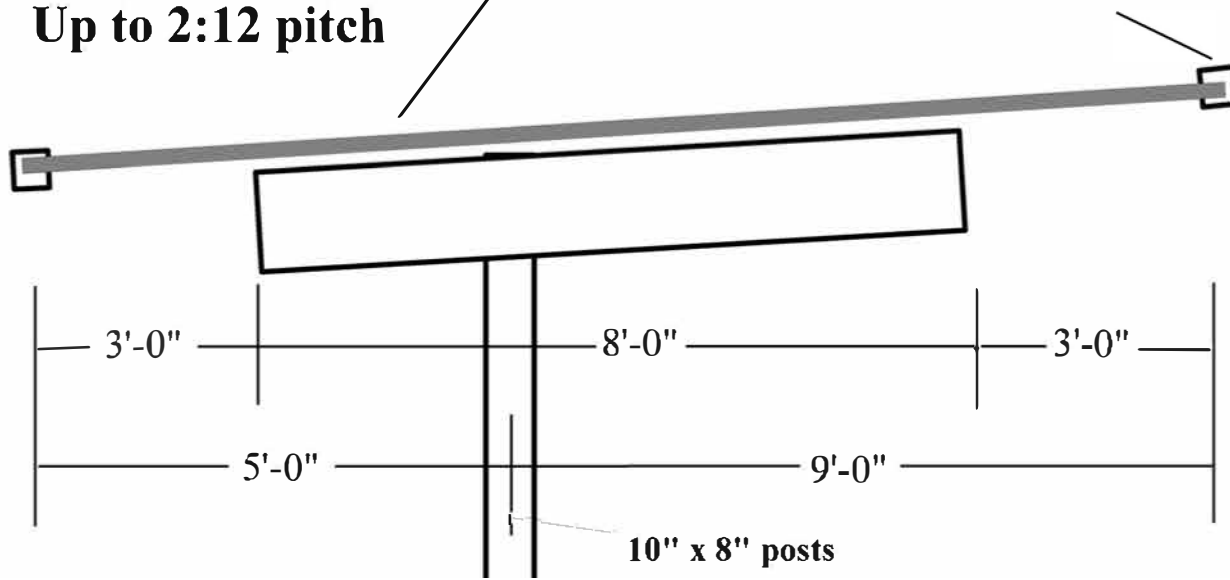
14' x 66' with 3 posts

Vinci ES Structure 'B'



Roofing is all 24 ga. Factory Painted
Over a dozen colors - Trim Included

1/4:12 STD
Up to 2:12 pitch



Valley School Shelters
P O Box 177
Tulare, CA 93275

nts

Single Post Walkway Cover DSA Pre-Checked Plans NO STAMP

Stamped plans to be provided if approved equal is approved.

Single Post Walkway Covers Used On:

- **Vinci Elementary Structure 'A'**
- **Vinci Elementary Structure 'B'**

1. ALL DIMENSIONS, CONDITIONS AND ELEVATIONS ARE TO BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORK OR FABRICATION. IF ANY DISCREPANCIES ARE FOUND OR IF ANY CONDITION EXISTS NOT AS SHOWN ON THE DRAWINGS THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

2. THE CONTRACTOR WILL HOLD HARMLESS, INDEMNIFY AND DEFEND THE OWNER, THE ENGINEER, AND HIS CONSULTANTS FROM THE CLAIMS OF THE OWNER'S EMPLOYEES AND AGENTS FROM ANY AND ALL LIABILITY CLAIMS, LOSSES OR DAMAGE ARISING OR ALLEGED TO ARISE FROM THE PERFORMANCE OF THE WORK DESCRIBED HEREIN, BUT NOT INCLUDING THE FOLE NEGLIGENCE OF THE ENGINEER, THE ENGINEER AND HIS CONSULTANTS, OR THE NEGLIGENCE OF THEIR EMPLOYEES AND AGENTS.

3. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE DESIGN INTENT. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES AND EQUIPMENT TO BE USED DURING WORK.

4. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY TO ALL WORK AND SHALL BE IN EFFECT FROM THE START TO THE END OF THE WORK.

5. ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION.

6. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON, OR NEAR THE CONSTRUCTION SITE.

7. ANY SUPPORT SERVICES PERFORMED BY THE ENGINEER OR HIS REPRESENTATIVES DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTIONS WHICH ARE PERFORMED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER OF MATERIAL OR WORK AND WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION SHALL BE FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATION, BUT THEY DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

8. ANY CHANGES TO THE APPROVED SET OF PLANS WITHOUT NOTIFYING THE ENGINEER PRIOR TO SUCH CHANGES ABSOLVES SAID ENGINEER FROM ANY AND ALL RESPONSIBILITY WITH RESPECT TO LIABILITY, DAMAGE OR EXTRA WORK RESULTING FROM SAID CHANGES.

9. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.

10. THE TYPICAL DETAILS SHOWN ON THESE SHEETS SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY OTHERWISE. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS SHOWN FOR OTHER SIMILAR WORK.

9. DESIGN LOADS: RISK CATEGORY II OR RISK CATEGORY III
ROOF LIVE LOAD: 20 PSF ROOF DEAD LOAD: $C_e = 1.3F$
SNOW LOAD: $W_s = 1.0$ AT RISK CATEGORY I & 30 PSF $C_e = 1.1, C_e = 1.2$ $C_e = 1.0$
 $I_a = 1.0$ AT RISK CATEGORY I $I_a = 1.1$ AT RISK CATEGORY III

SEISMIC: $I_a = 1.0$ AT RISK CATEGORY I $I_a = 1.25$ AT RISK CATEGORY III

SEISMIC DESIGN CATEGORY = D $I_a = 1.3$ FOR OFFSET CONFIGURATION
 $I_a = 1.0$ & $S_{DS} = 1.75$

STRUCTURE IS A STEEL, ORDINARY CANTILEVERED COLUMN SYSTEM (G2 PER ASCET-10)
 $R = 1.25$ (FOR WORKING STRESS)
 $C_e = 1.25$ $C_e = 1.25$
SITE CLASS = D
FOR 12' COLUMN HEIGHT $T = 0.361$ BASE SHEAR: $V = C_e W$
FOR 14' COLUMN HEIGHT $T = 0.45$ $V = 0.728W$ (ASD) FOR $S_{DS} =$
CONSTRUCTION CATEGORY II $C_e = 1.25$ $C_e = 1.25$ $C_e = 1.25$ $C_e = 1.25$ $C_e = 1.25$
FOR 16' COLUMN HEIGHT $T = 0.497$ HIGH SEISMIC $C_e = 1.25W$ (ASD) FOR $S_{DS} =$
HIGH SEISMIC $C_e = 1.25W$ (ASD) FOR $S_{DS} =$

WIND LOAD: 135 MPH, EXPOSURE C RISK CATEGORY II OR RISK CATEGORY III
 $g = 0.85$
 $g = 2142$ ($g = 0.02256 \times K_z \times K_t \times K_d \times V^2 \times 0.6$ (FOR WORKING STRESS))
MAXIMUM BASIC WIND LOAD FOR PROJECT LOCATED IN SPECIAL, WIND REGIONS SHALL BE EQUAL TO OR LESS THAN 135 mph. PROJECTS LOCATED IN SPECIAL, WIND REGIONS OR CITY AND COUNTY AREAS IN THE PROJECT SITE IS LOCATED AND SHALL BE APPROVED BY DSA-SS.

10. GOVERNING CODE: 2016 CBC

11. ALLOWABLE SOIL BEARING IS BASED ON 1500 PSF AND 100 PCF2 PASSIVE PRESSURE PER FOOT. SECTION 1800.4.2 & SECTION 1800.4.3. SOIL FRICITION IS PER 1810.3.3.1 & IS EQUAL TO 1500/6 = 250 PSF. FOR UPLIFT SKIN FRICION SHALL BE 125 PSF (F2, OF F2).

12. ALL WORK SHALL BE PERFORMED UNDER THE CONTINUOUS INSPECTION OF A D.S.A. APPROVED INSPECTOR.

13. ALL WORK SHALL CONFORM TO TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

14. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDA OR A CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY CHAPTER 4-338, PART 1, TITLE 24, CCR AND THE ENGINEER.

15. A PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE OFFICE OF THE DISTRICT ARCHITECT SHALL BE REQUIRED TO SUPERVISE THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.

1. ALL FOOTINGS SHALL EXTEND TO FIRM BEARING IN UNDISTURBED SOIL OR ENGINEERED FILL.
2. NOMINAL TOP OF FLOOR SLAB ELEVATION = DATUM +4'-0" UNLESS OTHERWISE NOTED.
3. ANY EXISTING FILL AT THE BUILDING PAD SHALL MEET THE 92% COMPACTION REQUIREMENTS. ALL ORGANIC MATERIAL, RUBBLE, OR OTHER DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SITE.
4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF EXTERIOR WALKWAYS.
5. ALL REINFORCING STEEL, ANCHOR BOLTS, AND OTHER EMBEDDED ITEMS SHALL BE SECURELY POSITIONED IN THE FORMS PRIOR TO POURING OF CONCRETE.
6. FORMING AND BRACING: IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DESIGN AND PROVIDE THE REQUIRED SHORING AND BRACING TO BE USED FOR THE CONSTRUCTION OF THIS BUILDING. PROVIDE TEMPORARY BRACING AS REQUIRED TO HOLD THE VARIOUS ELEMENTS IN PLACE UNTIL FINAL SUPPORT IS SECURELY ANCHORED.
7. EXCAVATION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL EXCAVATION PROCEDURES AND FOR THE PROTECTION ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
8. BACKFILL: DO NOT BACKFILL AROUND THE EXTERIOR PERIMETER WALL UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. IF THE FLOOR SLABS ARE CONCRETE, DO NOT BACKFILL UNTIL THE INTERIOR FLOORING IS COMPLETED. IF THE FLOOR SLABS DO NOT BACKFILL UNTIL AFTER COMPLETION AND INSPECTION OF DAMP-PROOFING.

CONCRETE NOTES

1. ALL MOIDS, ORNAMENTS, GROOVES, ETC. SHOWN ON THE ARCHITECTURAL DRAWINGS SHALL BE PROVIDED FOR IN THE FORM WORK BEFORE THE CONCRETE IS POURED.
2. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER EMBEDS SHALL BE IN PLACE AND SET TO FORM WORK PRIOR TO POURING OF CONCRETE.
3. REFER TO BOTH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF PLUMBING FIXTURES.
4. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE WALLS OR STRUCTURAL SLABS UNLESS SPECIFICALLY DETAILED.
5. CONSTRUCTION JOINTS NOT INDICATED ON THE DRAWINGS SHALL BE SO MADE AND LOCATED AS NOT TO WEAKEN THE STRENGTH OF THE STRUCTURE. PROVISION SHALL BE MADE FOR TRANSFER OF SHEAR AND OTHER FORCES THROUGHOUT THE JOINTS. THE CONTRACTOR SHALL OBTAIN THE ARCHITECT'S APPROVAL OF CONSTRUCTION JOINT LOCATION IN ALL STRUCTURAL SLAB, BEAMS AND STAIRS.

1. ALL STRUCTURAL STEEL EXCEPT W SHAPES SHALL CONFORM TO ASTM A-36 AND SHALL BE FABRICATED AND ERECTED AS PER AISI SPECIFICATIONS FOR BUILDINGS. W SHAPES SHALL CONFORM TO ASTM A-500.
2. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE "B" AND STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500 GRADE "B", Fy=46KSI.
3. ALL LIGHT GAGE STEEL TO CONFORM TO ASTM A563 GRADE 55 FOR ALL STRUCTURAL SHAPES, A563 GRADE 33 FOR ALL BLOCKING, FLASHINGS, MISCELLANEOUS CONNECTION PLATES, AND ANGLES.
4. ALL UNFINISHED BOLTS SHALL BE ASTM A-307 UNLESS NOTED OTHERWISE.
5. USE AISI USUAL GAGES FOR BOLT HOLES IN ALL STEEL SECTIONS UNLESS OTHERWISE NOTED.
6. THE STEEL FABRICATOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR ERECTION.
7. ALL BOLT HOLES ARE TO BE 1/16" OVERSIZED. ALL BOLTS SHALL HAVE WASHERS INSTALLED UNDER BOTH HEAD AND NUT.
8. ALL STEEL SHALL BE PROTECTED FROM WEATHER AS FOLLOWS: STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED (MINIMUM A153 CLASS D), OR PAINTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT; OR EQUIVALENT PAINT SYSTEM, "COLD-FORMED" STEEL SHALL BE GALVANNEAL (MINIMUM A153 CLASS D), OR PAINTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT. ALL STEEL SHALL BE PROTECTED TO A153 Z27/A153Z27/A153Z27 IN ACCORDANCE TO AISI Z20 TABLE A-1, CP 90 COATING DESIGNATION.
9. ALL EXPOSED STEEL FABRICATIONS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT-DIP GALVANIZED (A153 A153, CLASS D MINIMUM), OR PROTECTED WITH CORROSION-PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RED RUST IN 100 HOURS IN A SALT-SPRAY TEST. ALL STEEL FABRICATIONS SHALL BE PROTECTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT. ALL STEEL FABRICATIONS SHALL BE PROTECTED TO A153 Z27/A153Z27/A153Z27 IN ACCORDANCE TO AISI Z20 TABLE A-1, CP 90 COATING DESIGNATION. (EXAMPLE PROTECTIVE COATINGS THAT DO NOT COMPLY WITH THIS REQUIREMENT, (EXAMPLE PROTECTIVE COATINGS THAT DO COMPLY WITH THIS REQUIREMENT, Kwik-COTE BY HILTI, STALGARD BY EXLO, VISTACOR BY SFS INTL, ETC.)

GOVERNING CODES:

1. 2016 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE (PART 1, TITLE 24, CCR).
2. 2016 CALIFORNIA BUILDING CODE, VOLUMES 1 & 2 (PART 2, TITLE 24, CCR).
3. 2016 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR).
4. 2016 CALIFORNIA FIRE CODE (PART 9, TITLE 24, CCR).

NOTES

1. COVERS ARE NOT DESIGNED TO BE ENCLOSED OR FOR STORAGE OF COMBUSTIBLE MATERIALS.
2. WALKWAY COVER HAS BEEN CHECKED FOR OBSTRUCTED WIND FLOW CONDITION & CAN BE WITHIN 6" MIN. FROM AN EXISTING BUILDING. SHALL BE REVIEWED ON A SITE SPECIFIC BASIS.
3. WALKWAY PIER FOOTING HAS BEEN CHECKED FOR D.S.A. BULLETIN 09-06 REV..

TESTING & INSPECTIONS REQUIREMENTS

1. INSPECTOR CLASS (MINIMUM REQUIREMENTS) CLASS 2	
2. SELECTION OF THE PROJECT INSPECTOR AND TESTING AGENCY BY THE SCHOOL DISTRICT AND APPROVED BY D.S.A., A/E OF RECORD AND STRUCTURAL ENGINEER	
3. COST OF THE PROJECT INSPECTOR (CA ADMIN. CODE 4-333(B)) AND TESTING AGENCY (CA ADMIN. CODE 4-335) BY THE SCHOOL DISTRICT	
4. COPIES OF THE REPORT TO ARCHITECT; STRUCTURAL ENGINEER; SCHOOL DISTRICT; D.S.A. (ORIGINAL); IOR; MANUFACTURER	

NOTICE OF DISPARITY FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-318 (D & E) OF THE CALIFORNIA CODE OF REGULATIONS, THE DISTRICT SHALL HIRE AN ARCHITECT OR STRUCTURAL ENGINEER TO BE IN GENERAL RESPONSIBLE CHARGE OF SITE SPECIFIC PROJECT.

2. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBILITY CHARGE.

3. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR A PORTION OF THE PROJECT AS DESIGNED BY THE ARCHITECT FOR CONSTRUCTION INTO THE PROJECT.

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GERARD HOMER & ASSOCIATES RESPONSIBILITY FOR SITE SPECIFIC PROJECT.

SHADE STRUCTURE TESTING & INSPECTION GUIDELINE

DSA-103 STATEMENT OF STRUCTURAL TESTS & SPECIAL INSPECTIONS – 2016 CBC

SHEET INDEX

BUILDING DATA

GENERAL NOTES

45%
EMBEDMENT

COLUMN HEIGHT IS FROM TOP OF FOOTING, TYPICAL

COLUMN, FILL COLUMN W/ CONCRETE TO TOP OF FOOTING, MUST FILL FROM TOP OF COLUMN (1.25 CU.FT. MIN.)

CONCRETE OR AC PAVING

2" CLR

6" CLR

3" CLR

4-L 2x2x $\frac{3}{16}$ x3'-6" LONG ANGLES W/ 5- $\frac{1}{4}$ -14 TEK SCREWS TO COLUMN

REINFORCING, SEE COLUMN & FOOTING TABLE 4H/52

SCALE $\frac{1}{2}" = 1'-0"$

SHEET INDEX & STRUCTURAL TESTS AND INSPECTIONS SHEET

SITE SPECIFIC INFORMATION TABLE

* F2 IS REQUIRED FOR ANY SNOW LOAD 30 PSF OR GREATER

SINGLE POLE WALKWAY OPTIONS TABLE

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

AC TM F/LS gk SS St
DATE 5/29/18

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION
FOR CONSTRUCTION IS REQUIRED.

OPTIONAL SPREAD FOOTING @ ADDITIONAL COST

OPTIONS TABLE & SITE SPECIFIC INFORMATION TABLE

REVISIONS

DWN BY: T.E.H.	CHKD BY: G.B.H.
DATE: 5/25/18	

570 1000000 14200

DRAWING TITLE

DRAWING TITLE
FOUNDATION PLAN
GENERAL NOTES
DETAILS

SHEET NUMBER

COLUMN & FOOTING TABLE (NOTE 6)														
COLUMN HEIGHT	ROOF PITCH	LOAD	LOWER SEISMIC (S _{DS} = 1.0 RISK CATEGORY II) (S _{DS} ≤ 0.8 RISK CATEGORY III)						HIGHER SEISMIC (S _{DS} = 2.5 RISK CATEGORY II) (S _{DS} ≤ 2.0 RISK CATEGORY III)					
			≤ 12'-0" PROJECTION			≤ 14'-0" PROJECTION			≤ 12'-0" PROJECTION			≤ 14'-0" PROJECTION		
			COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1	COLUMN SIZE	PILE FOOTING	ALTERNATE SPREAD FOOTING (8B/S1) 1
12' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x1/4	2'-0"φx10'-3" DEEP	9'-3"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx11'-4" DEEP	10'-4"SQ.x2'-6" THICK (4)	H550x8x3/8	2'-0"φx11'-10" DEEP	10'-6"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx13'-2" DEEP	11'-4"SQ.x2'-6" THICK (5)
	4:12 MAX.	20psf LL/SL 30psf SL	H5510x8x3/8	2'-0"φx12'-2" DEEP	9'-8"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx13'-0" DEEP	10'-10"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx12'-2" DEEP		H5510x8x3/8		
14' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x1/4	2'-0"φx10'-8" DEEP	9'-8"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx12'-2" DEEP	10'-10"SQ.x2'-6" THICK (4)	H550x8x3/8	2'-0"φx12'-3" DEEP				
	4:12 MAX.	20psf LL/SL 30psf SL	H5510x8x3/8	2'-0"φx12'-6" DEEP	9'-10"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx13'-2" DEEP	11'-0"SQ.x2'-6" THICK (5)	H5510x8x3/8	2'-0"φx12'-6" DEEP	11'-0"SQ.x2'-6" THICK (5)	H5510x8x3/8	2'-0"φx13'-6" DEEP	11'-10"SQ.x2'-6" THICK (5)
16' COLUMN HEIGHT	2:12 MAX.	20psf LL/SL 30psf SL	H550x8x3/8	2'-0"φx11'-0" DEEP	10'-0"SQ.x2'-6" THICK (4)	H5510x8x3/8	2'-0"φx12'-6" DEEP	11'-3"SQ.x2'-6" THICK (5)		2'-0"φx12'-8" DEEP				
	4:12 MAX.	20psf LL/SL 30psf SL	H5510x8x3/8	2'-0"φx12'-8" DEEP	10'-2"SQ.x2'-6" THICK (4)	H5510x8x1/2	2'-0"φx13'-6" DEEP		H5510x8x3/8	2'-0"φx12'-8" DEEP	11'-4"SQ.x2'-6" THICK (5)	H5510x8x1/2	2'-0"φx14'-0" DEEP	12'-4"SQ.x2'-6" THICK (5)

NOTES:

1. ALTERNATE SPREAD FOOTINGS ARE OPTIONAL FOOTINGS @ AN ADDITIONAL COST.

2. NO SNOW LOAD ALLOWED W/ 3 P.S.F. FIRE SPRINKLER DEAD LOAD.

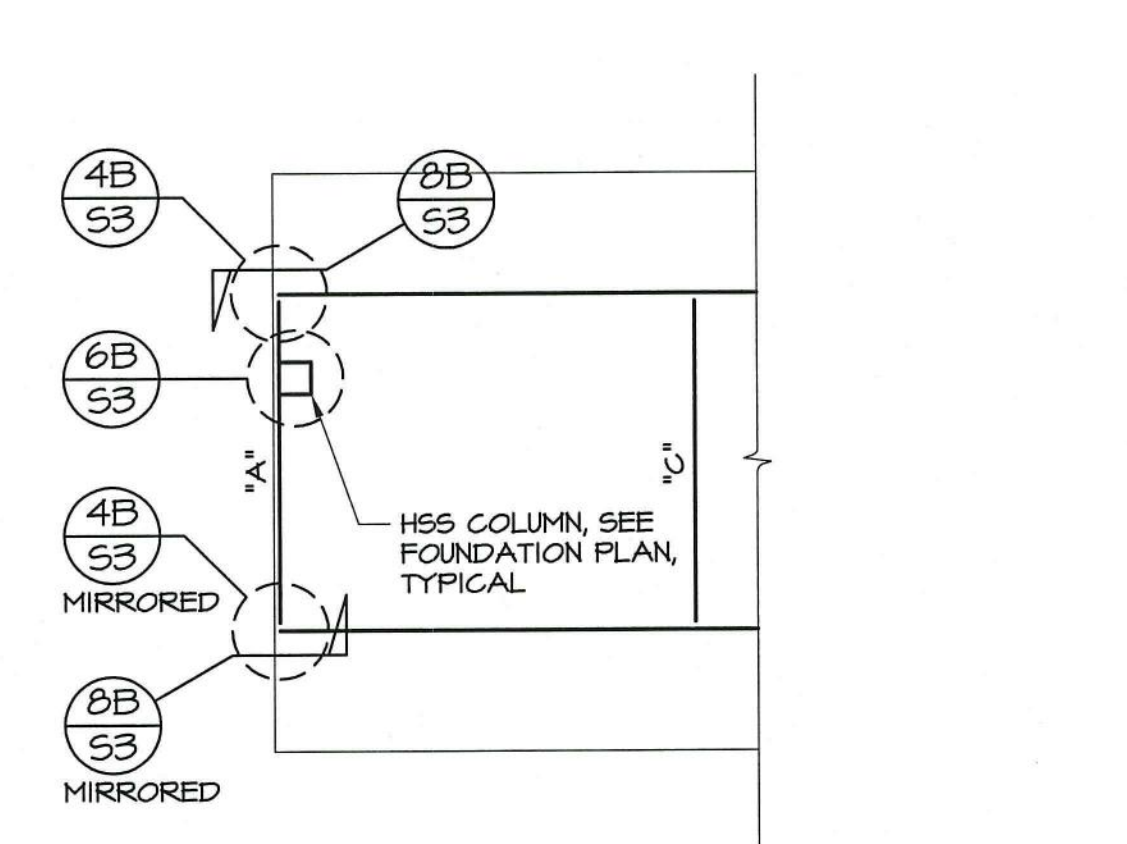
3. COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 3'-6" MAX. FROM BOTTOM, SEE SECTION 6K/53.

4. PROVIDE Ø-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.

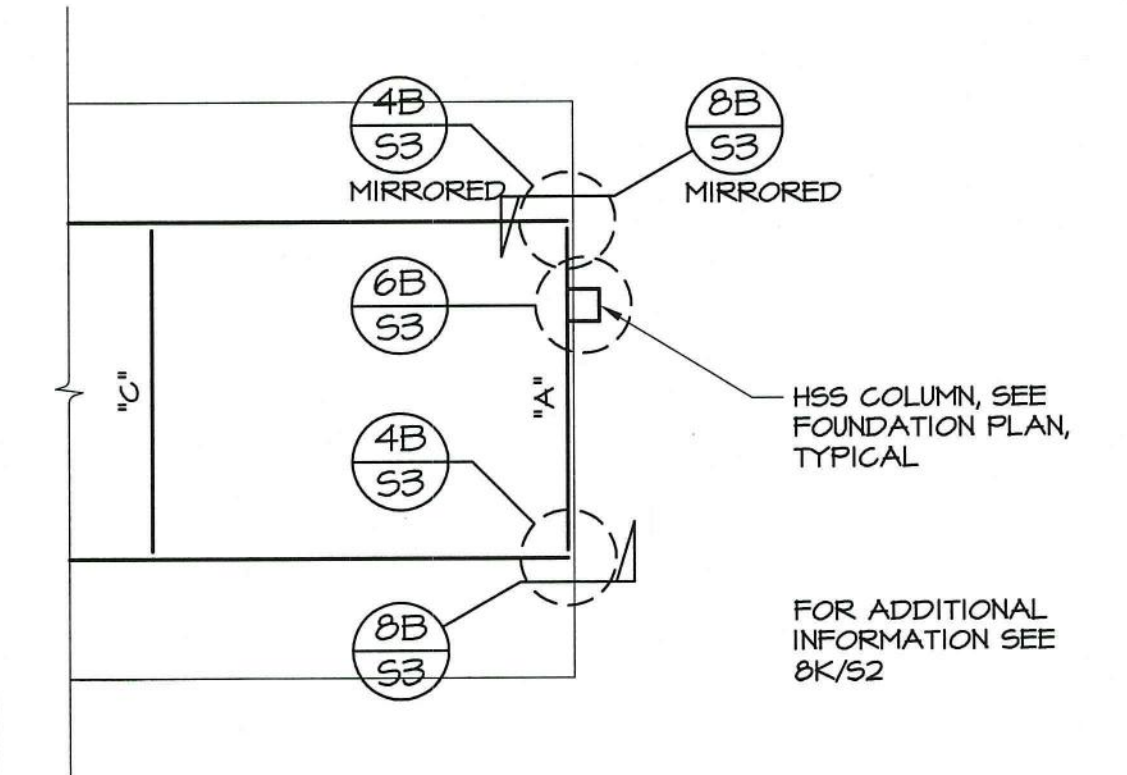
5. PROVIDE 10-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.

6. IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE

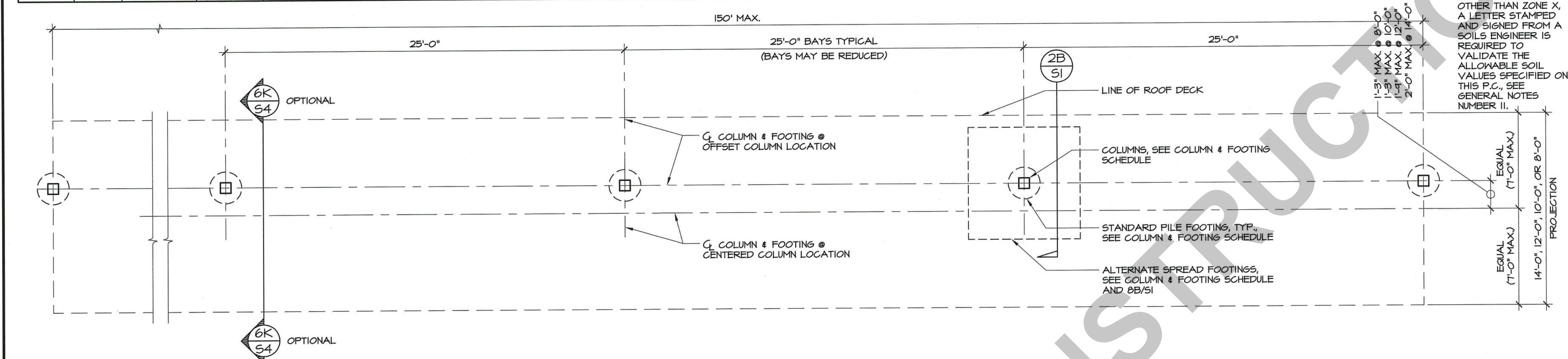
- NOTES:
1. ALTERNATE SPREAD FOOTINGS ARE OPTIONAL FOOTINGS @ AN ADDITIONAL COST.
 2. NO SNOW LOAD ALLOWED W/ 3 P.S.F. FIRE SPRINKLER DEAD LOAD.
 3. COLUMN EMBEDMENT OF PILE FOOTING TO EXTEND INTO FOOTING 3'-6" MAX. FROM BOTTOM, SEE SECTION 6K/S3.
 4. PROVIDE 8-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.
 5. PROVIDE 10-#6 REBARS EACH WAY @ TOP & BOTTOM OF FOOTING.
 6. IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C., SEE GENERAL NOTES NUMBER 11.



PLAN @ UPPER ROOF WHERE STEP OCCURS



PLAN @ LOWER ROOF WHERE STEP OCCURS



FOUNDATION PLAN

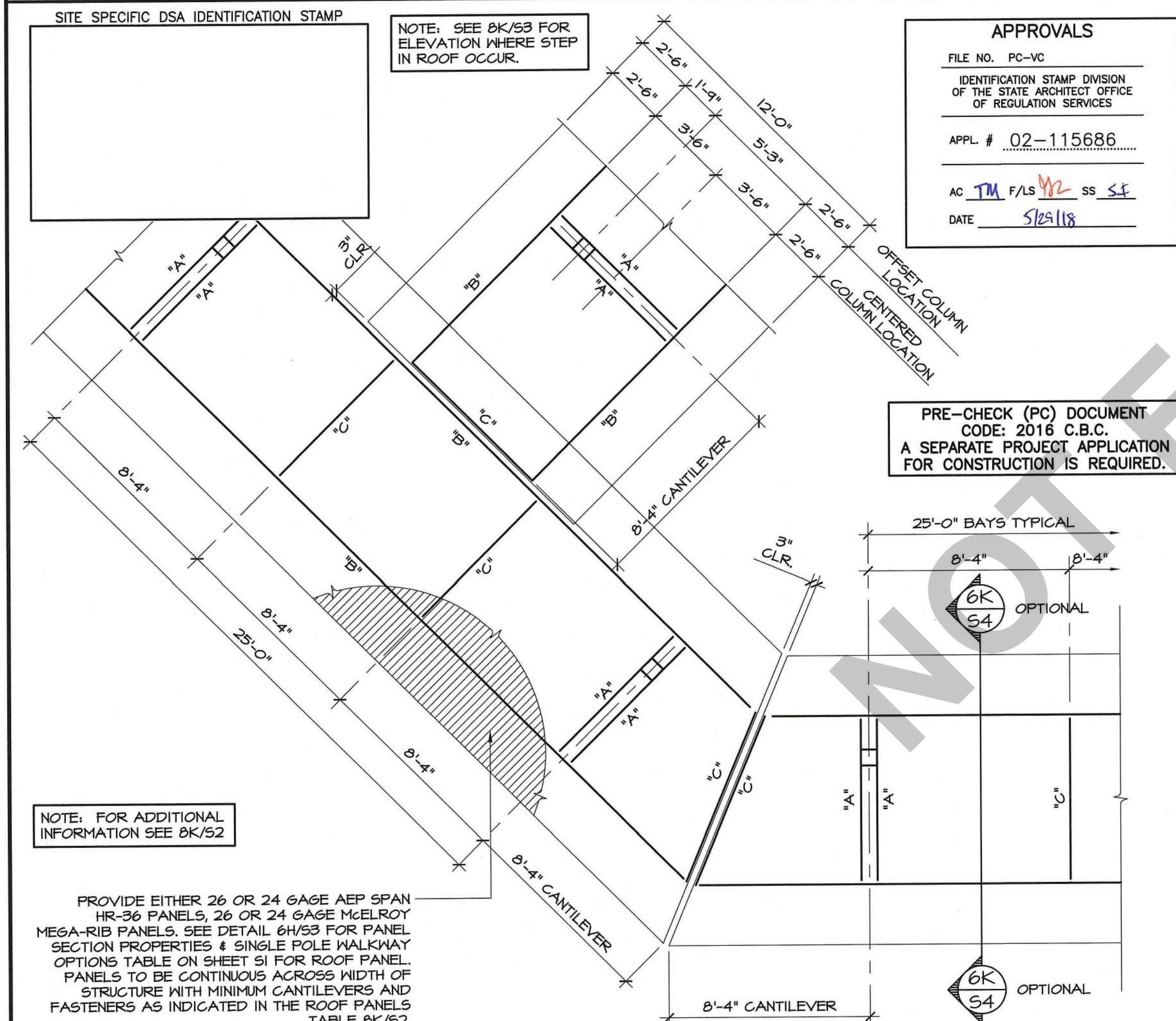
SCALE 1/4" = 1'-0"

4H

ROOF FRAMING PLAN

SCALE 1/4" = 1'-0"

4K

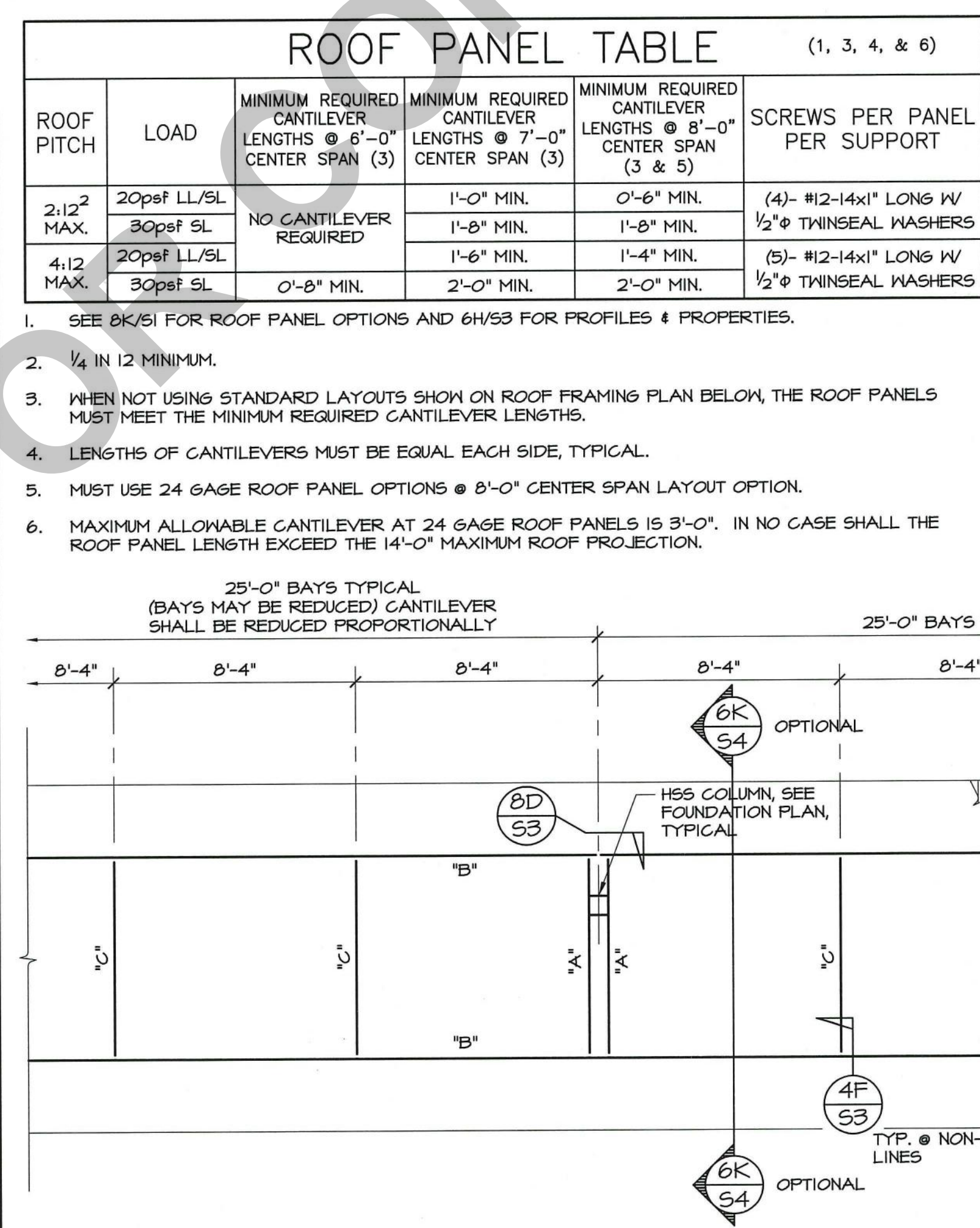


ROOF FRAMING PLAN @ 45° OPTION

SCALE 1/4" = 1'-0"

8B

ROOF FRAMING PLANS

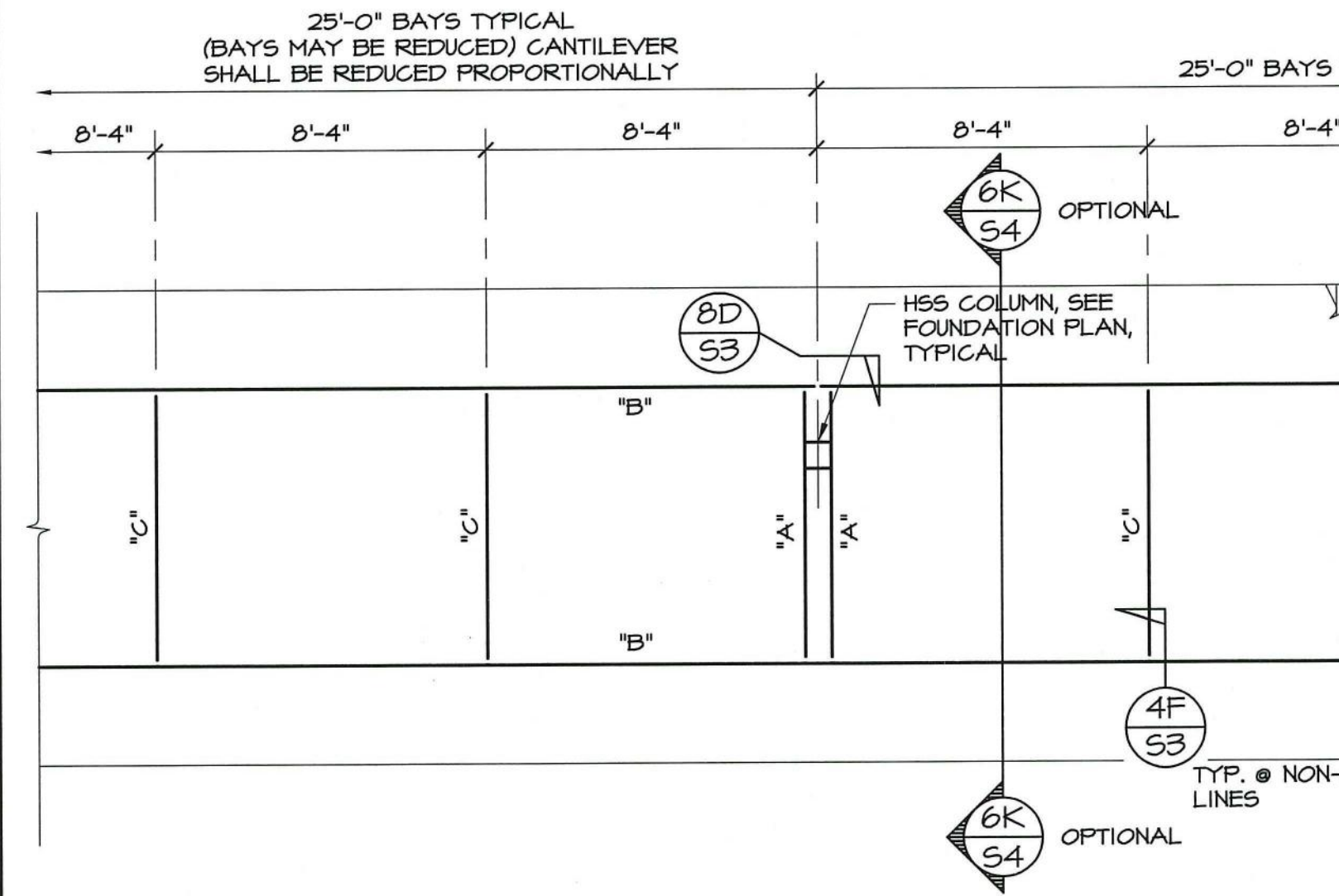


ROOF PANEL TABLE

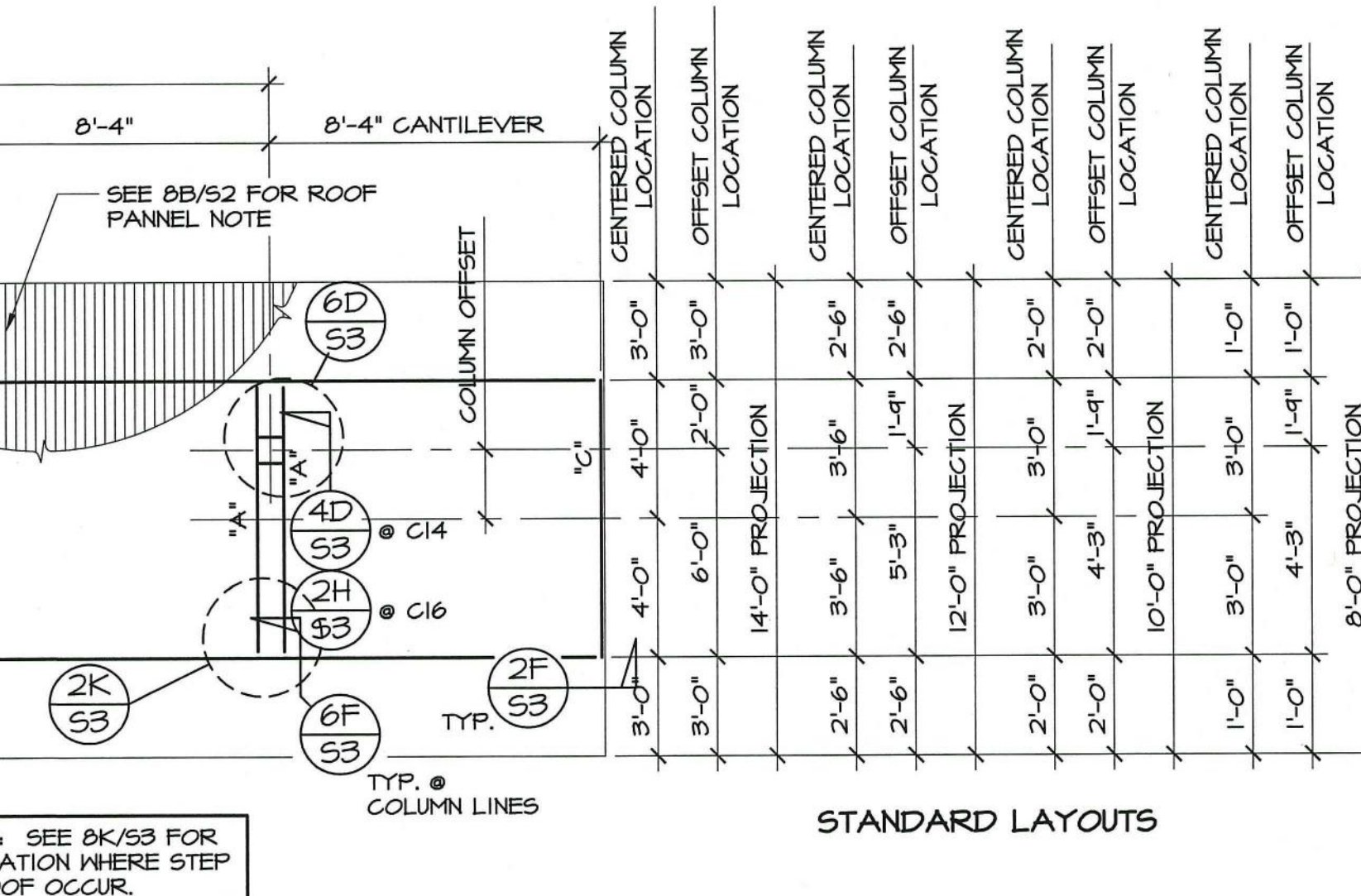
(1, 3, 4, & 6)

ROOF PITCH	LOAD	MINIMUM REQUIRED CANTILEVER LENGTHS @ 8'-0" CENTER SPAN (3)	MINIMUM REQUIRED CANTILEVER LENGTHS @ 7'-0" CENTER SPAN (3)	MINIMUM REQUIRED CANTILEVER LENGTHS @ 8'-0" CENTER SPAN (3 & 5)	SCREWS PER PANEL PER SUPPORT
2:12 MAX.	20psf LL/SL 30psf SL	NO CANTILEVER REQUIRED	1'-0" MIN. 1'-6" MIN.	0'-6" MIN. 1'-6" MIN.	(4)- #12-14x1" LONG W/ 1/2" ϕ THINSEAL WASHERS
4:12 MAX.	20psf LL/SL 30psf SL	0'-8" MIN.	2'-0" MIN.	1'-4" MIN. 2'-0" MIN.	(5)- #12-14x1" LONG W/ 1/2" ϕ THINSEAL WASHERS

1. SEE 8K/S1 FOR ROOF PANEL OPTIONS AND 6H/S3 FOR PROFILES & PROPERTIES.
2. 1/4 IN 12 MINIMUM.
3. WHEN NOT USING STANDARD LAYOUTS SHOWN ON ROOF FRAMING PLAN BELOW, THE ROOF PANELS MUST MEET THE MINIMUM REQUIRED CANTILEVER LENGTHS.
4. LENGTHS OF CANTILEVERS MUST BE EQUAL EACH SIDE, TYPICAL.
5. MUST USE 24 GAGE ROOF PANEL OPTIONS @ 8'-0" CENTER SPAN LAYOUT OPTION.
6. MAXIMUM ALLOWABLE CANTILEVER AT 24 GAGE ROOF PANELS IS 3'-0". IN NO CASE SHALL THE ROOF PANEL LENGTH EXCEED THE 14'-0" MAXIMUM ROOF PROJECTION.



BEAM TABLE						
PITCH	PROJECTION	LOADING	COLUMN OFFSET	CROSS BEAM "A"	SIDE BEAM "B"	BLOCKING & END CLOSURES "C"
1/4:12 MIN. 2:12 MAX.	$\leq 12'-0"$	20psf LL/SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C14x2 1/2x12 GAGE	20 GAGE CEE W 1 1/2" FLANGES, DEPTH TO MATCH SIDE BEAM DEPTH, SEE 2F & 4F/S3
		30psf SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	
	$> 12'-0"$, $\leq 14'-0"$	20psf LL/SL	1'-0" TO 2'-0"	C16x3 1/2x12 GAGE	C16x3 1/2x12 GAGE	
		30psf SL	0'-0" TO 0'-10"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	
4:12 MAX.	$\leq 12'-0"$	20psf LL/SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	20 GAGE CEE W 1 1/2" FLANGES, DEPTH TO MATCH SIDE BEAM DEPTH, SEE 2F & 4F/S3
		30psf SL	0'-0" TO 1'-4"	C14x2 1/2x12 GAGE	C16x3 1/2x12 GAGE	
	$> 12'-0"$, $\leq 14'-0"$	20psf LL/SL	1'-4" TO 2'-0"	C16x3 1/2x12 GAGE	C16x3 1/2x12 GAGE	
		30psf SL	0'-0" TO 1'-2"	C16x3 1/2x12 GAGE	C16x3 1/2x12 GAGE	



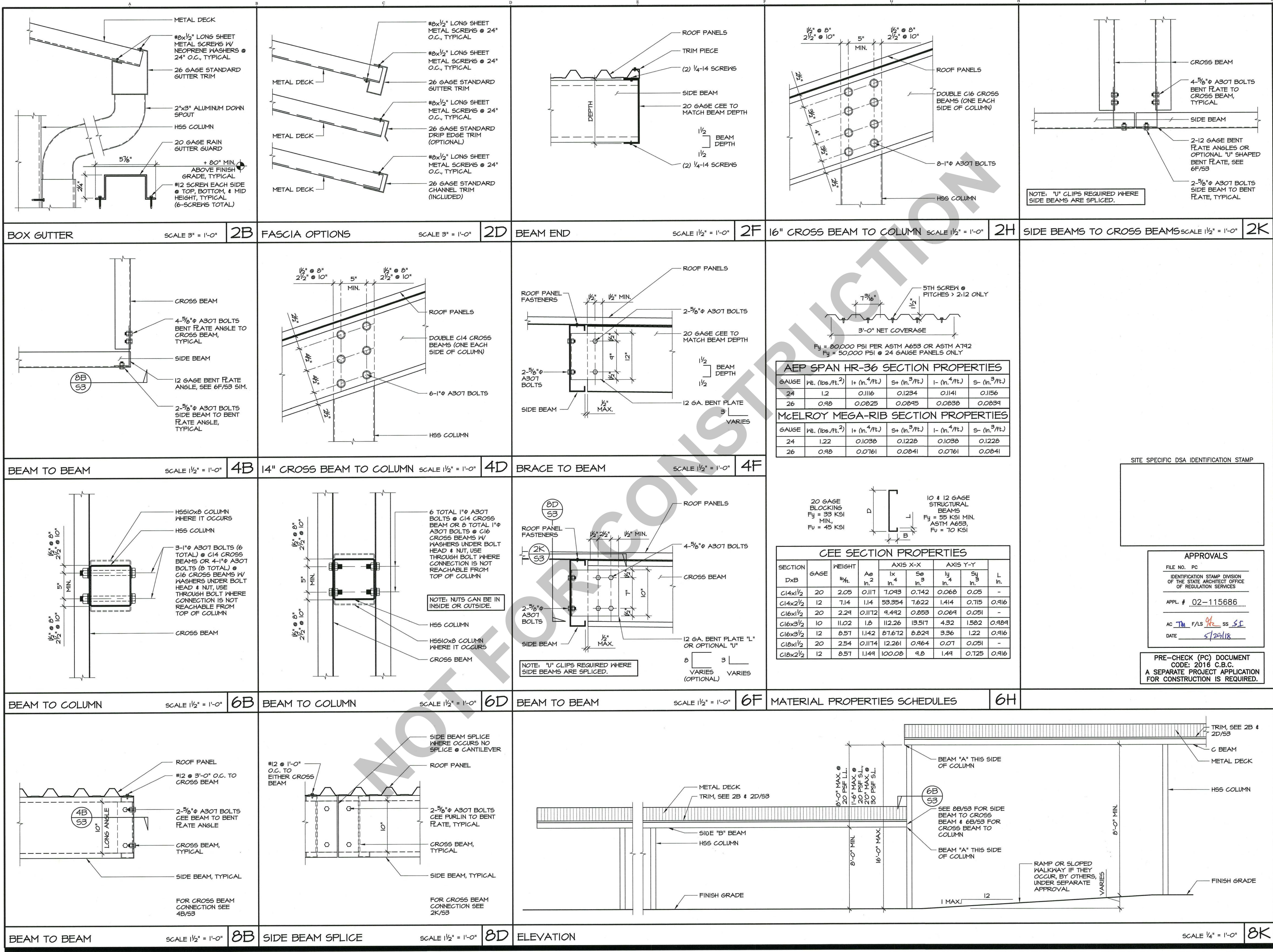
STANDARD LAYOUTS

SCALE 1/4" = 1'-0"

8K

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY:	CHKD BY:
T.E.H.	G.B.H.
DATE:	5/25/18
PROJECT NO:	16320
DRAWING TITLE	ROOF FRAMING PLANS



SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY: T.E.H. CHKD BY: G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE
SECTION
TYPICAL ELEVATION
DETAILS

SHEET NUMBER

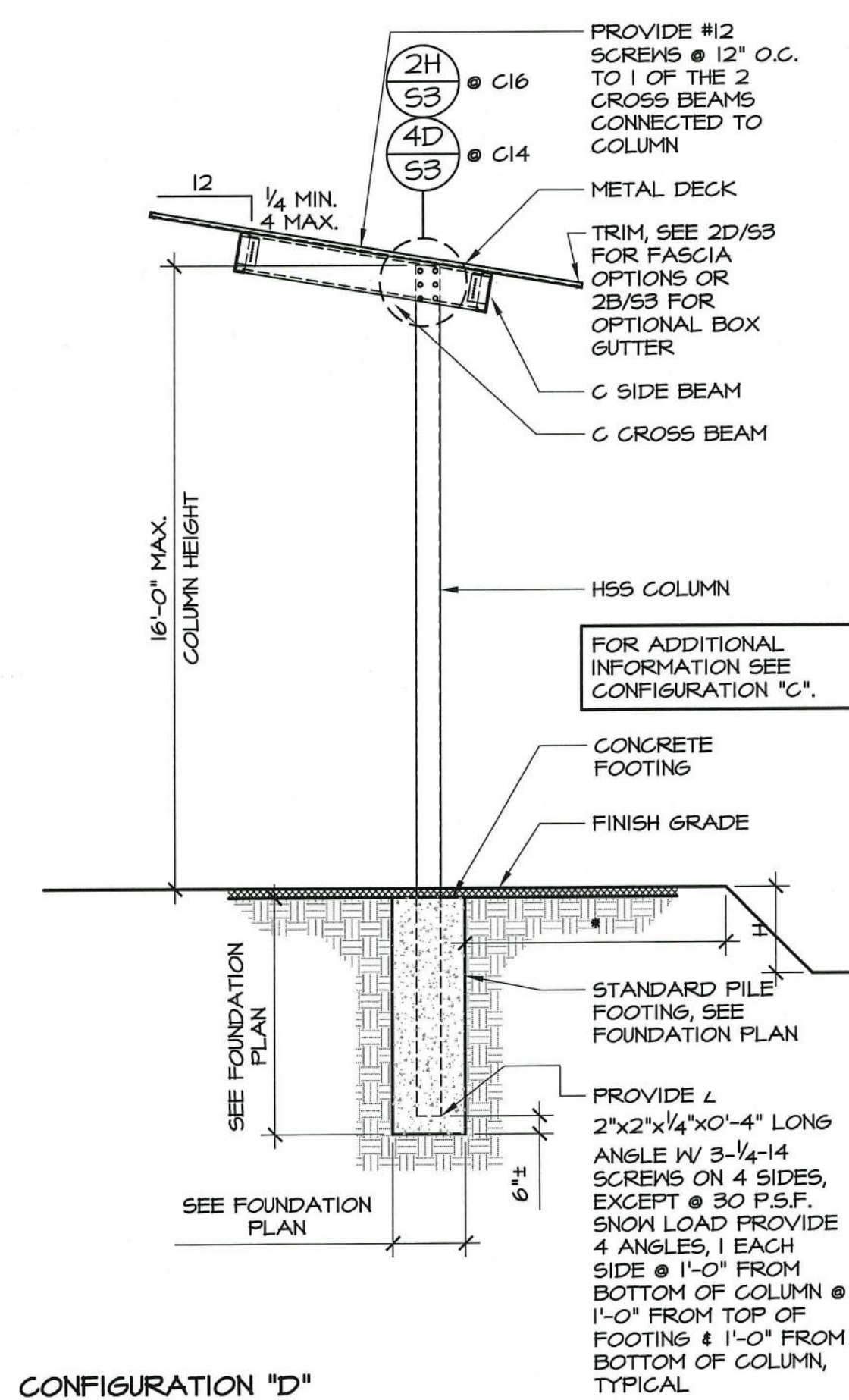
S3

OF 4 SHEETS

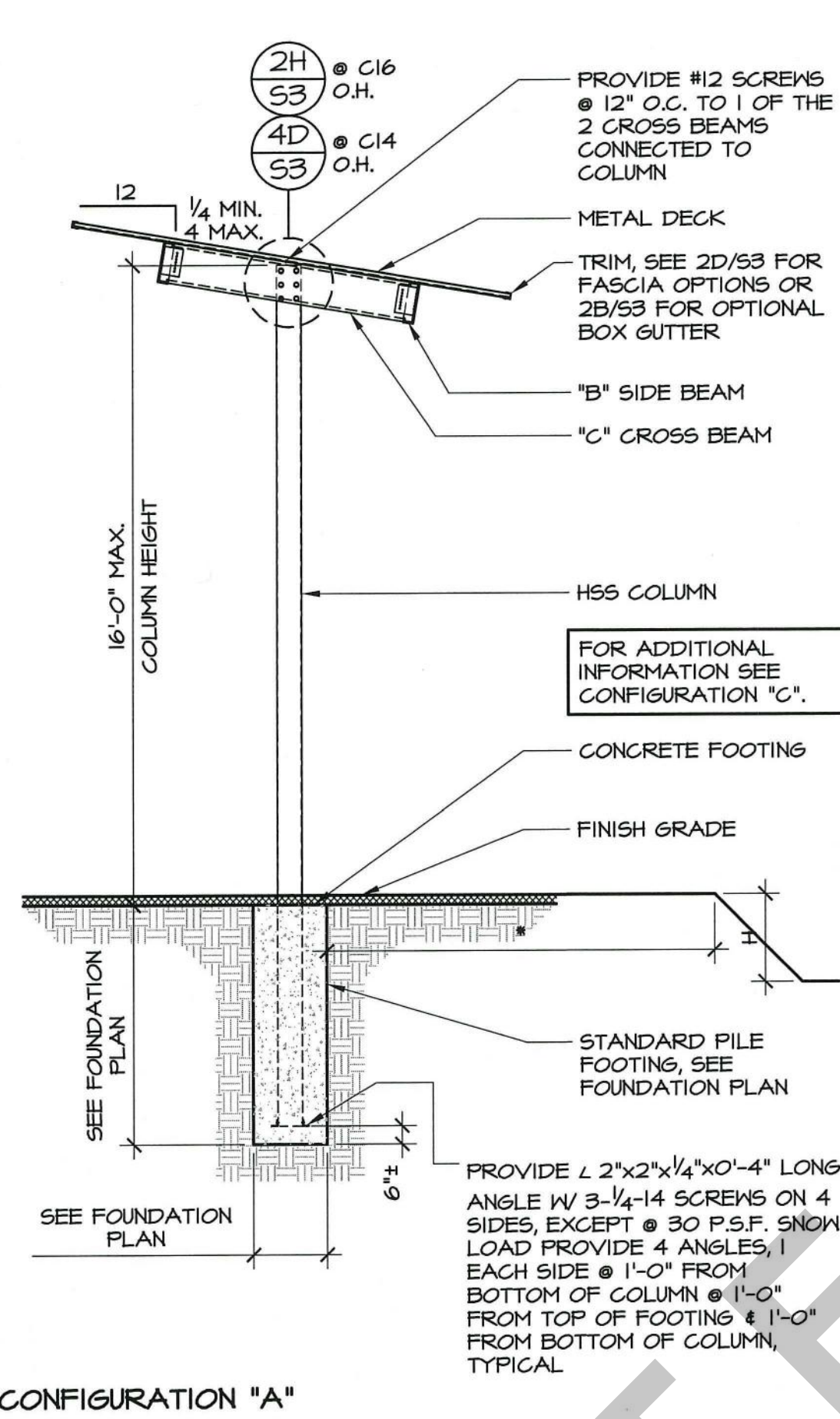
2374 WEST WHITEDALE
VISALIA, CA 93277

Grand
Glover and Associates
STRUCTURAL ENGINEERS
(559) 734-6675
FAX (559) 734-5332
Email: gloverse@gmail.com

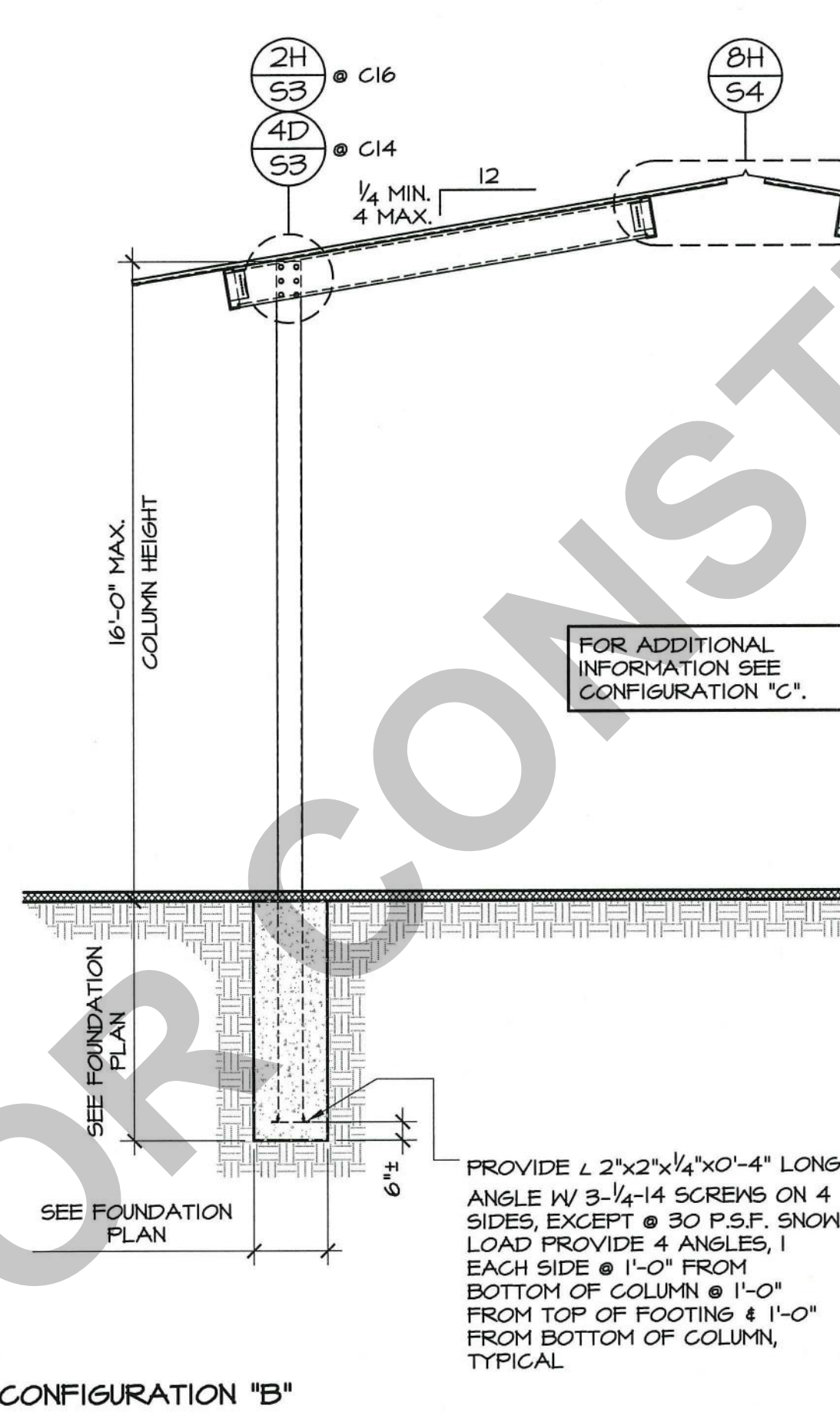
C:\Users\Cad\Documents\Drawings\Projects\6320\Structural Drawings\S04SecDet.dwg May 25 2018 10:58am Single Post Walkway Covers 2016 - Valley School Shelters



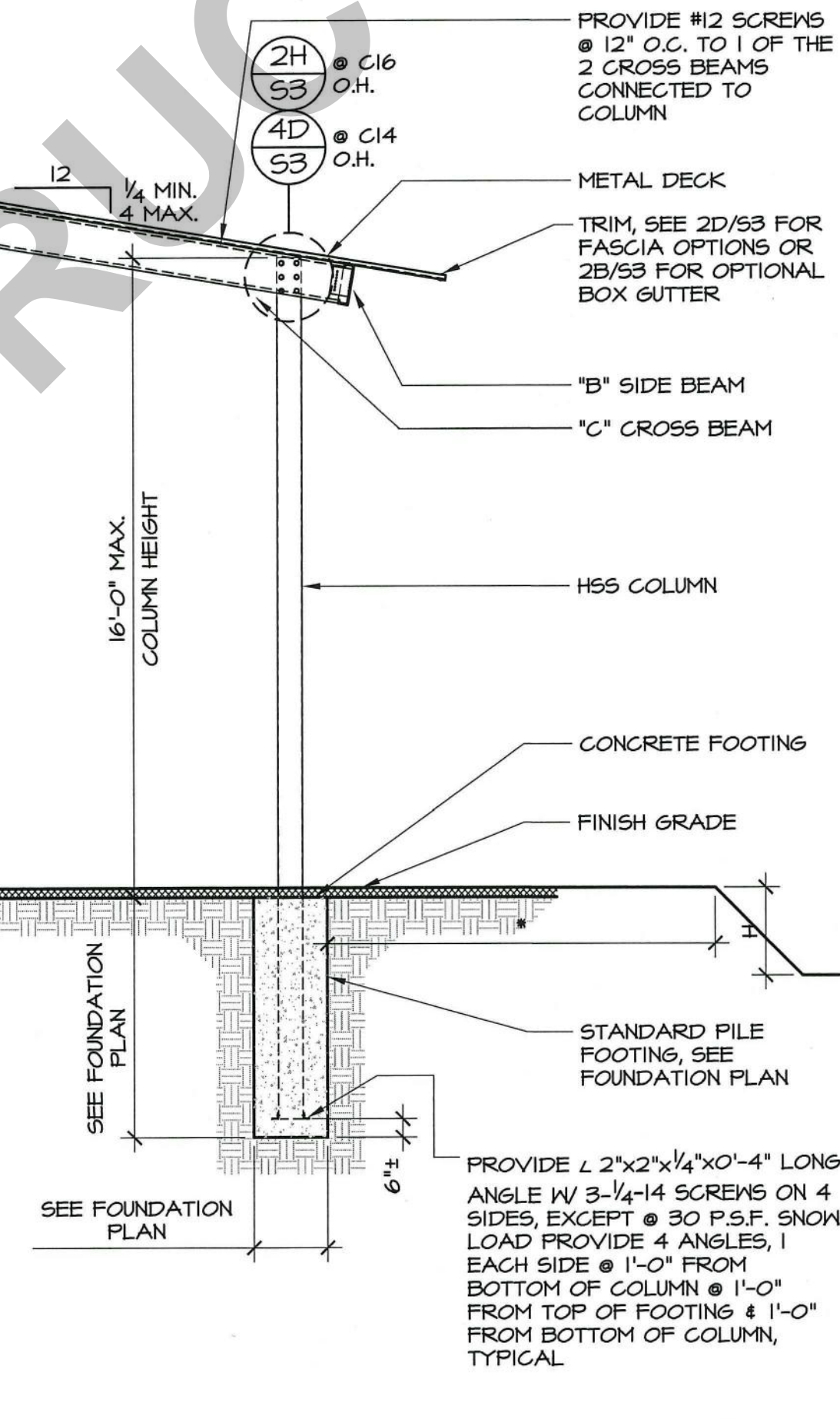
CONFIGURATION "D"



CONFIGURATION "A"



CONFIGURATION "B"



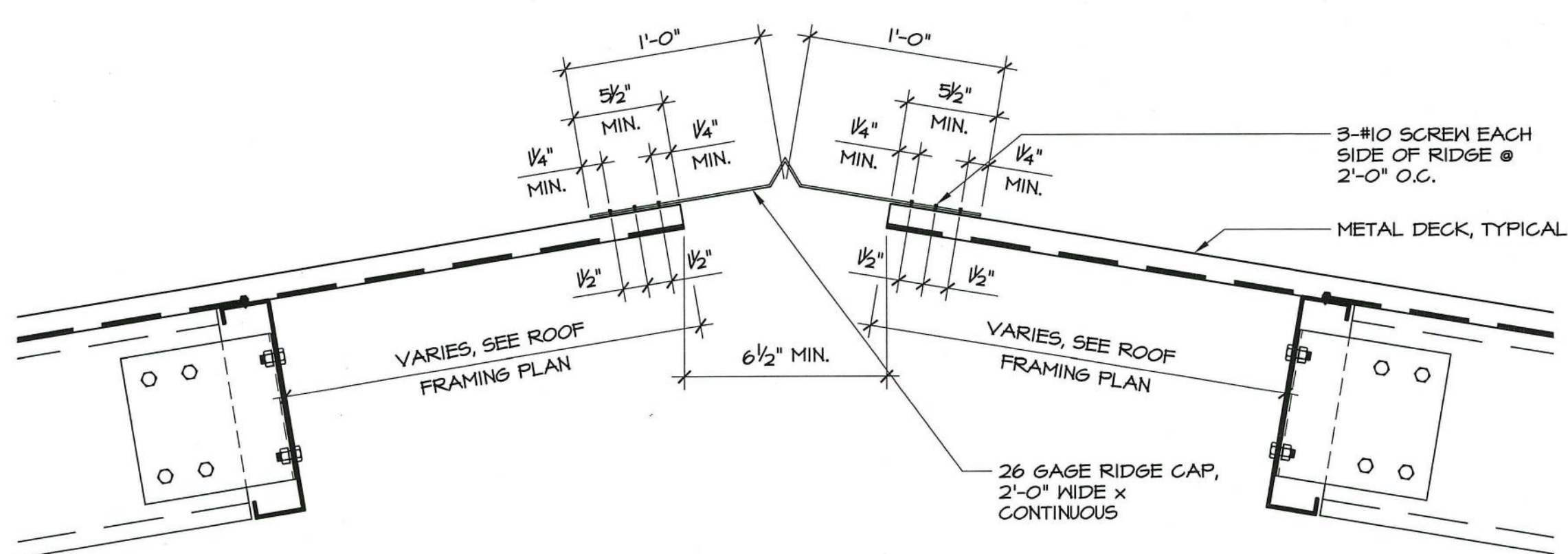
CONFIGURATION "C"

* DISTANCE TO TOP OF SLOPE EQUAL TO THE LEAST OF 1/2 OR 40 FEET BUT NOT LESS THAN 7 PIER DIAMETERS.

SECTIONS

SCALE 1/4" = 1'-0"

6K



RIDGE CAP

SCALE 1/2" = 1'-0"

8H

SITE SPECIFIC DSA IDENTIFICATION STAMP

APPROVALS

FILE NO. PC
IDENTIFICATION STAMP DIVISION
OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES

APPL. # 02-115686

AC TM F/LS SS
DATE

PRE-CHECK (PC) DOCUMENT
CODE: 2016 C.B.C.
A SEPARATE PROJECT APPLICATION
FOR CONSTRUCTION IS REQUIRED.

SINGLE POST
WALKWAY COVER
VALLEY SCHOOL SHELTERS

PROJECT:

REVISIONS			
NO.	DATE	BY	DESCRIPTION
1			
2			

DWN BY:
T.E.H.

CHKD BY:
G.B.H.

DATE: 5/25/18

PROJECT NO: 16320

DRAWING TITLE

SECTION
TYPICAL ELEVATION
DETAILS

SHEET NUMBER

S4

OF 4 SHEETS

2374 WEST WHITENDALE
VISALIA, CA 93277

Grand
Gomer and Associates
STRUCTURAL ENGINEERS

(559) 734-6675
FAX (559) 734-5232
Email: ghomerse@gmail.com

Phone (559) 329-8830
Fax (559) 329-8807

CA Lic. 981366

LABOR AND / OR MATERIALS WARRANTY/GUARANTEE

Warranty for that certain project known as <School Name>, located at: <School Address>, installed for:

<School District>

We hereby warrant that the labor and / or materials which we have provided for the above project have been completed in accordance with the requirements of specifications sections for lunch shelters and walkway covers, and the Contract Documents.

We agree to repair or replace any or all of our work, together with any other adjacent work which may be displaced by so doing, that may prove to be defective in its workmanship or material, within a period of one year from the date of final acceptance by Owner or from the Date of Certificate of Occupancy, whichever is the earlier.

We also agree to repair any and all damages resulting from such defects, all without additional expense to the Owner. Ordinary wear and tear and unusual abuse or neglect accepted.

In the event of our failure to comply with the above mentioned conditions within 30 days after being notified in writing by the Owner, we collectively or separately do hereby authorized the Owner to proceed to have such defective work repaired or replaced and made good at our expense, and we will honor and pay the costs and charges therefrom upon demand.

Signed: _____ Date: <Date>
(Name/Title) Michael Messerschmidt (Owner)

Insurance Company:
Street Address:
City/State/Zip:

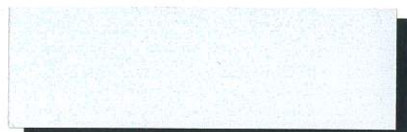
For maintenance, repair, or replacement service, contact:
Valley School Shelters
PO BOX 177
Tulare, CA 93275
(559) 329-8830

Contact person: Michael Messerschmidt

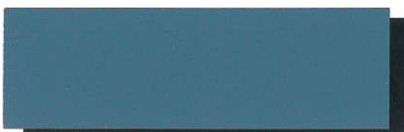


Valley School Shelters
P.O. Box 1499
Tulare, CA 93275-1499
559-329-8830
www.valleyschoolshelters.com

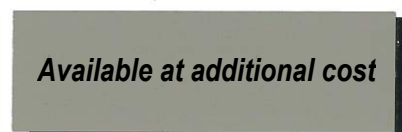
M
COLORS BY
McELROY METAL



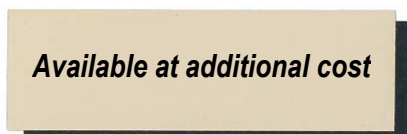
SP / REGAL WHITE



ROMAN BLUE



CLAY



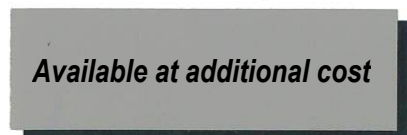
IVORY



SURREY BEIGE



PATRICIAN BRONZE



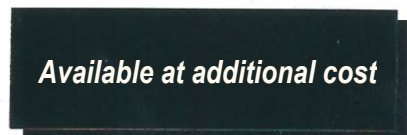
ASH GRAY



LIGHT STONE



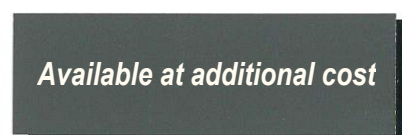
AUTUMN RED



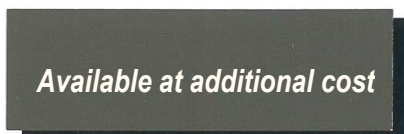
MATTE BLACK



TUDOR BROWN



CHARCOAL



TERRATONE
KYNAR 500® ONLY



EVERGREEN



BRANDYWINE



HARTFORD GREEN

• ADDITIONAL COST WILL APPLY FOR BRANDYWINE AND HARTFORD GREEN.
• AVAILABLE IN KYNAR 500® ONLY.

COLOR SELECTOR



ALL KYNAR 500® COLORS ARE ENERGY STAR COMPLIANT.

MM210CA



Request for Information

RFI #: _____

RFI Title: _____

Date Submitted: _____

Respond By: _____

Project Name: _____

Submitted By: _____

Project Customer: _____

Drawing(s): _____

Potential Cost Impact: Yes ☐ No ☐

Detail(s): _____

Potential Schedule Impact: Yes ☐ No ☐

Specifications: _____

INFORMATION REQUESTED & RECOMMENDED SOLUTION:

RESPONSE:

Signature: _____

Date: _____

This RFI is intended to provide clarification to the contract documents. M Bar C will proceed with work according to the response to this RFI unless otherwise directed or if M Bar C considers the response a change to the contract documents. In which case, M Bar C will submit written notice to the Contracting Officer immediately and stop all work related to the response unless specifically directed to proceed.



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